

1 **Large amplitude motions of a submerged circular cylinder in water with an ice**
2 **cover**

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9 **Abstract**

10 The hydrodynamic problem of a circular cylinder undergoing large amplitude oscillations in water
11 covered by an ice sheet is investigated. The ice sheet is modelled by a thin elastic sheet and
12 uniform physical properties are assumed. The fluid is assumed to be inviscid, incompressible and
13 homogeneous, and the depth to be infinite. The boundary condition on the ice sheet is linearized
14 and satisfied on its mean position, while the fully nonlinear boundary condition is imposed on the
15 instantaneous position of the body surface. The velocity potential is formulated by the multipole
16 expansion method in the polar coordinate system with its origin fixed at the centre of the cylinder.
17 Detailed results through the hydrodynamic force and deformation of the ice sheet are provided.
18 The effects of the ice sheet properties, and motion amplitude and frequency are investigated.

19 **Keywords**

20 Ice sheet; circular cylinder; large amplitude oscillation; multipole expansion.

21 **1. Introduction**

22 Water wave/structure interaction has always been a major interest in fluid mechanics. This is
23 primarily because of the importance of the ocean. It provides a major means for trade, as well as is
24 a rich source of food and resource. Its wave, current and wind can also provide clean and
25 renewable energy. Therefore, there has been extensive research on this topic. However, the most
26 work focuses on a body in open seas, in which the water surface is in direct contact with air, and
27 the surface is commonly called free surface.

28 A closely related problem is when the water surface is covered by an ice sheet, which can be
29 typically seen in the Arctic or Antarctic regions. There has also been a large number of work on
30 ice sheet/wave interaction. Review of early work was given by Squire, et al. [1] and more recent

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1 one was given by Squire [2]. Much of the work reviewed is about wave interaction with ice sheet.
2 It is quite often that the ice cover on the water surface is treated as an infinite or semi-infinite
3 elastic sheet or plate. Fox and Squire [3] considered a two dimensional problem of wave from
4 open water propagating into ice sheet. This method was extended later to consider the oblique
5 incidence problem [4]. Sahoo, et al. [5] considered similar problems using an inner product with
6 orthogonality in the eigenfunction expansions method. Eatock Taylor and Ohkusu [6] investigated
7 the flexural vibrations of a uniform elastic plate using the Green function. Other work includes
8 those by Balmforth and Craster [7], Tkacheva [8], Tkacheva [9] and Chung and Fox [10]. More
9 recently, Sturova and Korobkin [11] solved the problem of a floating elastic plate under periodic
10 pressure.

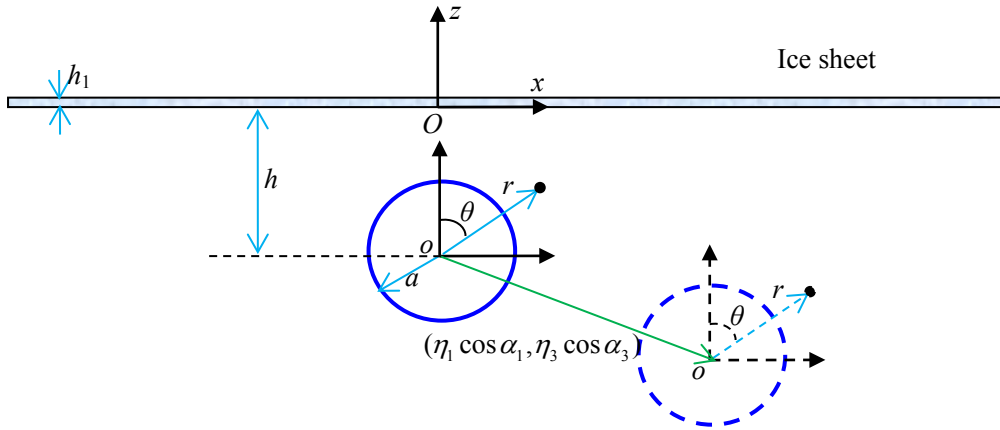
11 There have also been strong interests in wave/ice/body interaction, especially more recently, as a
12 new shipping through Arctic becomes more feasible in coming decades. Das and Mandal [12]
13 studied the oblique wave scattering by a circular cylinder submerged beneath an ice-cover through
14 the 2-D multipole expansion method. Sturova [13] considered the radiation problems of a
15 horizontal cylinder submerged in fluid covered by an ice sheet with semi-infinite length, and the
16 corresponding Green function was constructed using eigenfunction expansions. The method was
17 then extended to the cases of two semi-infinite elastic plates connected by the vertical and flexural
18 rotational springs [14], and the ice floe or polynya [15]. In addition, Savin and Savin [16] and
19 Il'ichev, et al. [17] used a two-dimensional dipole situated under the ice sheet to investigate the
20 generation of the flexural-gravity waves.

21 In the above work, the fully linearized theory has been used, in which all the conditions are
22 satisfied on the mean position of the boundary and higher order terms are ignored from the
23 condition. This is valid when the motion amplitude is small. Wu [18] considered a problem of a
24 circular cylinder submerged below the free surface. He has argued that when the body motion is
25 large, the boundary condition has to be satisfied on its instantaneous position. However, when the
26 body is sufficiently deeply submerged, the free surface boundary condition can still be linearized.
27 The solution of such a model subsequently led to some very interesting and new results. Here we
28 shall consider a circular cylinder in a large amplitude motion below an ice sheet. The body surface
29 boundary condition is satisfied on its exact position while the condition on the ice sheet is
30 linearized. The ice sheet is treated as a thin elastic beam of infinite extent with uniform properties,
31 and the incompressible velocity potential flow theory is adopted in the fluid domain. The polar
32 coordinate system, with its origin fixed at the centre of the circular cylinder, is introduced to
33 describe the instantaneous body surface boundary condition. The velocity potential is written in
34 terms of the multipole expansion [19] with unknown coefficients. Through this the problem is

1 transformed into an infinite system of linear equations. Numerical results for the hydrodynamic
 2 force and deformation of the ice sheet are provided and discussions are made. The effect of body
 3 oscillation amplitude, frequency, as well as ice thickness are investigated. A far field formula for
 4 the horizontal steady force is also derived and has been used for verifying the results.

5 **2. The problem and mathematical equations**

6 The problem of a submerged horizontal circular cylinder of radius a undergoing sinusoidal
 7 oscillation with frequency ω in fluid with an ice sheet is sketched in Fig. 1. A Cartesian
 8 coordinate system $O-xz$ is defined with the x -axis along the undisturbed mean water surface,
 9 and the z -axis pointing vertically upwards. A body-fixed polar coordinate system (r, θ) is also
 10 defined, with its origin fixed at the centre of the cylinder with its mean position located at $(0, -h)$.



11
 12 Fig. 1. Definition of the coordinate systems and sketch of the problem.

13 The relationships between the two coordinate systems can be written as

14
$$x = r \sin \theta + \eta_1 \cos \alpha_1 \tag{1}$$

15
$$z = r \cos \theta - (h - \eta_3 \cos \alpha_3) \tag{2}$$

16
$$\alpha_j = \omega t + \gamma_j \tag{3}$$

17 where η_1 and η_3 are the amplitude of horizontal and vertical motions respectively, γ_1 and γ_3 are
 18 the corresponding initial phases. The condition $\eta_3 + a < h$ keeps the cylinder beneath the ice sheet
 19 at all time.

20 The ice sheet with uniform properties is assumed to be in contact with water for all times. The
 21 draught of the ice sheet is neglected. The fluid of infinite depth is assumed to be inviscid,
 22 incompressible, homogeneous, and the flow is irrotational. Then the velocity potential Φ can be
 23 introduced, which satisfies the Laplace equation in the fluid domain

24
$$\nabla^2 \Phi = 0 \tag{4}$$

25 On the ice sheet S_F or $z = 0$, the combination of the dynamic and kinematic boundary conditions
 26 can be written as [1]

$$1 \quad \left(D \frac{\partial^4}{\partial x^4} + K \frac{1}{g} \frac{\partial^2}{\partial t^2} + 1 \right) \frac{\partial \Phi}{\partial z} + \frac{1}{g} \frac{\partial^2 \Phi}{\partial t^2} = 0 \quad (5)$$

2 where $D = Eh_1^3 / 12\rho g(1-\nu^2)$ and $K = h_1\rho_1 / \rho$, and E , ν , h_1 and ρ_1 are respectively the Young's
3 modulus, Poisson's ratio, the thickness and the densities of the ice sheet, and ρ is the density of
4 the fluid.

5 On the instantaneous body surface S_0 or $r = a$, the normal velocity of fluid should be equal to
6 that of the body based on the impermeable condition. Thus, we have

$$7 \quad \frac{\partial \Phi}{\partial n} = -\omega(n_1\eta_1 \sin \alpha_1 + n_3\eta_3 \sin \alpha_3) \quad (6)$$

8 where $\vec{n} = (n_1, n_3)$ is the normal vector pointing into the body, and for the circle above its
9 components along x -axis and z -axis can be written as $n_1 = -\sin \theta$ and $n_3 = -\cos \theta$ respectively.

10 In the far field, i.e. $x \rightarrow \pm\infty$, a radiation condition should be imposed, which requires the radiated
11 waves to propagate outwards.

12 To solve the above boundary value problem, we may rewrite the total velocity potential in the
13 following form

$$14 \quad \Phi = -\omega\eta_1 \operatorname{Re}(\phi_1 e^{i\gamma_1}) - \omega\eta_3 \operatorname{Re}(\phi_3 e^{i\gamma_3}) \quad (7)$$

15 ϕ_j satisfies the same equations as Φ apart from the body surface boundary condition which can
16 be given as

$$17 \quad \frac{\partial \phi_1}{\partial r} = \frac{1}{2} (e^{i(\omega t + \theta)} - e^{i(\omega t - \theta)}) \quad (8)$$

$$18 \quad \frac{\partial \phi_3}{\partial r} = \frac{i}{2} (e^{i(\omega t + \theta)} + e^{i(\omega t - \theta)}) \quad (9)$$

19 **3. Method of solution**

20 In the context of the present problem, the velocity potential can be written in terms of multipole
21 expansion [19]. Each term in the series satisfies the Laplace equation and boundary conditions
22 apart from that on the cylinder surface.

23 *3.1 Expansion of the velocity potential*

24 When the cylinder is in oscillation with small amplitude, the boundary conditions in Eqs. (8) and
25 (9) can be imposed on its mean position. As a result, the time factor $\exp(i\omega t)$ can be taken out. At
26 large amplitude, Eqs. (8) and (9) should be imposed on the instantaneous position of the body. Its
27 sinusoidal motion with frequency ω will generate fluid motions of frequencies $n\omega$ ($n=1,2,\dots$).
28 Thus, we can write the potentials ϕ_j in terms of multipole expansions in the polar coordinate
29 system as

$$30 \quad \phi_j = \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} A_m^s a^m \left[\frac{e^{im\theta + is\omega t}}{r^m} + f_m^s(r, \theta, t) \right] + \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} B_m^s a^m \left[\frac{e^{-im\theta + is\omega t}}{r^m} + g_m^s(r, \theta, t) \right] \quad (10)$$

31 where f_m^s and g_m^s are introduced to satisfy the boundary condition on the ice sheet. To do that, we

1 use [18]

$$2 \quad \frac{e^{\pm im\theta + is\omega t}}{r^m} = \frac{1}{(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} (-1)^q (\pm i)^q e^{ip\alpha_3 + iq\alpha_1 + is\omega t} \int_0^{\infty} k^{m-1} e^{[-k(z+h) \pm ikx]} I_p(k\eta_3) J_q(k\eta_1) dk \quad (11)$$

3 where $J_q(k\eta_1)$ and $I_p(k\eta_3)$ are the Bessel functions and the modified Bessel functions
4 respectively. We may then write f_m^s and g_m^s in the following form

$$5 \quad f_m^s, g_m^s = \frac{1}{(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} (-1)^q (\pm i)^q e^{ip\alpha_3 + iq\alpha_1 + is\omega t} \int_0^{\infty} A(k) k^{m-1} e^{[k(z-h) \pm ikx]} I_p(k\eta_3) J_q(k\eta_1) dk \quad (12)$$

6 Substituting Eqs. (11) and (12) into Eq. (5), we obtain

$$7 \quad A(k) = \frac{k(Dk^4 - K\nu(p+q+s)^2 + 1) + \nu(p+q+s)^2}{k(Dk^4 - K\nu(p+q+s)^2 + 1) - \nu(p+q+s)^2} \quad (13)$$

8 where $\nu = \omega^2 / g$.

9 Then the potential ϕ_j in Eq. (10) can be written as

$$10 \quad \begin{aligned} \phi_j = & \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} A_m^s a^m \left[\frac{e^{im\theta + is\omega t}}{r^m} + \frac{1}{(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} (-i)^q e^{ip\alpha_3 + iq\alpha_1 + is\omega t} \right. \\ & \times \int_0^{\infty} \frac{k(Dk^4 - K\nu(p+q+s)^2 + 1) + \nu(p+q+s)^2}{k(Dk^4 - K\nu(p+q+s)^2 + 1) - \nu(p+q+s)^2} k^{m-1} e^{[k(z-h) + ikx]} I_p(k\eta_3) J_q(k\eta_1) dk \\ & + \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} B_m^s a^m \left[\frac{e^{-im\theta + is\omega t}}{r^m} + \frac{1}{(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} (+i)^q e^{ip\alpha_3 + iq\alpha_1 + is\omega t} \right. \\ & \times \int_0^{\infty} \frac{k(Dk^4 - K\nu(p+q+s)^2 + 1) + \nu(p+q+s)^2}{k(Dk^4 - K\nu(p+q+s)^2 + 1) - \nu(p+q+s)^2} k^{m-1} e^{[k(z-h) - ikx]} I_p(k\eta_3) J_q(k\eta_1) dk \end{aligned} \quad (14)$$

11 where the integration route passes over the pole $k = \lambda$ with $p+q+s > 0$ and passes under the
12 pole $k = \lambda$ with $p+q+s < 0$ to satisfy the outgoing waves condition at $x \rightarrow \pm\infty$. Here λ is the
13 only real positive root of the dispersion equation

$$14 \quad k[Dk^4 - K\nu(p+q+s)^2 + 1] - \nu(p+q+s)^2 = 0 \quad (15)$$

15 at each given $p+q+s$. Eq. (14) then satisfies the governing Laplace equation and the boundary
16 conditions on the ice sheet and at infinity. The remaining boundary condition is that on the body
17 surface. To satisfy this remaining condition, we use [18]

$$18 \quad e^{k(z-h) \pm ikx} = e^{-2kh} \sum_{n=0}^{\infty} \frac{k^n r^n e^{\pm in\theta}}{n!} \sum_{p_1=-\infty}^{\infty} \sum_{q_1=-\infty}^{\infty} (\pm i)^{q_1} I_{p_1}(k\eta_3) J_{q_1}(k\eta_1) e^{iq_1\alpha_1} e^{ip_1\alpha_3} \quad (16)$$

19 to expand Eq. (14) in the polar coordinate system as

$$20 \quad \begin{aligned} \phi_j = & \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} A_m^s a^m \left[\frac{e^{im\theta + is\omega t}}{r^m} + \frac{1}{(m-1)!} \sum_{n=0}^{\infty} \frac{r^n e^{+in\theta}}{n!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{q_1=-\infty}^{\infty} \right. \\ & \times (-i)^q (+i)^{q_1} \exp(ip\alpha_3 + iq\alpha_1 + is\omega t + iq_1\alpha_1 + ip_1\alpha_3) F(m, n, p, q, p_1, q_1, p+q+s) \\ & + \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} B_m^s a^m \left[\frac{e^{-im\theta + is\omega t}}{r^m} + \frac{1}{(m-1)!} \sum_{n=0}^{\infty} \frac{r^n e^{-in\theta}}{n!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{q_1=-\infty}^{\infty} \right. \\ & \times (+i)^q (-i)^{q_1} \exp(ip\alpha_3 + iq\alpha_1 + is\omega t + iq_1\alpha_1 + ip_1\alpha_3) F(m, n, p, q, p_1, q_1, p+q+s) \end{aligned} \quad (17)$$

21 where

$$F(m, n, p, q, p_1, q_1, s) = \int_0^\infty \frac{k(Dk^4 - K\upsilon s^2 + 1) + \upsilon s^2}{k(Dk^4 - K\upsilon s^2 + 1) - \upsilon s^2} \times k^{m+n-1} e^{-2kh} I_{p_1}(k\eta_3) J_{q_1}(k\eta_1) I_p(k\eta_3) J_q(k\eta_1) dk \quad (18)$$

Invoking the boundary conditions in Eqs. (8) and (9), then using the orthogonality of trigonometric function, we have for ϕ_1

$$-m \frac{A_m^s}{a} + \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{u=-\infty}^{\infty} \times (-i)^q (+i)^{q_1} e^{i(q+q_1)\gamma_1 + i(p+p_1)\gamma_3} F(n, m, p, q, p_1, q_1, p+q+u) A_n^u = -\frac{1}{2} \delta(s-1) \delta(m-1) \quad (19)$$

$$-m \frac{B_m^s}{a} + \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{u=-\infty}^{\infty} \times (+i)^q (-i)^{q_1} e^{i(q+q_1)\gamma_1 + i(p+p_1)\gamma_3} F(n, m, p, q, p_1, q_1, p+q+u) B_n^u = \frac{1}{2} \delta(s-1) \delta(m-1)$$

where $u = s - q - q_1 - p - p_1$, and $\delta(s-1) = 1$ if $s=1$ and $\delta(s-1) = 0$ otherwise. Similarly, we obtain for ϕ_3

$$-m \frac{A_m^s}{a} + \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{u=-\infty}^{\infty} \times (-i)^q (+i)^{q_1} e^{i(q+q_1)\gamma_1 + i(p+p_1)\gamma_3} F(n, m, p, q, p_1, q_1, p+q+u) A_n^u = -\frac{i}{2} \delta(s-1) \delta(m-1) \quad (20)$$

$$-m \frac{B_m^s}{a} + \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!(m-1)!} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{u=-\infty}^{\infty} \times (+i)^q (-i)^{q_1} e^{i(q+q_1)\gamma_1 + i(p+p_1)\gamma_3} F(n, m, p, q, p_1, q_1, p+q+u) B_n^u = -\frac{i}{2} \delta(s-1) \delta(m-1)$$

The comparison of Eq. (19) with Eq. (20) yields

$$A_m^s(3) = iA_m^s(1); B_m^s(3) = -iB_m^s(1) \quad (21)$$

where j in $A_m^s(j)$ and $B_m^s(j)$ indicates that the coefficients correspond to ϕ_j .

Substituting Eqs. (19) and (20) into Eq. (17), and let $r = a$, we have

$$\phi_1 = 2 \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} [A_m^s(1) e^{im\theta} + B_m^s(1) e^{-im\theta}] e^{is\omega t} - iae^{i\omega t} \sin \theta + \phi_{c1}(t) \quad (22)$$

$$\phi_3 = 2 \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} [A_m^s(3) e^{im\theta} + B_m^s(3) e^{-im\theta}] e^{is\omega t} - iae^{i\omega t} \cos \theta + \phi_{c3}(t) \quad (23)$$

where $\phi_{c1}(t)$ and $\phi_{c3}(t)$ are functions of time only and they arise from the term $n=0$ in Eq. (17) and can be given as

$$\phi_{c1}(t) = \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{q_1=-\infty}^{\infty} \left[\frac{a^m}{(m-1)!} (A_m^s(1) (-i)^q (+i)^{q_1} + B_m^s(1) (+i)^q (-i)^{q_1}) \times \exp(ip\alpha_3 + iq\alpha_1 + is\omega t + iq_1\alpha_1 + ip_1\alpha_3) F(m, 0, p, q, p_1, q_1, p+q+s) \right] \quad (24)$$

$$\phi_{c3}(t) = \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} \sum_{p_1=-\infty}^{\infty} \sum_{q_1=-\infty}^{\infty} \left[\frac{a^m}{(m-1)!} (A_m^s(3) (-i)^q (+i)^{q_1} + B_m^s(3) (+i)^q (-i)^{q_1}) \times \exp(ip\alpha_3 + iq\alpha_1 + is\omega t + iq_1\alpha_1 + ip_1\alpha_3) F(m, 0, p, q, p_1, q_1, p+q+s) \right] \quad (25)$$

Then invoking Eq. (7), we obtain

$$\Phi = 2 \operatorname{Re} \left[\sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} (C_m^s e^{im\theta} + D_m^s e^{-im\theta}) e^{is\omega t} \right] - \omega a \eta_1 \sin \alpha_1 \sin \theta - \omega a \eta_3 \sin \alpha_3 \cos \theta + \Phi_c(t) \quad (26)$$

where

$$\begin{aligned} C_m^s &= -A_m^s(1) \omega \eta_1 e^{i\gamma_1} - A_m^s(3) \omega \eta_3 e^{i\gamma_3} = -\omega (\eta_1 e^{i\gamma_1} + i \eta_3 e^{i\gamma_3}) A_m^s(1) \\ D_m^s &= -B_m^s(1) \omega \eta_1 e^{i\gamma_1} - B_m^s(3) \omega \eta_3 e^{i\gamma_3} = -\omega (\eta_1 e^{i\gamma_1} - i \eta_3 e^{i\gamma_3}) B_m^s(1) \end{aligned} \quad (27)$$

and

$$\Phi_c(t) = -\omega \eta_1 \operatorname{Re}[\phi_{c1}(t) e^{i\gamma_1}] - \omega \eta_3 \operatorname{Re}[\phi_{c3}(t) e^{i\gamma_3}] \quad (28)$$

3.2 The hydrodynamic force

When the problem for the velocity potential has been solved, the hydrodynamic force F_j can be found by direct integration of the pressure obtained from the Bernoulli equation over the surface of the cylinder, which can be equivalently written as [18]

$$\begin{aligned} F_j &= -\frac{d}{dt} \int_{s_0} \rho \Phi n_j ds \\ &\quad - \frac{\rho}{2} \int_{s_0} \nabla(\Phi + \omega \eta_1 x \sin \alpha_1 + \omega \eta_3 z \sin \alpha_3) \cdot \nabla(\Phi + \omega \eta_1 x \sin \alpha_1 + \omega \eta_3 z \sin \alpha_3) n_j ds \end{aligned} \quad (29)$$

The substitution of Eq. (26) into Eq. (29) yields

$$F_j = \operatorname{Re} \left[\sum_{s=-\infty}^{\infty} F_j(s) e^{is\omega t} \right] \quad (30)$$

where the horizontal force components $F_1(s)$ and vertical force components $F_3(s)$ are given as follow

$$\begin{aligned} F_1(s) &= -\frac{1}{2} \rho \pi \omega^2 a^2 \eta_1 e^{is\gamma_1} [\delta(s-1) + \delta(s+1)] + \rho \pi \{ 2as\omega(-C_1^s + D_1^s) \\ &\quad + \frac{i}{a} \sum_{m=1}^{\infty} \sum_{s_1=-\infty}^{\infty} [m(m+1)(C_{m+1}^{s-s_1} D_m^{s_1} - D_{m+1}^{s-s_1} C_m^{s_1} - C_m^{s-s_1} D_{m+1}^{s_1} + D_m^{s-s_1} C_{m+1}^{s_1} \\ &\quad + C_{m+1}^{s+s_1} \bar{C}_m^{s_1} - D_{m+1}^{s+s_1} \bar{D}_m^{s_1} - C_m^{s+s_1} \bar{C}_{m+1}^{s_1} + D_m^{s+s_1} \bar{D}_{m+1}^{s_1})] \} \end{aligned} \quad (31)$$

and

$$\begin{aligned} F_3(s) &= -\frac{1}{2} \rho \pi \omega^2 a^2 \eta_3 e^{is\gamma_3} [\delta(s-1) + \delta(s+1)] + \rho \pi \{ 2ais\omega(C_1^s + D_1^s) \\ &\quad + \frac{1}{a} \sum_{m=1}^{\infty} \sum_{s_1=-\infty}^{\infty} [m(m+1)(C_{m+1}^{s-s_1} D_m^{s_1} + D_{m+1}^{s-s_1} C_m^{s_1} + C_m^{s-s_1} D_{m+1}^{s_1} + D_m^{s-s_1} C_{m+1}^{s_1} \\ &\quad + C_{m+1}^{s+s_1} \bar{C}_m^{s_1} + D_{m+1}^{s+s_1} \bar{D}_m^{s_1} + C_m^{s+s_1} \bar{C}_{m+1}^{s_1} + D_m^{s+s_1} \bar{D}_{m+1}^{s_1})] \} \end{aligned} \quad (32)$$

We may also write Eq. (30) as

$$F_j = \operatorname{Re} \{ F_j(0) + \sum_{s=1}^{\infty} [F_j(s) e^{is\omega t} + F_j(-s) e^{-is\omega t}] \} = \operatorname{Re} \{ F_j(0) + \sum_{s=1}^{\infty} [F_j(s) + \bar{F}_j(-s)] e^{is\omega t} \} \quad (33)$$

where the over-bar denotes conjugation. The non-dimensional force amplitudes $c_j(s)$ are then defined as

$$c_j(0) = \operatorname{Re}[F_j(0)] / \rho \pi \omega^2 a^2 \eta_i; \quad c_j(s) = |F_j(s) + \bar{F}_j(-s)| / \rho \pi \omega^2 a^2 \eta_i \quad \text{if } s > 0 \quad (34)$$

We notice that the horizontal steady force $F_1(0)$ can be also computed by the far field method (see appendix), which can be used to verify the formulation and solution procedure.

1 3.3 Deformation of the ice sheet

2 The deformation of the ice sheet $w(x,t)$ can be obtained by the integration of the kinematic
3 boundary condition with respect to time, or

$$4 \quad w(x,t) = \int \Phi_z \Big|_{z=0} dt \quad (35)$$

5 Through Eqs. (7), (11), (13) and (14), we have

$$6 \quad w(x,t) = \text{Re}[W(x,t)] + f(x) \quad (36)$$

7 where $f(x)$ is a time independent function and

$$8 \quad \begin{aligned} W(x,t) = & \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{iu\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} C_m^s \\ & \times \sum_{p=-\infty}^{\infty} (-i)^q e^{ip\gamma_3 + iq\alpha_1} \int_0^{\infty} [A(k) - 1] e^{-kh+ikx} k^m I_p(k\eta_3) J_q(k\eta_1) dk \\ & + \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{iu\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} D_m^s \\ & \times \sum_{p=-\infty}^{\infty} (+i)^q e^{ip\gamma_3 + iq\alpha_1} \int_0^{\infty} [A(k) - 1] e^{-kh-ikx} k^m I_p(k\eta_3) J_q(k\eta_1) dk \end{aligned} \quad (37)$$

9 with $q = u - p - s$. The summation in Eq. (37) excludes the term of $u = 0$, as this term does not
10 appear in the series of Φ_z . From the dynamic condition on the ice sheet, it can also be confirmed
11 that $f(x) = 0$.

12 Let $x \rightarrow \pm\infty$ in Eq. (37), we obtain the asymptotic expressions for the deformation of the ice sheet

$$13 \quad \begin{aligned} \lim_{x \rightarrow \pm\infty} W = & \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{u\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} C_m^s \sum_{p=-\infty}^{\infty} (-i)^q e^{ip\gamma_3 + iq\gamma_1} \\ & \times [\pm 1 - \text{sgn}(u)] \pi \frac{e^{i\lambda x} e^{-\lambda h} \lambda^m I_p(\lambda\eta_3) J_q(\lambda\eta_1) \alpha(\lambda, u)}{\beta'(\lambda, u)} \\ & + \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{u\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} D_m^s \sum_{p=-\infty}^{\infty} (+i)^q e^{ip\gamma_3 + iq\gamma_1} \\ & \times [\mp 1 - \text{sgn}(u)] \pi \frac{e^{-i\lambda x} e^{-\lambda h} \lambda^m I_p(\lambda\eta_3) J_q(\lambda\eta_1) \alpha(\lambda, u)}{\beta'(\lambda, u)} \end{aligned} \quad (38)$$

14 where $\alpha(\lambda, u)$ and $\beta(\lambda, u)$ are defined in appendix Eq. (68).

15 3.4 Some typical cases

16 Here, we will consider four typical cases, i.e. linear case, purely horizontal motion case, purely
17 vertical motion case, and circular motion case, which have been considered previously by Wu [18]
18 for the free surface condition.

19 3.4.1 Linear case

20 When the motion amplitudes are sufficiently small, we can linearize the body surface boundary
21 condition at its mean position. Thus, we can set $\eta_1 = \eta_3 = 0$ in the expansion of ϕ_j , which means
22 that we need to retain only the terms of $p = p_1 = q = q_1 = 0$ in Eqs. (19) and (20). Then we have

1 for ϕ

$$-m \frac{A_m^1}{a} + \frac{1}{(m-1)!} \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!} F(n, m, 0, 0, 0, 0, 1) A_n^1 = -\frac{1}{2} \delta(m-1) \quad (39)$$

$$B_m^s = -A_m^1$$

3 and for ϕ_3

$$-m \frac{A_m^1}{a} + \frac{1}{(m-1)!} \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!} F(n, m, 0, 0, 0, 0, 1) A_n^1 = -\frac{i}{2} \delta(m-1) \quad (40)$$

$$B_m^s = A_m^1$$

5 The above two equations suggest that ϕ_1 in Eq. (14) is antisymmetric about $\theta = 0$ or $x = 0$, and

6 ϕ_3 in Eq. (14) is symmetric about $\theta = 0$ and $F_1 = 0$ in Eq. (31).

7 Similarly, we need to retain only the terms of $p = q = 0$ and $s = 1$ for the deformation of the ice
8 sheet. Thus

$$W = \frac{e^{i\omega t}}{i\omega} \sum_{m=1}^{\infty} \frac{C_m^1 a^m}{(m-1)!} \int_0^{\infty} [A(k) - 1] e^{-kh+ikx} k^m dk + \frac{e^{i\omega t}}{i\omega} \sum_{m=1}^{\infty} \frac{D_m^1 a^m}{(m-1)!} \int_0^{\infty} [A(k) - 1] e^{-kh-ikx} k^m dk \quad (41)$$

10 and

$$\lim_{x \rightarrow +\infty} W = -2\pi \frac{e^{i\omega t}}{\omega} \frac{e^{-i\lambda x}}{\beta'(\lambda, 1)} e^{-\lambda h} \alpha(\lambda, 1) \sum_{m=1}^{\infty} \frac{D_m^1 a^m \lambda^m}{(m-1)!} \quad (42)$$

$$\lim_{x \rightarrow -\infty} W = -2\pi \frac{e^{i\omega t}}{\omega} \frac{e^{i\lambda x}}{\beta'(\lambda, 1)} e^{-\lambda h} \alpha(\lambda, 1) \sum_{m=1}^{\infty} \frac{C_m^1 a^m \lambda^m}{(m-1)!} \quad (43)$$

13 3.4.2 Purely horizontal motion

14 When $\eta_3 = 0$, we need to retain only the terms of $p = p_1 = 0$ in Eq. (19). We have

$$-m \frac{A_m^s}{a} + \frac{1}{(m-1)!} \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!} \sum_{q=-\infty}^{\infty} \sum_{u=-\infty}^{\infty} (-1)^q (+i)^{s-u} e^{i(s-u)\gamma_1} \times F(n, m, 0, q, 0, s-q-u, q+u) A_n^u = -\frac{1}{2} \delta(s-1) \delta(m-1) \quad (44)$$

$$B_m^s = (-1)^s A_m^s$$

16 These two equations means that $F_1(s) = 0$ when $s = 2n\omega$, and $F_3(s) = 0$ when $s = (2n+1)\omega$,
17 when Eqs. (27), (31) and (32) are used.

18 Similarly, we retain the terms $p = 0$ for the deformation of the ice sheet, thus we obtain

$$W = \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{iu\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} C_m^s (-i)^q e^{iq\gamma_1} \int_0^{\infty} [A(k) - 1] e^{-kh+ikx} k^m J_q(k\eta_1) dk + \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{iu\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} D_m^s (+i)^q e^{iq\gamma_1} \int_0^{\infty} [A(k) - 1] e^{-kh-ikx} k^m J_q(k\eta_1) dk \quad (45)$$

20 and

$$\begin{aligned}
\lim_{x \rightarrow \pm\infty} W &= \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{u\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} C_m^s (-i)^q e^{iq\gamma_1} \\
&\quad \times [\pm 1 - \text{sgn}(u)] \pi \frac{e^{i\lambda x} e^{-\lambda h} \lambda^m J_q(\lambda \eta_1) \alpha(\lambda, u)}{\beta'(\lambda, u)} \\
&+ \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{u\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} D_m^s (+i)^q e^{iq\gamma_1} \\
&\quad \times [\mp 1 - \text{sgn}(u)] \pi \frac{e^{-i\lambda x} e^{-\lambda h} \lambda^m J_q(\lambda \eta_1) \alpha(\lambda, u)}{\beta'(\lambda, u)}
\end{aligned} \tag{46}$$

3.4.3 Purely vertical motion

When $\eta_1 = 0$, we need to retain only the terms of $q = q_1 = 0$ in Eq. (20). We have

$$\begin{aligned}
-m \frac{A_m^s}{a} + \frac{1}{(m-1)!} \sum_{n=1}^{\infty} \frac{a^{n+m-1}}{(n-1)!} \sum_{p=-\infty}^{\infty} \sum_{u=-\infty}^{\infty} e^{i(s-u)\gamma_3} F(n, m, p, 0, s-p-u, 0, p+u) A_n^u \\
= -\frac{i}{2} \delta(s-1) \delta(m-1)
\end{aligned} \tag{47}$$

$$B_m^s = A_m^s$$

These two equations imply that ϕ_3 in Eq. (14) is symmetric about $\theta = 0$ and $F_1 = 0$ in Eq. (31).

Similarly, we retain only the terms $q = 0$ for the deformation of the ice sheet. Thus we obtain

$$\begin{aligned}
W &= \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{iu\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} C_m^s \sum_{p=-\infty}^{\infty} e^{ip\gamma_3} \int_0^{\infty} [A(k) - 1] e^{-kh+ikx} k^m I_p(k\eta_3) dk \\
&+ \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{iu\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} D_m^s \sum_{p=-\infty}^{\infty} e^{ip\gamma_3} \int_0^{\infty} [A(k) - 1] e^{-kh-ikx} k^m I_p(k\eta_3) dk
\end{aligned} \tag{48}$$

and

$$\begin{aligned}
\lim_{x \rightarrow \pm\infty} W &= \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{u\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} C_m^s \sum_{p=-\infty}^{\infty} e^{ip\gamma_3} \\
&\quad \times [\pm 1 - \text{sgn}(u)] \pi \frac{e^{i\lambda x} e^{-\lambda h} \lambda^m I_p(\lambda \eta_3) \alpha(\lambda, u)}{\beta'(\lambda, u)} \\
&+ \sum_{u=-\infty}^{\infty} \frac{e^{iu\omega t}}{u\omega} \sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} D_m^s \sum_{p=-\infty}^{\infty} e^{ip\gamma_3} \\
&\quad \times [\mp 1 - \text{sgn}(u)] \pi \frac{e^{-i\lambda x} e^{-\lambda h} \lambda^m I_p(\lambda \eta_3) \alpha(\lambda, u)}{\beta'(\lambda, u)}
\end{aligned} \tag{49}$$

3.4.4 Circular motion

When $\eta_1 = \eta_3 = \eta$ and $\gamma_3 = \gamma_1 \pm \pi/2 = \gamma$, the cylinder moves in a circular path with the centre at $(0, -h)$, where the positive sign means the motion is clockwise, while the negative sign means counterclockwise. Without losing generality, γ can be set to be zero. Here we may also adopt the special form of multipole expansions for circular motion case [18]. We notice that Eq. (6) becomes

$$\frac{\partial \Phi}{\partial r} = -\omega \eta \sin[\omega t - (\pm)\theta] \tag{50}$$

1 We may define

$$\Phi = -\omega\eta \operatorname{Re}(\phi) \quad (51)$$

3 Then the body surface boundary condition becomes

$$\partial\phi / \partial r = -ie^{i\omega t - (\pm)\theta} \quad (52)$$

5 Taking clockwise motion as an example, the multipole expansion can be given as

$$\phi = \sum_{m=1}^{\infty} \sum_{s=-\infty}^{\infty} B_m^s a^m \left[\frac{e^{-im\theta + is\omega t}}{r^m} + \frac{1}{(m-1)!} \sum_{p=0}^{\infty} \frac{\eta^p}{p!} e^{ip\omega t + is\omega t} \int_0^{\infty} A(k) k^{m+p-1} \exp(kz - kh - ikx) dk \right] \quad (53)$$

7 Invoking the upper surface condition, we have

$$A(k) = \frac{k[DK^4 - K\nu(p+s)^2 + 1] + \nu(p+s)^2}{k[DK^4 - K\nu(p+s)^2 + 1] - \nu(p+s)^2} \quad (54)$$

9 Eq. (53) is noticeably simpler than (14), which means the subsequent solution procedure is much
10 simpler. In fact, the equation for the coefficients can be obtained as

$$A_m^s = 0 - \frac{m}{a} B_m^s + \frac{1}{(m-1)!} \sum_{n=1}^{\infty} \sum_{u=-\infty}^{\infty} \frac{a^{n+m-1}}{(n-1)!} \sum_{p=0}^{\infty} \frac{\eta^{p+p_1}}{p! p_1!} F(m, n, p, p_1, p+u) B_n^u = -i\delta(s-1)\delta(m-1) \quad (55)$$

12 where $p_1 = u - s - p$.

13 For the deformation of the ice sheet, we have

$$W = -\sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} B_m^s \sum_{p=0}^{\infty} \frac{\eta^{p+1}}{p!} \frac{e^{ip\omega t + is\omega t}}{i(p+s)} \int_0^{\infty} (A(k) - 1) k^{m+p} e^{-kh - ikx} dk \quad (56)$$

15 This gives

$$\lim_{x \rightarrow \pm\infty} W = -\sum_{m=1}^{\infty} \frac{a^m}{(m-1)!} \sum_{s=-\infty}^{\infty} B_m^s \sum_{p=0}^{\infty} \frac{\eta^{p+1}}{p!} \frac{e^{ip\omega t + is\omega t}}{(p+s)} \times [\mp 1 - \operatorname{sgn}(p+s)] \pi \frac{e^{-i\lambda x} e^{-\lambda h} \lambda^{m+p} \alpha(\lambda, p+s)}{\beta'(\lambda, p+s)} \quad (57)$$

17 4. Numerical results and discussions

18 In the following numerical results and discussions, we will use dimensionless variables. The basic
19 parameters are chosen as radius of the circular cylinder a , density of the water ρ and
20 gravitational acceleration g . The other parameters are nondimensionalized as

$$h/a, D/a^4, K/a \quad (58)$$

22 It should be noted that the infinite series is truncated at a finite number, i.e. the numerical results
23 are obtained by taking $m = M$ and $s = S$, as their upper limits respectively.

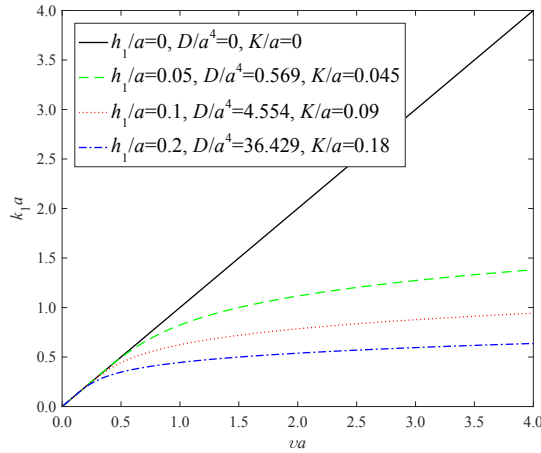
24 4.1 Analysis of the ice sheet effects on dispersion relationship

25 Wave property is very much affected by its dispersion relationship. For the free surface flow, this
26 relationship is in a simple form, especially in deep water. For the ice sheet, such a relationship in
27 Eq. (15) is more complicated. Here we take $p + q + s = 1$ as an example to conduct the analysis,
28 which corresponds to the first time harmonic component, or

$$k_1 = \nu / (DK_1^4 - K\nu + 1) \quad (59)$$

1 We can consider some typical values of ice sheet $E = 5 \times 10^9 \text{ Pa}$, $\rho_1 = 922.5 \text{ kg/m}^3$, $\nu = 0.3$ for a
 2 cylinder $a = 10 \text{ m}$ with ice thickness varying from $h_1 = 0.5 \text{ m}$ to 2 m . The other parameters are
 3 $\rho = 1025 \text{ kg/m}^3$, $g = 9.81 \text{ m/s}^2$. This gives a typical range of $h_1/a = 0.05$ to 0.2 , $D/a^4 = 0.569$
 4 to 36.429 , $K/a = 0.045$ to 0.18 , which are similar to those in Sturova [15] for the fully linear
 5 wave radiation problem.

6 Fig. 2 gives the root of the above equation $k_1 a$ against νa with different ice sheet thicknesses
 7 $h_1/a = 0.05, 0.1$ and 0.2 , together with the free surface case in which $h_1 = 0$, $D/a^4 = 0$,
 8 $K/a = 0$, and therefore $k_1 = \nu$. The figure shows when $\nu a \ll 1$, $k_1 \cong \nu$ in all cases. This can be
 9 explained by Eq. (59), in which the leading term in the denominator will be 1 when ν is small.
 10 Difference between curves increases when νa increases. At very large ν , it can be shown that
 11 $k_1 \rightarrow \sqrt[4]{K\nu/D}$ if we rewrite Eq. (59) as $\nu = (Dk_1^4 + 1)k_1 / (Kk_1 + 1)$. We can also see from the
 12 figure that k_1 decreases as the ice thickness h_1/a increases. This indicates that the wavelength of
 13 the first flexural-wave component in the ice sheet in Eq. (38) is larger than that in the free surface
 14 case. On the other hand, the wave decays more slowly along the depth when there is an ice sheet.



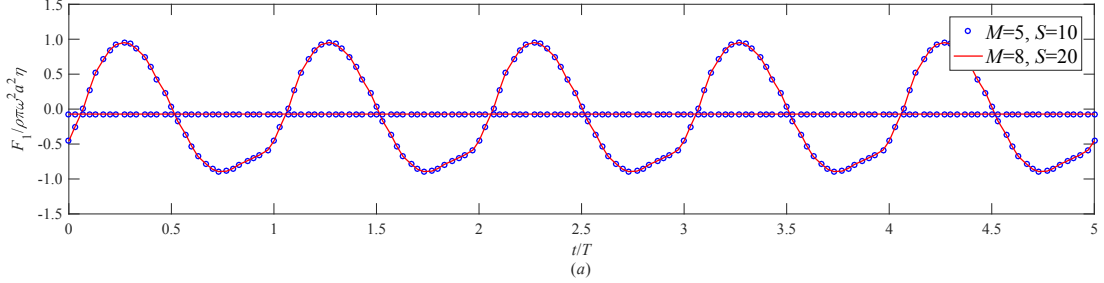
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 16 Fig. 2. Root of the dispersion Eq. (59) with different ice thicknesses.

17 4.2 Convergence study and comparison

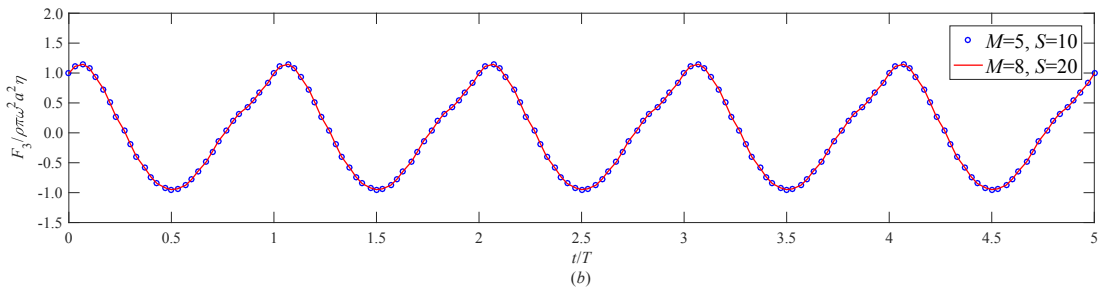
18 To conduct the convergence study with M and S , we consider the case of the cylinder
 19 undergoing clockwise circular oscillations beneath the ice sheet. A relatively large amplitude
 20 $\eta/a = 0.8$ is taken to ensure the choices M and S are also suitable for other more mild
 21 amplitudes. The other parameters are $\nu a = 2$, $h/a = 2$, $h_1/a = 0.15$, $D/a^4 = 15.368$,
 22 $K/a = 0.135$.

23 The horizontal and vertical forces with different M and S are shown in Fig. 3, while the time
 24 history of the deformation of the ice sheet at $x = 0$ as well as the wave profile at $t/T = 0.5$ are
 25 given in Fig. 4 (a) and (b) respectively. Here $T = 2\pi/\omega$ is the period of the oscillation. It can be
 26 seen that the results obtained with $M = 5$, $S = 10$ and with $M = 8$, $S = 20$ are graphically

1 undistinguishable. This means that the results have converged, and $M = 5$ and $S = 10$ will be
 2 used in the following simulations, unless otherwise specified. The horizontal steady force
 3 computed by taking the average of the force over a period and by using the far field formula is
 4 also provided. Fig. 3 (a) shows that they are in excellent agreement.

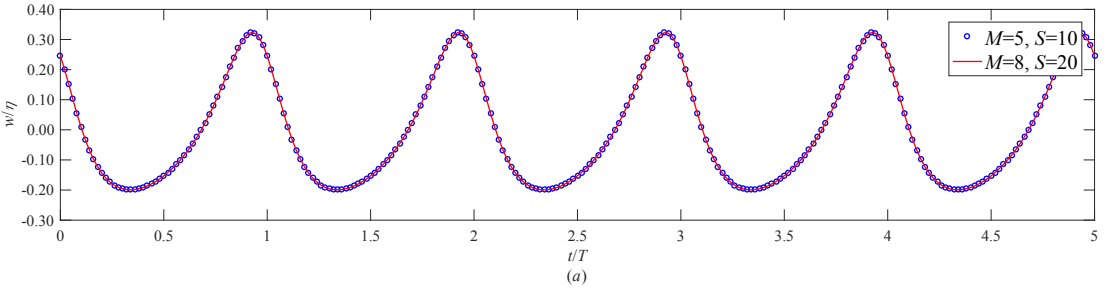


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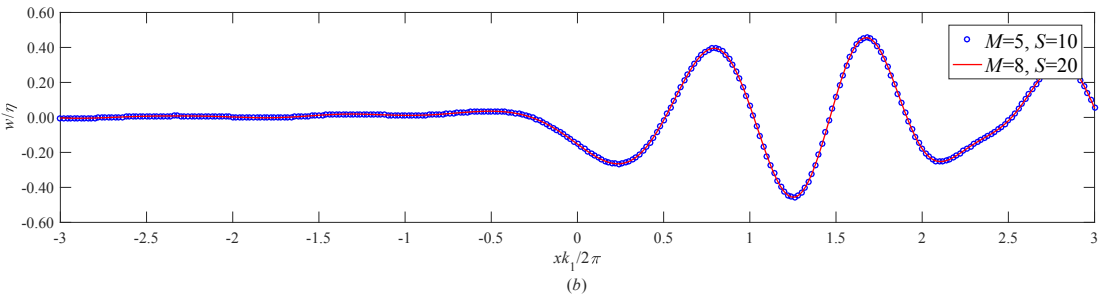


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7 Fig. 3. Hydrodynamic force with different M and S . (a) horizontal force; (b) vertical force. ($\eta / a = 0.8$;
 8 $\nu a = 2$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$). The horizontal line in (a) is the steady
 9 force which is computed by taking $M=5$ and $S=10$: open circles: by taking the average of the force over a period;
 10 solid line: by using the far field formula.



11



12

13 Fig. 4. Deformation of the ice sheet with different M and S . (a) time history of the deformation at $x = 0$; (b)
 14 profile of the deformation at $t / T = 0.5$. ($\eta / a = 0.8$; $\nu a = 2$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$,
 15 $K / a = 0.135$)

1 4.3 Purely horizontal motion

2 In this and the following sections, the wave radiation problems by a submerged horizontal circular
 3 cylinder undergoing various forced sinusoidal oscillations in water with an ice cover is studied.

4 The influence of oscillation amplitude, frequency and ice thickness on the nonlinear features of
 5 the hydrodynamic force and deformation of the ice sheet are analyzed.

6 We first consider the case of purely horizontal motions. Fig. 5 and Fig. 6 respectively show the
 7 horizontal and vertical forces at different motion amplitudes $\eta_1 / a = 0.1, 0.4$ and 0.8 respectively.

8 The other parameters are $\nu a = 0.5$, $h / a = 2$ and $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$.

9 The fully linear solutions are also provided. We notice that for the horizontal force, the nonlinear
 10 effect is small. This is can be partly explained by the fact that when the body is in such a motion,

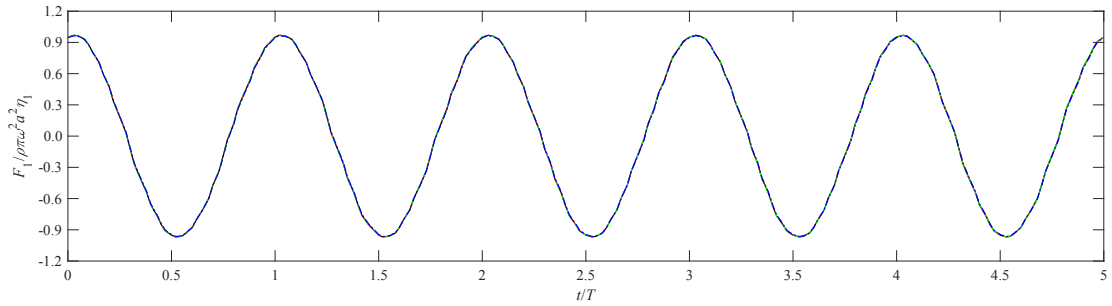
11 the horizontal force only consists of components of frequencies $(2n + 1)\omega$, while the vertical force
 12 consists of components of frequencies $2n\omega$, $n=0, 1, 2, \dots$, [20]. The horizontal force is dominated

13 by the component of ω , which is almost linearly proportional to η_1 . Thus the nonlinear effect in

14 Fig.5 is not obvious. For the same reason, we notice that the vertical force in Fig. 6 is dominated

15 by the 2ω terms and the nonlinear effect is evident, i.e. the amplitudes of both the steady
 16 component and the higher components increase with the oscillation amplitude. However, the

17 vertical force is zero for the fully linear solution.

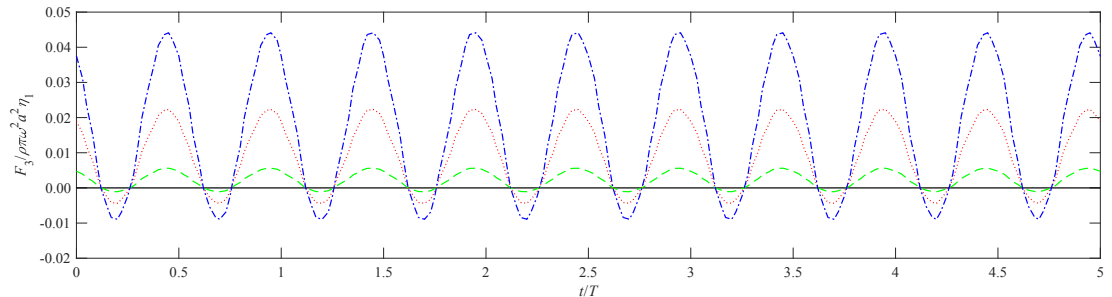


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19 Fig. 5. Horizontal force at different η_1 ($\eta_3 = 0$): solid line: fully linear solution; dashed line: $\eta_1 / a = 0.1$;

20 dotted line: $\eta_1 / a = 0.4$; dash-dotted line: $\eta_1 / a = 0.8$. ($\nu a = 0.5$; $h / a = 2$; $h_1 / a = 0.15$,

21 $D / a^4 = 15.368$, $K / a = 0.135$)



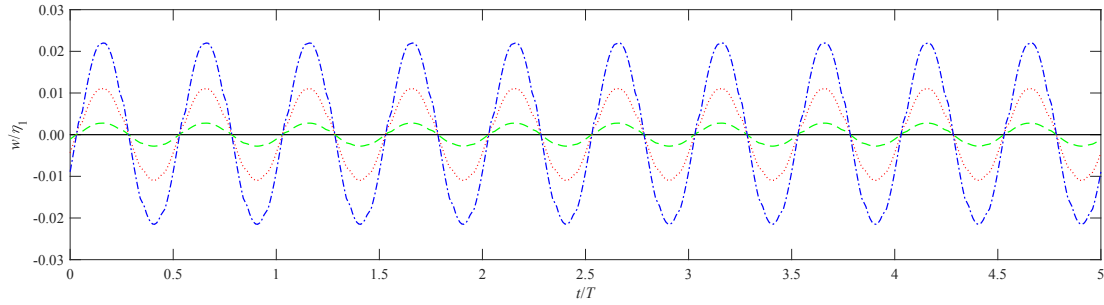
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23 Fig. 6. Vertical force at different η_1 ($\eta_3 = 0$): solid line: fully linear solution; dashed line: $\eta_1 / a = 0.1$; dotted

24 line: $\eta_1 / a = 0.4$; dash-dotted line: $\eta_1 / a = 0.8$. ($\nu a = 0.5$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$,

1 $K/a = 0.135$)

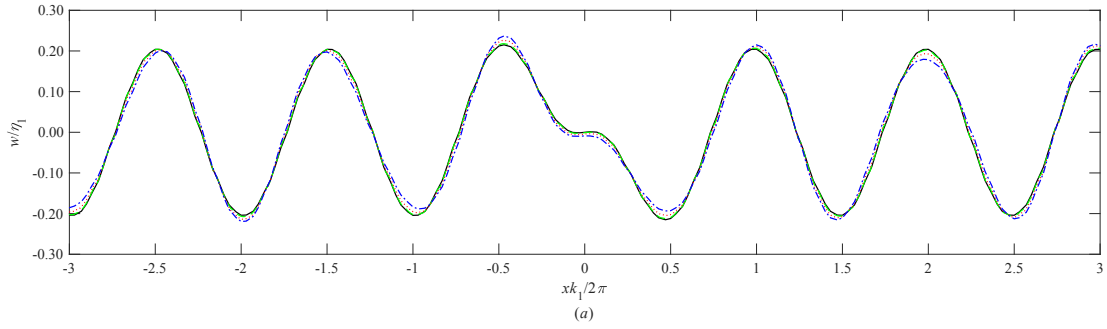
2 Fig. 7 gives the time history of the deformation of the ice sheet at $x = 0$. For the fully linear case,
 3 the problem is antisymmetric about $x=0$, or $w(-x,t) = -w(x,t)$, and therefore $w(0,t) = 0$, as
 4 shown in the figure. For the nonlinear case we notice that the oscillation has a period $T/2$, similar
 5 to the vertical force. Eqs. (27) and (44) show that in this case $D_m^s = (-1)^s C_m^s$. Substituting this
 6 and $x=0$ into Eq. (45) and noticing $q=u-s$, we find that only the terms of even u have
 7 contributions to the deformation $w(0,t)$. Thus oscillation period in Fig. 7 is $T/2$. This can also be
 8 explained based on the argument of Wu [21]. Because the body is symmetric about the vertical
 9 line passing its centre, during its periodic horizontal oscillation about $x=0$, $w(0,t)$ observed from
 10 one side (by looking into the page) is the $w(0,t+T/2)$ observed from the other side (by looking
 11 out of the page), or $w(0,t) = w(0,t+T/2)$. Such an argument is valid even when the fully
 12 nonlinear free surface boundary condition is used.



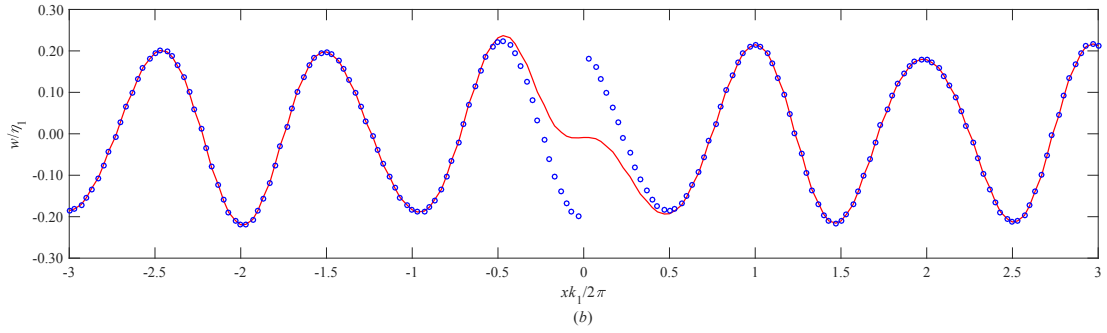
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14 Fig. 7. Time history of the deformation at $x=0$ with different η_1 ($\eta_3 = 0$): solid line: fully linear solution; dashed
 15 line: $\eta_1/a = 0.1$; dotted line: $\eta_1/a = 0.4$; dash-dotted line: $\eta_1/a = 0.8$. ($\nu a = 0.5$; $h/a = 2$;
 16 $h_1/a = 0.15$, $D/a^4 = 15.368$, $K/a = 0.135$)

17 Fig. 8 (a) shows the profiles of the deformation of the ice sheet with different η_1 . When η_1 is
 18 small, the profile of the deformation is proportional to η_1 linearly. When η_1 increases, the higher
 19 components appear and the change of the profiles becomes noticeable. The profile becomes
 20 asymmetric about $x=0$. In fact substituting $D_m^s = (-1)^s C_m^s$ into Eq. (45), we can find that the
 21 components of the profile with an even u is symmetric about $x=0$, while the components with an
 22 odd u is anti-symmetric about $x=0$. Profiles obtained from Eq.(64) and far field formula in Eq. (38)
 23 are presented in Fig. 8 (b). We can see that the local wave in Eq. (37) attenuates quite fast, and
 24 the wave tends to that obtained from Eq. (38) quickly. In fact, when $|xk_1/2\pi| > 1.0$ there is
 25 hardly any graphical difference between the two curves.



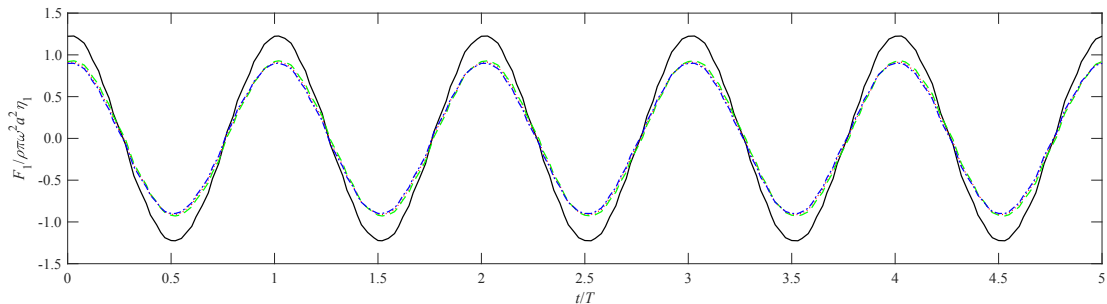
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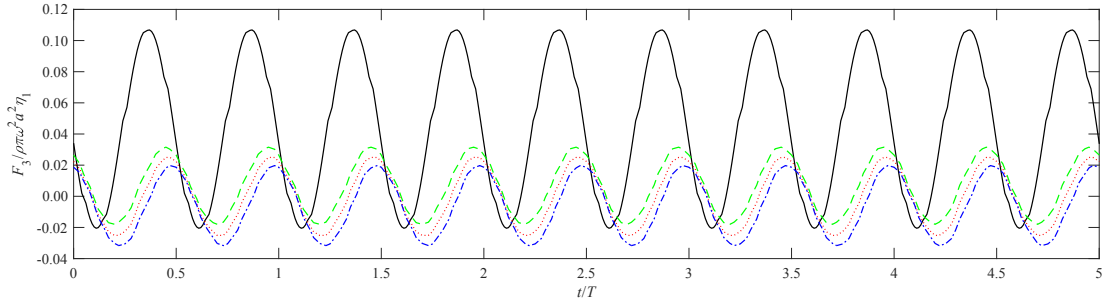
3 Fig. 8. Profile of the deformation at $t/T = 0.5$. (a) profiles with different η_1 ($\eta_3 = 0$): solid line: fully linear
 4 solution; dashed line: $\eta_1/a = 0.1$; dotted line: $\eta_1/a = 0.4$; dash-dotted line: $\eta_1/a = 0.8$; (b) profiles at
 5 $\eta_1/a = 0.8$ ($\eta_3 = 0$): solid line: Eq. (37); open circles: far field formula Eq. (38). ($\nu a = 0.5$; $h/a = 2$;
 6 $h_1/a = 0.15$, $D/a^4 = 15.368$, $K/a = 0.135$)

7 Then the hydrodynamic force and deformation of the ice sheet at different oscillation frequencies
 8 $\nu a = 0.1, 1, 2, 4$ are computed. Other parameters are taken as $\eta_1/a = 0.8$, $h/a = 2$, $h_1/a = 0.15$,
 9 $D/a^4 = 15.368$ and $K/a = 0.135$. It can be seen from Fig. 9 that the amplitude of the horizontal
 10 force which has components at $(2n+1)\omega$ only decreases when ν increases, and tends to a
 11 constant value very quickly. The vertical force which has components at $2n\omega$ only tends to a
 12 constant more slowly when ν increases, as shown in Fig. 10. The leading term in the horizontal
 13 force is ω , which is dominated by the linear contribution, while in the vertical force, the leading
 14 term is from the nonlinear contribution. From Figs. 9 and 10, it can be seen that the nonlinear
 15 contribution is more sensitive to the variation of ν . The time history of the ice sheet deformation
 16 at $x = 0$ and its profile at $t/T = 0.5$ are respectively given in Fig. 11 and Fig. 12 and these results
 17 are highly frequency dependent. It can be seen when ν is small, the deformation shape is very
 18 erratic, which shows that various components are highly significant.

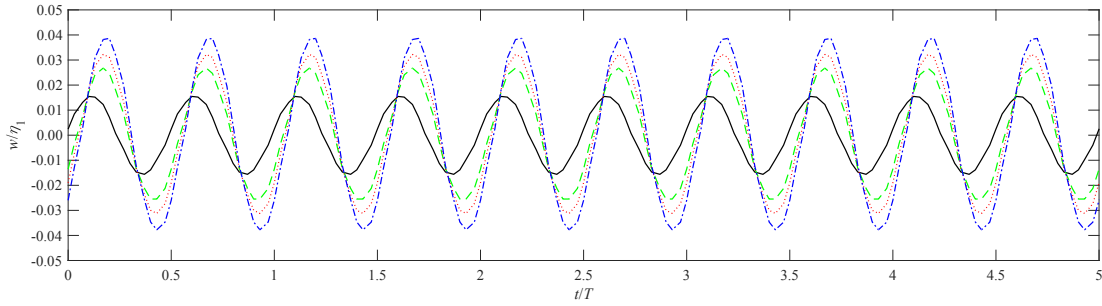


19

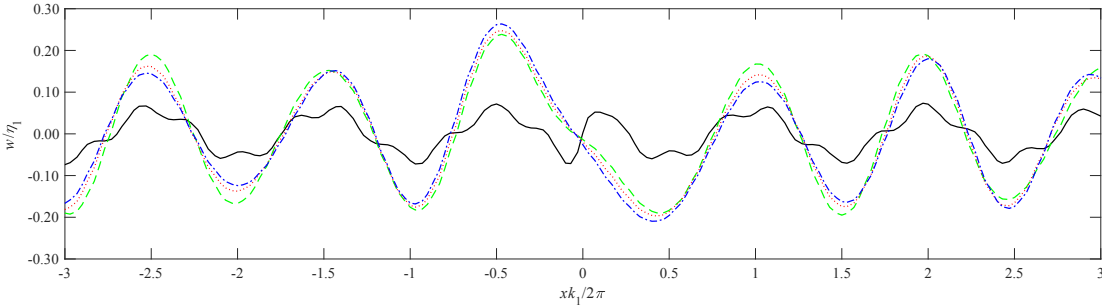
1 Fig. 9. Horizontal force at different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$; dotted line: $\nu a = 2$; dash-
 2 dotted line: $\nu a = 4$. ($\eta_1 / a = 0.8$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$)



3
 4 Fig. 10. Vertical force at different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$; dotted line: $\nu a = 2$; dash-
 5 dotted line: $\nu a = 4$. ($\eta_1 / a = 0.8$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$)



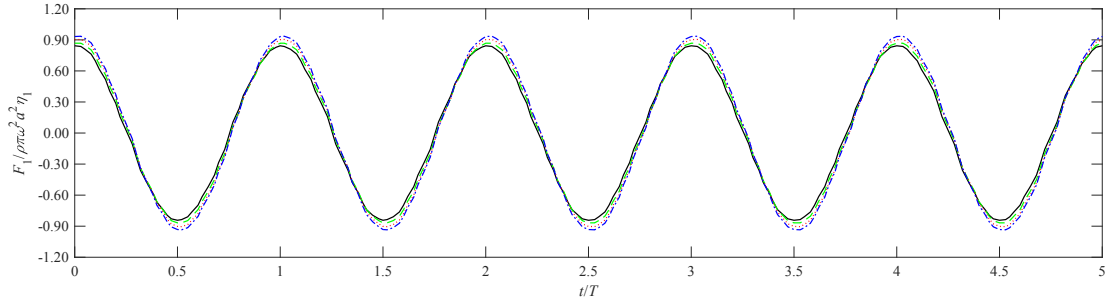
6
 7 Fig. 11. Time history of the deformation at $x = 0$ with different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$;
 8 dotted line: $\nu a = 2$; dash-dotted line: $\nu a = 4$. ($\eta_1 / a = 0.8$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$,
 9 $K / a = 0.135$)



10
 11 Fig. 12. Profile of the deformation at $t / T = 0.5$ with different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$;
 12 dotted line: $\nu a = 2$; dash-dotted line: $\nu a = 4$. ($\eta_1 / a = 0.8$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$,
 13 $K / a = 0.135$)

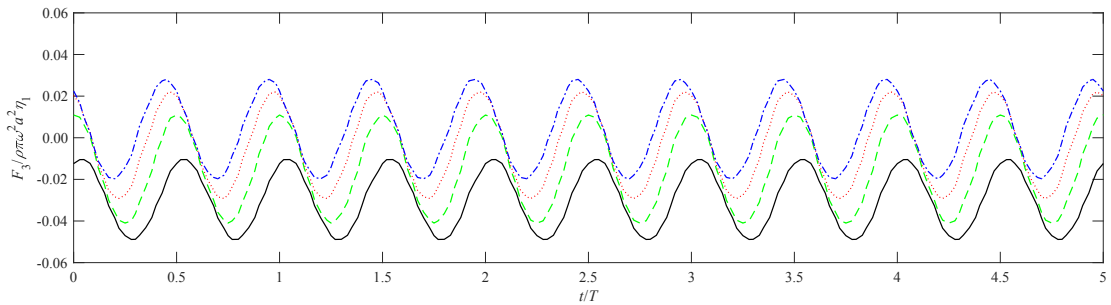
14 We undertake further simulations to investigate the effect of ice thickness. h_1 / a is taken as 0.05,
 15 0.1, 0.15 and 0.2 respectively. The other parameters are respectively chosen as $\eta_1 / a = 0.8$,
 16 $h / a = 2$ and $\nu a = 3$. It can be seen from Fig. 13 that the effect of the thickness on the horizontal
 17 force is small at this frequency. However, its effect is highly significant on the vertical force, for
 18 example the steady component increases with h_1 , as shown in Fig. 14. Fig. 15 and Fig. 16
 19 respectively show the time history and profile of the deformation of the ice sheet. We notice that

1 as h_1 increases, the ice sheet becomes more rigid and therefore its deformation becomes smaller
 2 as shown in the figures. Fig. 15 shows that the peaks and troughs at different h_1/a do not occur at
 3 the same t , which implies that the higher oscillatory components in the wave are significant. Fig.
 4 16 shows that the nonlinear effect of the body motion on the deformation of the ice sheet is most
 5 significant right above the body, and the results from different h_1 are much closer away from the
 6 body.



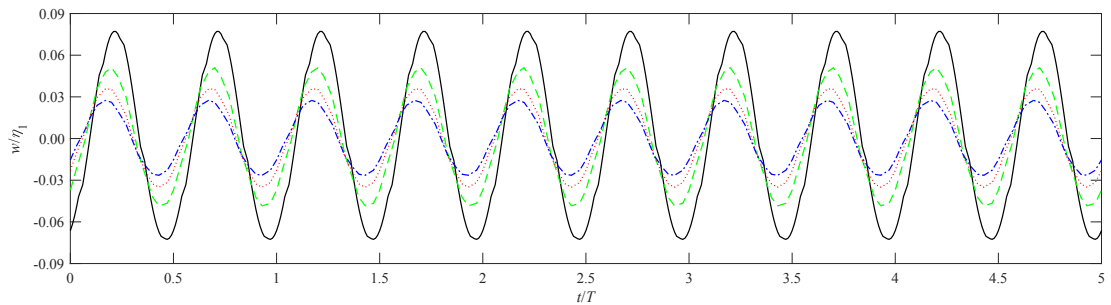
7

8 Fig. 13. Horizontal force at different h_1 : solid line: $h_1/a = 0.05$, $D/a^4 = 0.569$, $K/a = 0.045$; dashed
 9 line: $h_1/a = 0.1$, $D/a^4 = 4.554$, $K/a = 0.09$; dotted line: $h_1/a = 0.15$, $D/a^4 = 15.368$,
 10 $K/a = 0.135$; dash-dotted line: $h_1/a = 0.2$, $D/a^4 = 36.429$, $K/a = 0.18$. ($\eta_1/a = 0.8$; $va = 3$;
 11 $h/a = 2$)



12

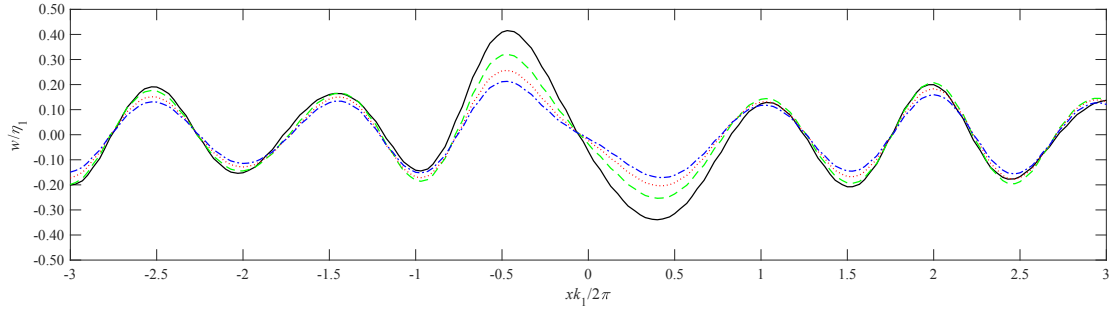
13 Fig. 14. Vertical force at different h_1 : solid line: $h_1/a = 0.05$, $D/a^4 = 0.569$, $K/a = 0.045$; dashed line:
 14 $h_1/a = 0.1$, $D/a^4 = 4.554$, $K/a = 0.09$; dotted line: $h_1/a = 0.15$, $D/a^4 = 15.368$, $K/a = 0.135$;
 15 dash-dotted line: $h_1/a = 0.2$, $D/a^4 = 36.429$, $K/a = 0.18$. ($\eta_1/a = 0.8$; $va = 3$; $h/a = 2$)



16

17 Fig. 15. Time history of the deformation at $x=0$ with different h_1 : solid line: $h_1/a = 0.05$, $D/a^4 = 0.569$,
 18 $K/a = 0.045$; dashed line: $h_1/a = 0.1$, $D/a^4 = 4.554$, $K/a = 0.09$; dotted line: $h_1/a = 0.15$,
 19 $D/a^4 = 15.368$, $K/a = 0.135$; dash-dotted line: $h_1/a = 0.2$, $D/a^4 = 36.429$, $K/a = 0.18$. ($\eta_1/a = 0.8$; $va = 3$; $h/a = 2$)

1 $\eta_1 / a = 0.8 ; \nu a = 3 ; h / a = 2$)

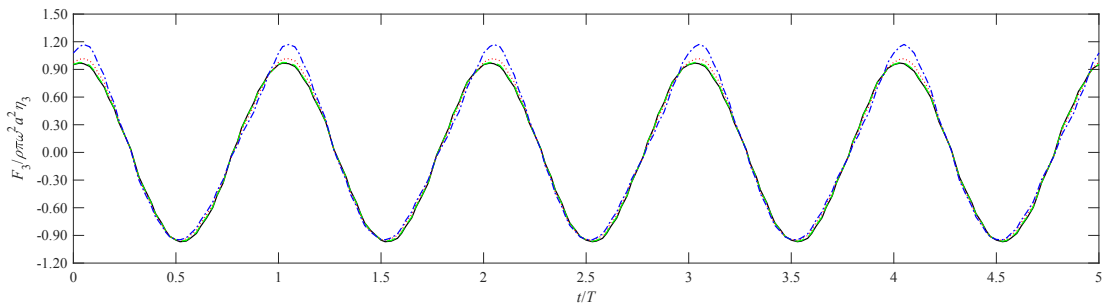


2

3 Fig. 16. Deformation of the ice sheet at $t/T = 0.5$ with different h_1 : solid line: $h_1 / a = 0.05$, $D / a^4 = 0.569$,
 4 $K / a = 0.045$; dashed line: $h_1 / a = 0.1$, $D / a^4 = 4.554$, $K / a = 0.09$; dotted line: $h_1 / a = 0.15$,
 5 $D / a^4 = 15.368$, $K / a = 0.135$; dash-dotted line: $h_1 / a = 0.2$, $D / a^4 = 36.429$, $K / a = 0.18$. (
 6 $\eta_1 / a = 0.8 ; \nu a = 3 ; h / a = 2$)

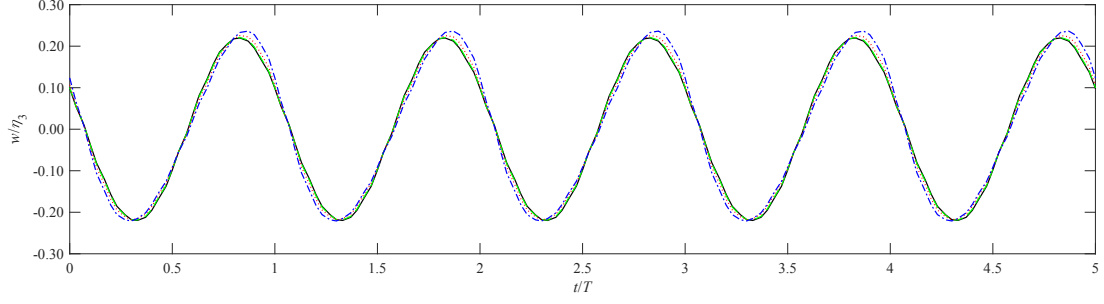
7 *4.4 Purely vertical motion*

8 Simulations are undertaken at $\nu a = 0.5$, $h / a = 2$ and $h_1 / a = 0.15$, $D / a^4 = 15.368$,
 9 $K / a = 0.135$ for $\eta_3 / a = 0.1, 0.4$ and 0.8 respectively, together with the fully linear solutions. In
 10 this case, the horizontal force is zero due to symmetry. The time histories of the vertical force is
 11 presented in Fig. 17, and the corresponding deformation of the ice sheet at $x=0$ is shown in Fig. 18.
 12 The obtained results are normalized by η_3 . Thus when η_3 is small, the curves should coincide
 13 with that obtained by the fully linear solutions, as shown in these two figures. When η_3 increases,
 14 the nonlinear effect appears. We can see that the nonlinearity is particularly strong at the crest of
 15 the force. Similar nonlinear features are also observed in the deformations in Fig. 19 (a). The
 16 deformations respectively computed by the far field formula Eq. (38) and by Eq. (37) are
 17 compared in Fig. 19 (b). It can be seen that as in the previous case the local wave attenuates fast,
 18 and when $|xk_1 / 2\pi| > 1.0$ these two curves nearly coincide with each other.



19

20 Fig. 17. Vertical force at different η_3 ($\eta_1 = 0$): solid line: fully linear solution; dashed line: $\eta_3 / a = 0.1$; dotted
 21 line: $\eta_3 / a = 0.4$; dash-dotted line: $\eta_3 / a = 0.8$. ($\nu a = 0.5 ; h / a = 2 ; h_1 / a = 0.15$, $D / a^4 = 15.368$,
 22 $K / a = 0.135$)

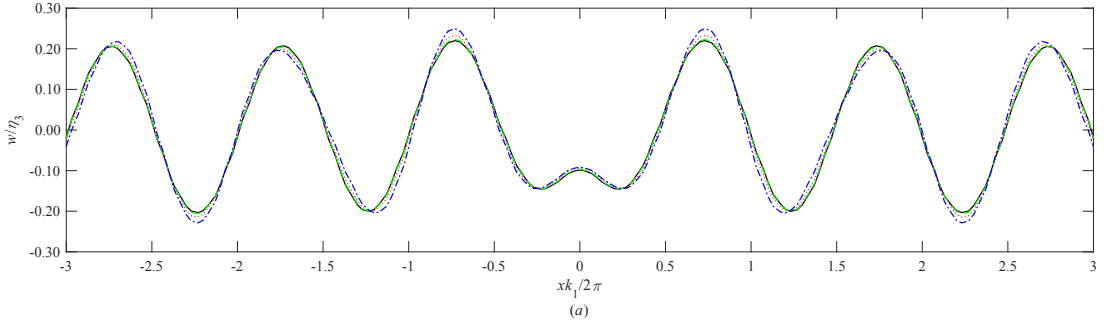


1

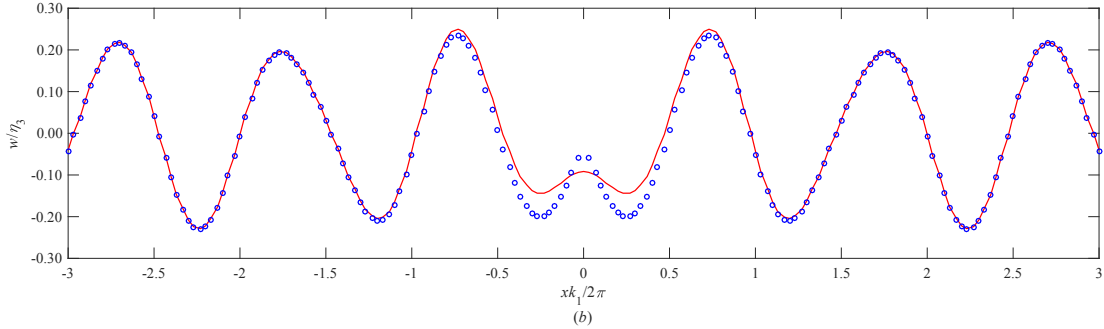
2 Fig. 18. Time history of the deformation at $x = 0$ with different η_3 ($\eta_1 = 0$): solid line: fully linear solution;

3 dashed line: $\eta_3 / a = 0.1$; dotted line: $\eta_3 / a = 0.4$; dash-dotted line: $\eta_3 / a = 0.8$. ($\nu a = 0.5$; $h / a = 2$;

4 $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$)



5



6

7 Fig. 19. Profile of the deformation at $t / T = 0.5$. (a) profiles with different η_3 ($\eta_1 = 0$): solid line: fully linear

8 solution; dashed line: $\eta_3 / a = 0.1$; dotted line: $\eta_3 / a = 0.4$; dash-dotted line: $\eta_3 / a = 0.8$; (b) profiles at

9 $\eta_3 / a = 0.8$ ($\eta_1 = 0$): solid line: Eq. (37); open circles: far field formula Eq. (38). ($\nu a = 0.5$; $h / a = 2$;

10 $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$)

11 Then the computations are carried out at different oscillation frequencies, with $\nu a = 0.1, 1, 2$, and

12 4. The other parameters are taken as $\eta_3 / a = 0.8$, $h / a = 2$ and $h_1 / a = 0.15$, $D / a^4 = 15.368$,

13 $K / a = 0.135$. The time histories of the vertical force with different ν are shown in Fig. 20. We

14 can see that the vertical force decreases fast when ν increases. As shown in Section 4.1, when ν

15 increases, the variation of wave number k_1 with ice sheet is much milder than that with free

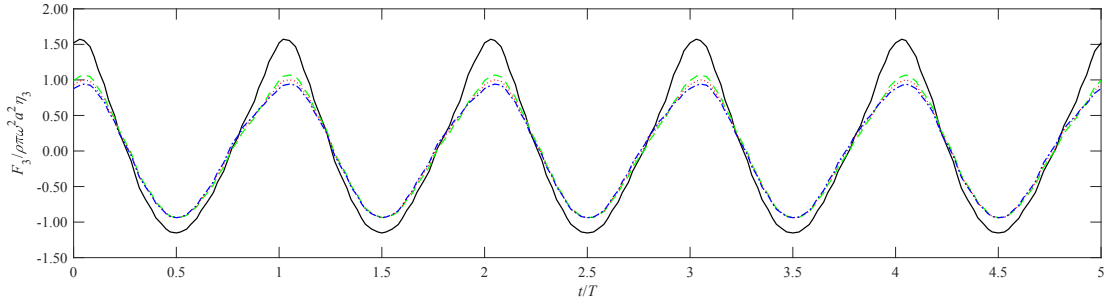
16 surface, especially at large ν . As k_1 has major effect on the result, the variation of ν on the

17 vertical force becomes small when $\nu a > 1$, as shown in Fig. 20. Similar behavior can be seen in

18 the deformation presented in Fig. 21 and Fig. 22. Compared with the purely horizontal motion, it

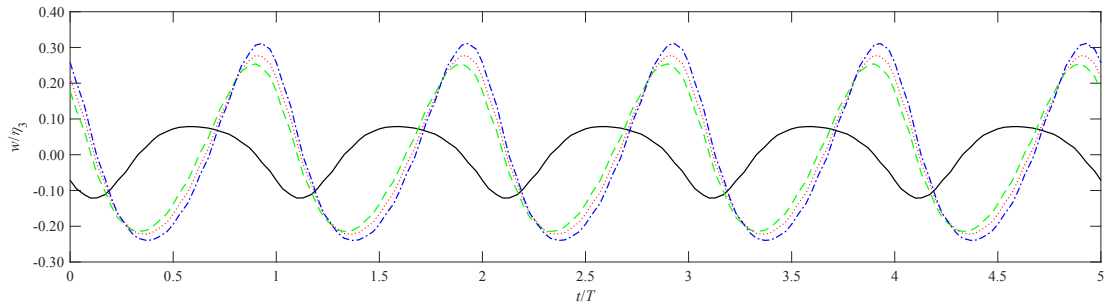
19 is important to note the distance from the centre of the cylinder to ice sheet changes during the

1 oscillation, which indicates the nonlinearity may have more significant effect.



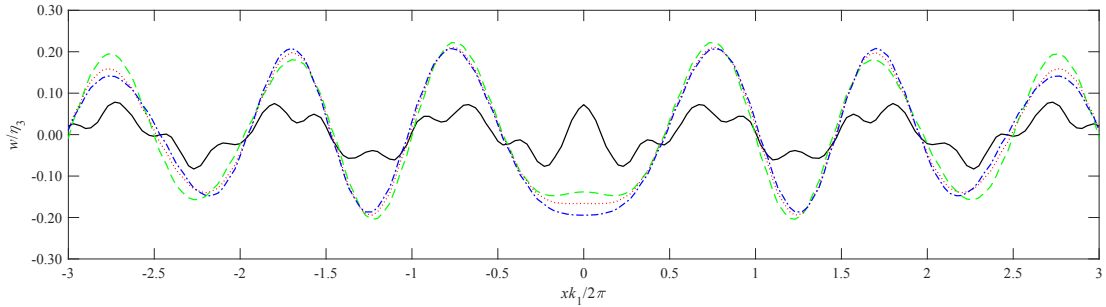
2

3 Fig. 20. Vertical force at different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$; dotted line: $\nu a = 2$; dash-
4 dotted line: $\nu a = 4$. ($\eta_3 / a = 0.8$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$)



5

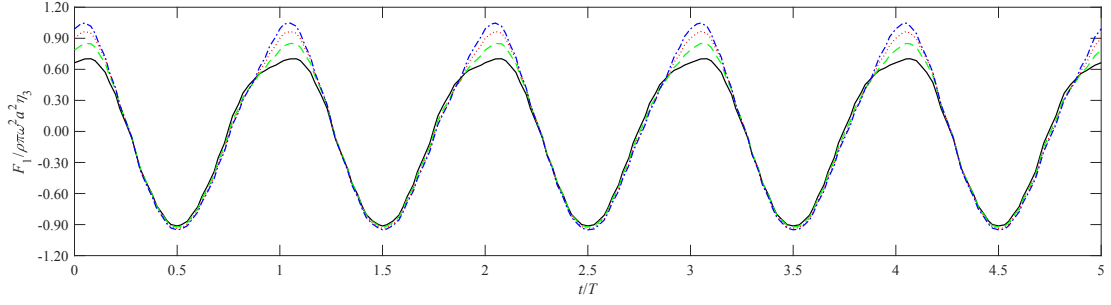
6 Fig. 21. Time history of the deformation at $x = 0$ with different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$;
7 dotted line: $\nu a = 2$; dash-dotted line: $\nu a = 4$. ($\eta_3 / a = 0.8$; $h / a = 2$; $h_1 / a = 0.15$, $D / a^4 = 15.368$,
8 $K / a = 0.135$)



9

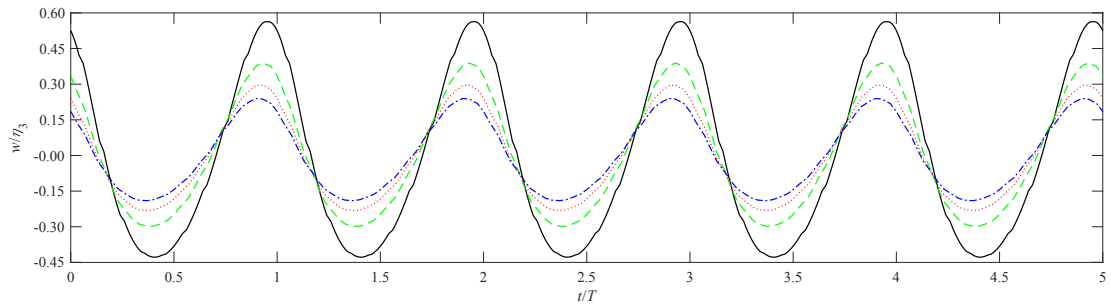
10 Fig. 22. Profile of the deformation at $t / T = 0.5$ with different ν : solid line: $\nu a = 0.1$; dashed line:
11 $\nu a = 1$; dotted line: $\nu a = 2$; dash-dotted line: $\nu a = 4$. ($\eta_3 / a = 0.8$; $h / a = 2$; $h_1 / a = 0.15$,
12 $D / a^4 = 15.368$, $K / a = 0.135$)

13 Simulations are further carried out at $h_1 / a = 0.05, 0.1, 0.15$ and 0.2 , with $\eta_3 / a = 0.8$, $h / a = 2$
14 and $\nu a = 3$. The time histories of the vertical force are shown in Fig. 23. It can be seen that h_1 has
15 a strong effect on the crest of the vertical force, while its effect on the trough is much smaller. We
16 notice in Figs. 24 and 25 that smaller h_1 leads to a larger deformation, due to the fact that the ice
17 is more flexible. As a result, vertical motion of the cylinder generates force of stronger
18 nonlinearity



1

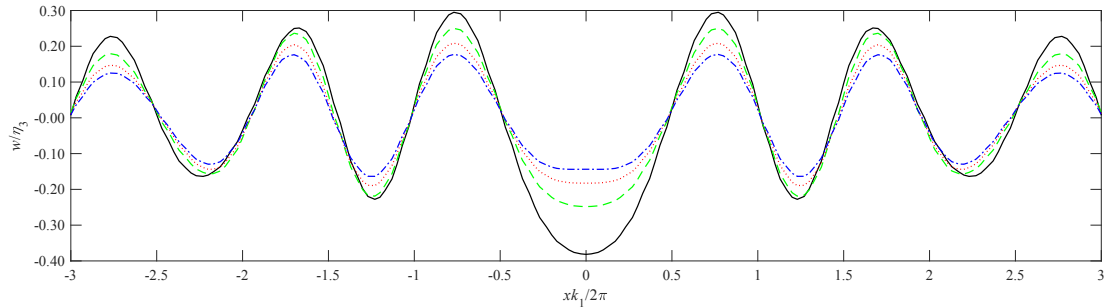
2 Fig. 23. Vertical force at different h_1 : solid line: $h_1 / a = 0.05$, $D / a^4 = 0.569$, $K / a = 0.045$; dashed line:
 3 $h_1 / a = 0.1$, $D / a^4 = 4.554$, $K / a = 0.09$; dotted line: $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$;
 4 dash-dotted line: $h_1 / a = 0.2$, $D / a^4 = 36.429$, $K / a = 0.18$. ($\eta_3 / a = 0.8$; $\nu a = 3$; $h / a = 2$)



5

6 Fig. 24. Time history of the deformation at $x=0$ with different h_1 : solid line: $h_1 / a = 0.05$, $D / a^4 = 0.569$,
 7 $K / a = 0.045$; dashed line: $h_1 / a = 0.1$, $D / a^4 = 4.554$, $K / a = 0.09$; dotted line: $h_1 / a = 0.15$,
 8 $D / a^4 = 15.368$, $K / a = 0.135$; dash-dotted line: $h_1 / a = 0.2$, $D / a^4 = 36.429$, $K / a = 0.18$. ($\eta_3 / a = 0.8$; $\nu a = 3$; $h / a = 2$)

9



10

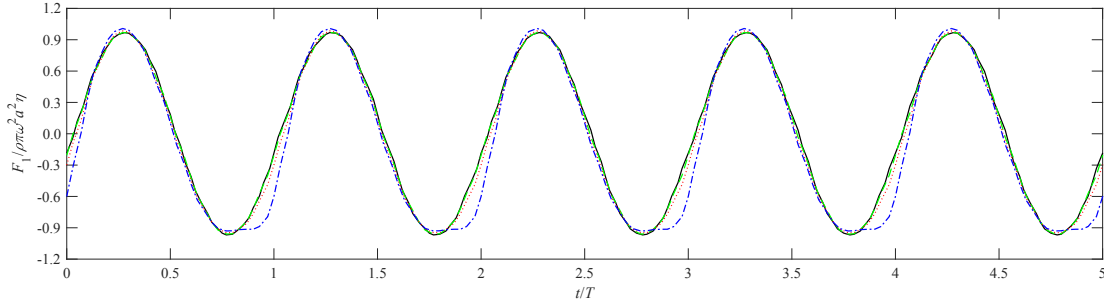
11 Fig. 25. Deformation of the ice sheet at $t/T = 0.5$ with different h_1 : solid line: $h_1 / a = 0.05$, $D / a^4 = 0.569$,
 12 $K / a = 0.045$; dashed line: $h_1 / a = 0.1$, $D / a^4 = 4.554$, $K / a = 0.09$; dotted line: $h_1 / a = 0.15$,
 13 $D / a^4 = 15.368$, $K / a = 0.135$; dash-dotted line: $h_1 / a = 0.2$, $D / a^4 = 36.429$, $K / a = 0.18$. ($\eta_3 / a = 0.8$; $\nu a = 3$; $h / a = 2$)

14

15 4.5 Circular motion

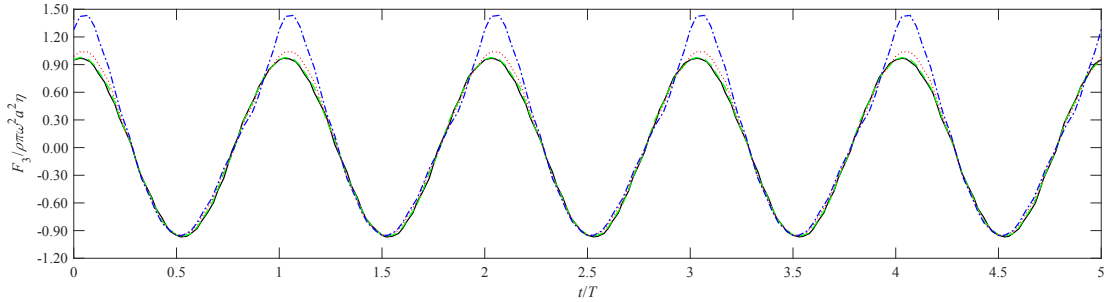
16 The hydrodynamic forces along x and y directions are respectively presented in Fig. 26 and Fig. 27
 17 for clockwise motion with three different amplitudes, at $\nu a = 0.5$, $h / a = 2$ and $h_1 / a = 0.15$,
 18 $D / a^4 = 15.368$, $K / a = 0.135$. η / a is set to be 0.1, 0.4 and 0.8 respectively. The fully linear

1 solutions are also provided for a comparison. As expected, the nonlinearity becomes important at
 2 two larger amplitudes. At $\eta/a = 0.8$, near the trough of the horizontal force, the curve becomes
 3 nearly flat, when the peak of the vertical force becomes sharper.



4

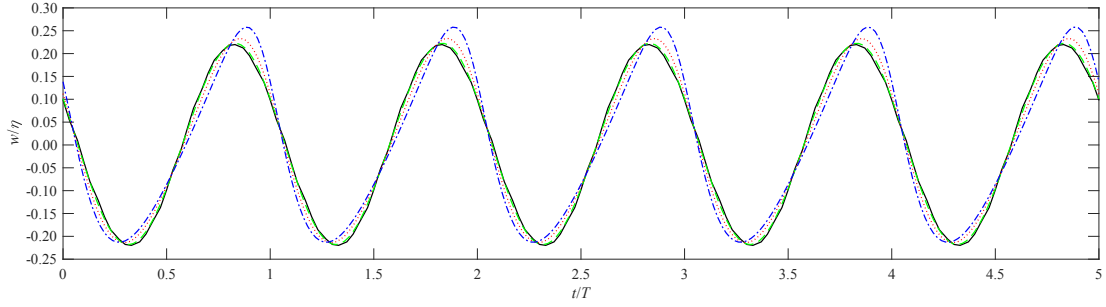
5 Fig. 26. Horizontal force at different η : solid line: fully linear solution; dashed line: $\eta/a = 0.1$; dotted line:
 6 $\eta/a = 0.4$; dash-dotted line: $\eta/a = 0.8$. ($\nu a = 0.5$; $h/a = 2$; $h_1/a = 0.15$, $D/a^4 = 15.368$,
 7 $K/a = 0.135$)



8

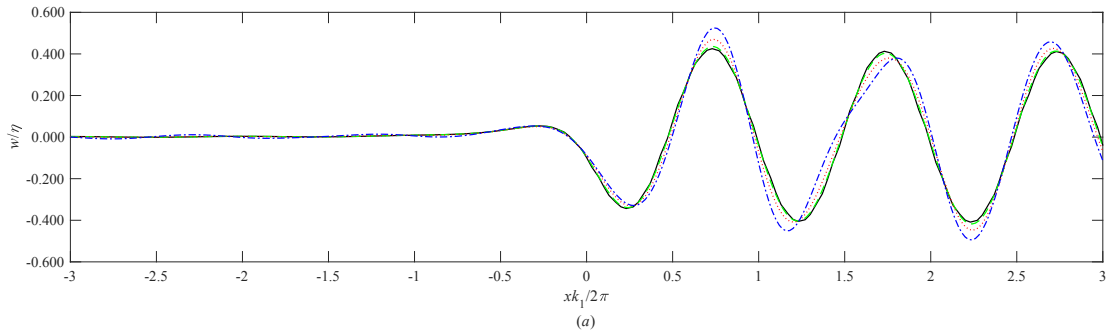
9 Fig. 27. Vertical force at different η : solid line: fully linear solution; dashed line: $\eta/a = 0.1$; dotted line:
 10 $\eta/a = 0.4$; dash-dotted line: $\eta/a = 0.8$. ($\nu a = 0.5$; $h/a = 2$; $h_1/a = 0.15$, $D/a^4 = 15.368$,
 11 $K/a = 0.135$)

12 Fig. 28 shows the corresponding time histories of the deformation at $x=0$ with different η . We
 13 can see that the effect of variation of η in circular motion is stronger than that in horizontal or
 14 vertical motion. The profiles of deformation of the ice sheet at $t/T=0.5$ are shown in Fig. 29 (a). It
 15 is interesting to see from the figure that the wave propagates mainly to the right hand side, and
 16 there is hardly any wave propagating to the left. In fact, it is known that based on the fully linear
 17 theory for the free surface flow problem, when a circular cylinder is in clockwise motion, no wave
 18 will propagate to $x = -\infty$ [22]. This conclusion can be found to be also valid for the present ice
 19 cover case. In fact this can be confirmed in Eq. (57) by taking $p=0$, $s=1$. However, when the
 20 fully nonlinear boundary condition is imposed on the body surface, Eq. (57) in section 3.5.4
 21 shows that there will be wave at $x = -\infty$ although it may be small, which is the same as the free
 22 surface problem [18]. The wave at far field on the left hand side is shown in Fig. 30. It shows that
 23 when the nonlinear effect is included there will be wave at $x = -\infty$ even though it may be small.

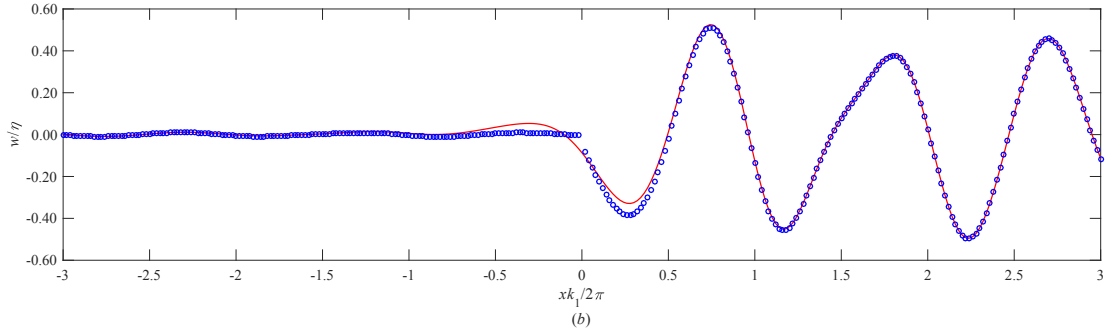


1

2 Fig. 28. Time history of the deformation at $x=0$ with different η : solid line: fully linear solution; dashed line:
 3 $\eta/a=0.1$; dotted line: $\eta/a=0.4$; dash-dotted line: $\eta/a=0.8$. ($\nu a=0.5$; $h/a=2$; $h_1/a=0.15$,
 4 $D/a^4=15.368$, $K/a=0.135$)

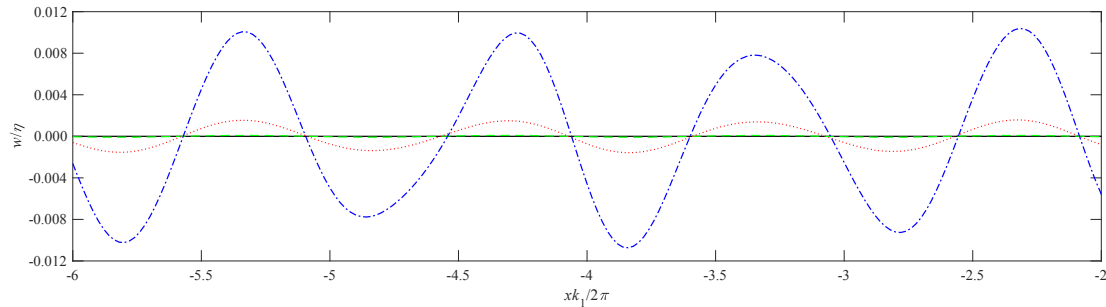


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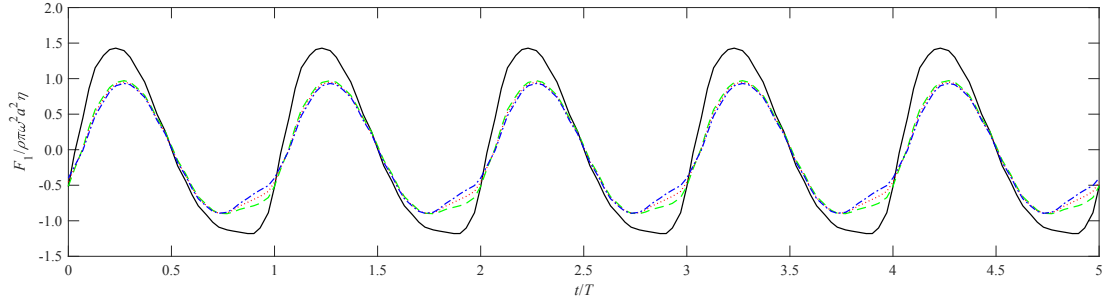
7 Fig. 29. Profile of the deformation at $t/T=0.5$. (a) profiles with different η : solid line: fully linear solution;
 8 dashed line: $\eta/a=0.1$; dotted line: $\eta/a=0.4$; dash-dotted line: $\eta/a=0.8$; (b) profiles at $\eta/a=0.8$: solid
 9 line: Eq. (37); open circles: far field formula Eq. (38). ($\nu a=0.5$; $h/a=2$; $h_1/a=0.15$, $D/a^4=15.368$,
 10 $K/a=0.135$)



11

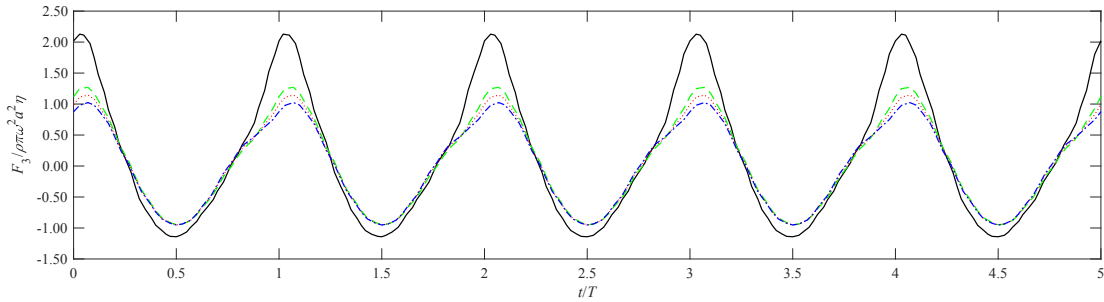
12 Fig. 30. The same as figure 29, but for the far field on the left hand side ($x<0$) only calculated by the far field
 13 formula Eq. (38).

1 Simulations are then carried out for $\nu a = 0.1, 1, 2$ and 4 respectively, with $\eta/a = 0.8$, $h/a = 2$
 2 and $h_1/a = 0.15$, $D/a^4 = 15.368$, $K/a = 0.135$. Fig. 31 and Fig. 32 respectively show the time
 3 histories of the horizontal force and vertical force. The behavior is very similar to that in the two
 4 previous cases. However, at each given ν , the curve is far less regular. The corresponding time
 5 history and profile of the deformation of the ice sheet are presented in Fig. 33 and Fig. 34
 6 respectively. It can be seen that the profile is not just dominated by the k_1 components, and other
 7 components from Eq. (15) are also important.



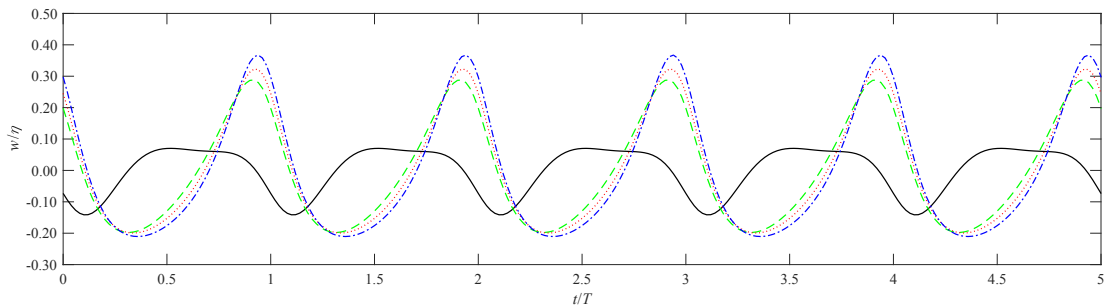
8

9 Fig. 31. Horizontal force at different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$; dotted line: $\nu a = 2$; dash-
 10 dotted line: $\nu a = 4$. ($\eta/a = 0.8$; $h/a = 2$; $h_1/a = 0.15$, $D/a^4 = 15.368$, $K/a = 0.135$)



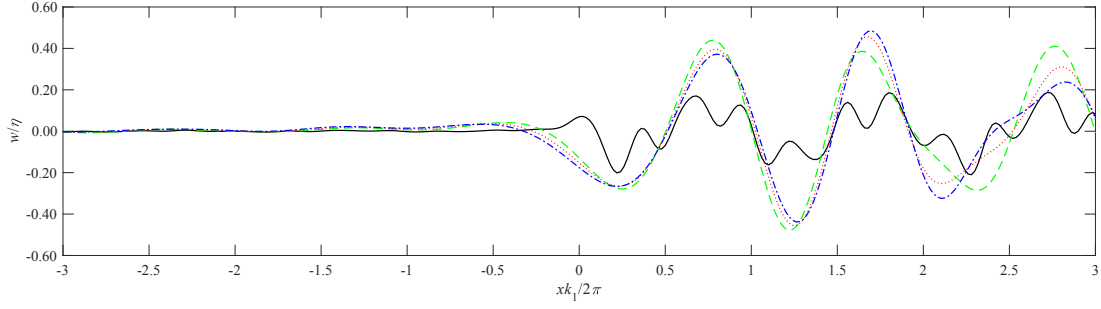
11

12 Fig. 32. Vertical force at different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$; dotted line: $\nu a = 2$; dash-
 13 dotted line: $\nu a = 4$. ($\eta/a = 0.8$; $h/a = 2$; $h_1/a = 0.15$, $D/a^4 = 15.368$, $K/a = 0.135$)



14

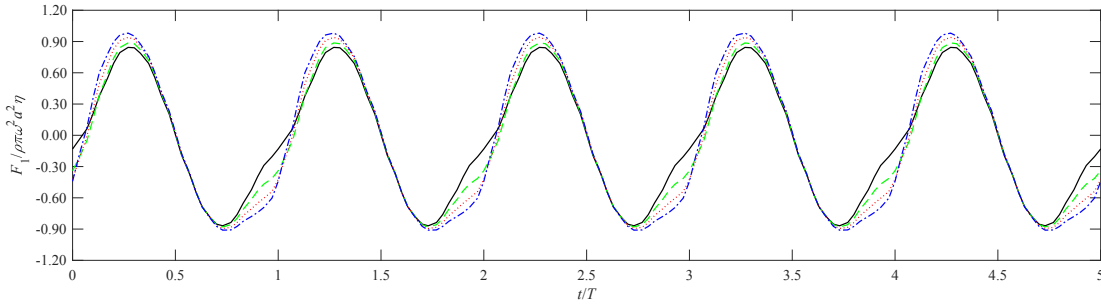
15 Fig. 33. Time history of the deformation at $x = 0$ with different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$;
 16 dotted line: $\nu a = 2$; dash-dotted line: $\nu a = 4$. ($\eta/a = 0.8$; $h/a = 2$; $h_1/a = 0.15$, $D/a^4 = 15.368$,
 17 $K/a = 0.135$)



1

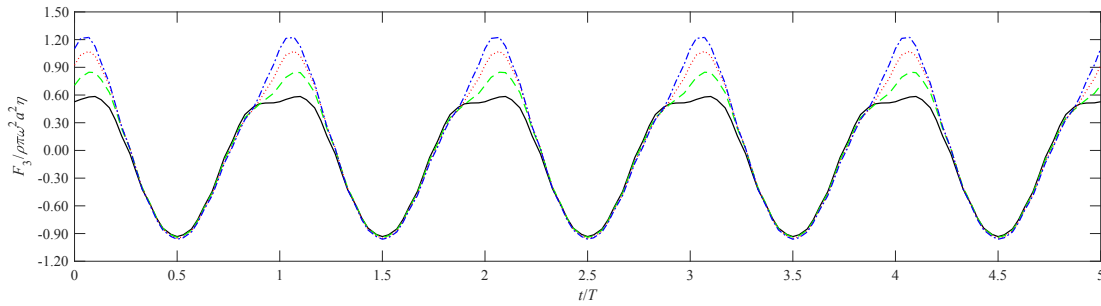
2 Fig. 34. Profile of the deformation at $t/T = 0.5$ with different ν : solid line: $\nu a = 0.1$; dashed line: $\nu a = 1$;
 3 dotted line: $\nu a = 2$; dash-dotted line: $\nu a = 4$. ($\eta/a = 0.8$; $h/a = 2$; $h_1/a = 0.15$, $D/a^4 = 15.368$,
 4 $K/a = 0.135$)

5 Forces for $h_1/a = 0.05, 0.1, 0.15$ and 0.2 at $\eta_1/a = 0.8$, $h/a = 2$ and $\nu a = 3$ are shown in Fig. 35
 6 and Fig. 36. The figures show that the thickness has bigger effect on the force than that in the
 7 previous two cases. Higher oscillatory components are obvious for small h_1/a , partly because of
 8 the larger deformation of ice sheet in Figs. 37 and 38, as the ice sheet becomes more flexible.



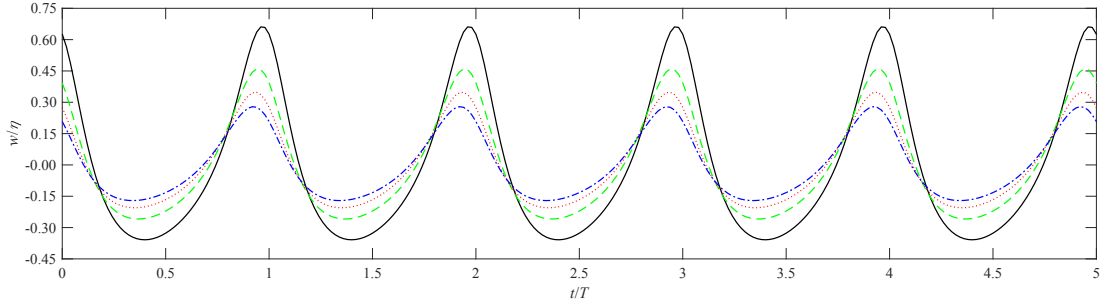
9

10 Fig. 35. Horizontal force at different h_1 : solid line: $h_1/a = 0.05$, $D/a^4 = 0.569$, $K/a = 0.045$; dashed
 11 line: $h_1/a = 0.1$, $D/a^4 = 4.554$, $K/a = 0.09$; dotted line: $h_1/a = 0.15$, $D/a^4 = 15.368$,
 12 $K/a = 0.135$; dash-dotted line: $h_1/a = 0.2$, $D/a^4 = 36.429$, $K/a = 0.18$. ($\eta/a = 0.8$; $\nu a = 3$;
 13 $h/a = 2$)



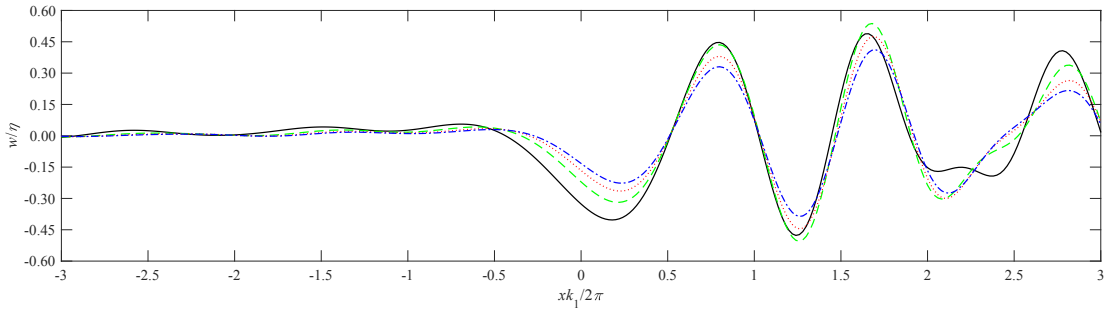
14

15 Fig. 36. Vertical force at different h_1 : solid line: $h_1/a = 0.05$, $D/a^4 = 0.569$, $K/a = 0.045$; dashed line:
 16 $h_1/a = 0.1$, $D/a^4 = 4.554$, $K/a = 0.09$; dotted line: $h_1/a = 0.15$, $D/a^4 = 15.368$, $K/a = 0.135$;
 17 dash-dotted line: $h_1/a = 0.2$, $D/a^4 = 36.429$, $K/a = 0.18$. ($\eta/a = 0.8$; $\nu a = 3$; $h/a = 2$)



1

2 Fig. 37. Time history of the deformation at $x = 0$ with different h_1 : solid line: $h_1 / a = 0.05$, $D / a^4 = 0.569$,
 3 $K / a = 0.045$; dashed line: $h_1 / a = 0.1$, $D / a^4 = 4.554$, $K / a = 0.09$; dotted line: $h_1 / a = 0.15$,
 4 $D / a^4 = 15.368$, $K / a = 0.135$; dash-dotted line: $h_1 / a = 0.2$, $D / a^4 = 36.429$, $K / a = 0.18$. (
 5 $\eta / a = 0.8$; $\nu a = 3$; $h / a = 2$)



6

7 Fig. 38. Deformation of the ice sheet at $t / T = 0.5$ with different h_1 : solid line: $h_1 / a = 0.05$,
 8 $D / a^4 = 0.569$, $K / a = 0.045$; dashed line: $h_1 / a = 0.1$, $D / a^4 = 4.554$, $K / a = 0.09$; dotted line:
 9 $h_1 / a = 0.15$, $D / a^4 = 15.368$, $K / a = 0.135$; dash-dotted line: $h_1 / a = 0.2$, $D / a^4 = 36.429$,
 10 $K / a = 0.18$. ($\eta / a = 0.8$; $\nu a = 3$; $h / a = 2$)

11 5. Conclusions

12 The multipole expansion method has been used to investigate the problem of a submerged circular
 13 cylinder undergoing large amplitude oscillations in water covered by an ice sheet. The method and
 14 solution procedure are verified by the convergence study and comparisons. Simulations are then
 15 carried out for various cases, from which the following conclusions can be drawn:

16 (1) For the purely horizontal motion, the problem is anti-symmetric when the problem is fully
 17 linearized. When fully nonlinear condition is imposed on the body surface, the problem becomes
 18 neither symmetric nor anti-symmetric. The motion contains components with frequencies of $n\omega$,
 19 $n = 1, 2, \dots$, instead of $n = 1$ as in the linear case. In the ice sheet deformation, the components are
 20 symmetric when n is an even number and anti-symmetric when n is an odd number. The
 21 horizontal force will have components of $(2n+1)\omega$ only and vertical force of $2n\omega$ only. The
 22 higher oscillatory components become more obvious when the motion amplitude η_1 increases.

23 (2) The nonlinearity effect becomes more obvious in vertical motion as the distance from the body
 24 centre to the ice sheet changes during the oscillation.

1 (3) For clockwise circular motion, when the body surface boundary condition is linearized there is
2 no wave at $x = -\infty$. When fully nonlinear boundary condition is imposed on the body surface, the
3 solution shows that there are waves at $x = -\infty$, although they may be still relatively small. The
4 effect of nonlinearity in the circular motion is more complex than that in the horizontal or vertical
5 motion.

6 (4) With some typical physical properties of the ice sheet, the solution of the dispersion
7 relationship shows that the wave number is far less sensitive to the oscillation frequency than that
8 in the free surface problem. As a result, as the oscillation frequency increases, its effect on the
9 results becomes far less obvious.

10 (5) At a given frequency when the ice thickness h_i increases, both the horizontal and vertical
11 forces become larger, while the deformation of the ice sheet becomes smaller due to the fact that
12 the ice sheet becomes more rigid.

13 The present work has also provided some accurate and reliable results for verifying numerical
14 methods for more general problems. This is particularly important here as the boundary condition
15 on the ice sheet involves the fifth derivative. When a Rankine source method is used for such a
16 problem with ice sheet of non uniform physical properties, the calculation of this high derivative
17 will be a major challenge. This challenge is already fully recognized by the well known second
18 derivative of the m-term [23] in the wave/current/body problem and some special treatments are
19 needed (e.g. Wu [24]). The present work therefore provides a useful way to verify this kind of
20 numerical method.

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28 **Appendix. Far field formula for horizontal steady force**

29 There are a large number of equations for the linear free surface flow problem, from which some
30 of the forces on the body can be obtained from the wave radiated into infinity [25]. Far field
31 equation was also derived by Wu [18] for the steady force on the body undergoing large amplitude
32 motion. For the present case, the derivation may be less straightforward because of the complexity
33 in the upper surface boundary condition.

34 The horizontal force in Eq. (29) can be given as [18]

$$\begin{aligned}
F_1 &= -\rho \frac{d}{dt} \int_{S_0} \Phi n_1 ds + \frac{\rho}{2} \int_{S_0} \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial n} - \frac{\partial^2 \Phi}{\partial n \partial x} \Phi \right) ds \\
&= -\rho \frac{d}{dt} \int_{S_0} \Phi n_1 ds - \frac{\rho}{2} \int_{S_F} \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial n} - \frac{\partial^2 \Phi}{\partial n \partial x} \Phi \right) ds - \frac{\rho}{2} \int_{S_\infty} \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial n} - \frac{\partial^2 \Phi}{\partial n \partial x} \Phi \right) ds
\end{aligned} \tag{60}$$

where S_∞ comprises two vertical lines at $x = \pm\infty$ respectively.

The steady component can be obtained from the average over the period $T = 2\pi / \omega$, or

$$F_1(0) = \frac{1}{T} \int_0^T F_1 dt = F_{1,\infty}(0) + F_{1,F}(0) \tag{61}$$

where

$$F_{1,\infty}(0) = -\frac{\rho}{2T} \int_0^T \int_{S_\infty} \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial n} - \frac{\partial^2 \Phi}{\partial n \partial x} \Phi \right) ds dt \tag{62}$$

and

$$F_{1,F}(0) = -\frac{\rho}{2T} \int_0^T \int_{S_F} \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial n} - \frac{\partial^2 \Phi}{\partial n \partial x} \Phi \right) ds dt \tag{63}$$

Let $x \rightarrow \pm\infty$ in Eq. (14) and using Eq. (13.16) of Wehausen and Laitone [26], we obtain the asymptotic expressions for the velocity potentials as

$$\lim_{x \rightarrow +\infty} \Phi = \text{Re} \left\{ \sum_{s=1}^{\infty} [f_1(s) e^{i\lambda x} e^{\lambda z} e^{-is\omega t} + f_2(s) e^{-i\lambda x} e^{\lambda z} e^{is\omega t}] \right\} \tag{64}$$

$$\lim_{x \rightarrow -\infty} \Phi = \text{Re} \left\{ \sum_{s=1}^{\infty} [g_1(s) e^{i\lambda x} e^{\lambda z} e^{is\omega t} + g_2(s) e^{-i\lambda x} e^{\lambda z} e^{-is\omega t}] \right\} \tag{65}$$

where

$$f_1(s) = \sum_{m=1}^{\infty} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} C_m^{-s-p-q} \frac{a^m}{(m-1)!} (-i)^q e^{ip\gamma_3 + iq\gamma_1} \left[2i\pi \frac{e^{-\lambda h} \lambda^{m-1} I_p(\lambda\eta_3) J_q(\lambda\eta_1) \alpha(\lambda, -s)}{\beta'(\lambda, -s)} \right] \tag{66}$$

$$f_2(s) = \sum_{m=1}^{\infty} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} D_m^{s-p-q} \frac{a^m}{(m-1)!} (+i)^q e^{ip\gamma_3 + iq\gamma_1} \left[(-2)i\pi \frac{e^{-\lambda h} \lambda^{m-1} I_p(\lambda\eta_3) J_q(\lambda\eta_1) \alpha(\lambda, s)}{\beta'(\lambda, s)} \right]$$

$$g_1(s) = \sum_{m=1}^{\infty} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} C_m^{s-p-q} \frac{a^m}{(m-1)!} (-i)^q e^{ip\gamma_3 + iq\gamma_1} \left[-2i\pi \frac{e^{-\lambda h} \lambda^{m-1} I_p(\lambda\eta_3) J_q(\lambda\eta_1) \alpha(\lambda, s)}{\beta'(\lambda, s)} \right] \tag{67}$$

$$g_2(s) = \sum_{m=1}^{\infty} \sum_{p=-\infty}^{\infty} \sum_{q=-\infty}^{\infty} D_m^{-s-p-q} \frac{a^m}{(m-1)!} (+i)^q e^{ip\gamma_3 + iq\gamma_1} \left[2i\pi \frac{e^{-\lambda h} \lambda^{m-1} I_p(\lambda\eta_3) J_q(\lambda\eta_1) \alpha(\lambda, -s)}{\beta'(\lambda, -s)} \right]$$

$$\begin{aligned}
\alpha(k, s) &= k(Dk^4 - Kvs^2 + 1) + vs^2 \\
\beta(k, s) &= k(Dk^4 - Kvs^2 + 1) - vs^2
\end{aligned} \tag{68}$$

The integration over S_∞ can be rewritten as

$$\begin{aligned}
F_{1,\infty}(0) &= -\frac{\rho}{2T} \int_0^T \int_{-\infty}^0 \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial x} - \frac{\partial^2 \Phi}{\partial x^2} \Phi \right) \Bigg|_{x \rightarrow +\infty} dz dt \\
&\quad + \frac{\rho}{2T} \int_0^T \int_{-\infty}^0 \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial x} - \frac{\partial^2 \Phi}{\partial x^2} \Phi \right) \Bigg|_{x \rightarrow -\infty} dz dt
\end{aligned} \tag{69}$$

Substituting Eqs. (64) and (65) into the above equation, we have

$$F_{1,\infty}(0) = -\frac{\rho}{4} \sum_{s=1}^{\infty} \lambda (|f(s)|^2 - |g(s)|^2) \tag{70}$$

where

$$\begin{aligned} f(s) &= \bar{f}_1(s) + f_2(s) \\ g(s) &= \bar{g}_1(s) + g_2(s) \end{aligned} \quad (71)$$

To compute the integral over S_F in Eq. (63), we notice Eqs. (7) and (10) and write the potential as

$$\Phi = \text{Re} \left(\sum_{s=-\infty}^{+\infty} \phi'_s e^{is\omega t} \right) \quad (72)$$

Invoking Eqs. (64) and (65), we have

$$\phi'_s \Big|_{+\infty} = [\delta(s+|s|)f_1(-s)e^{i\lambda x} + \delta(s-|s|)f_2(s)e^{-i\lambda x}]e^{\lambda z} \quad (73)$$

$$\phi'_s \Big|_{-\infty} = [\delta(s-|s|)g_1(s)e^{i\lambda x} + \delta(s+|s|)g_2(-s)e^{-i\lambda x}]e^{\lambda z} \quad (74)$$

where the function $\delta(s)$ has been defined below Eq. (19). Eq. (72) can be also written as

$$\Phi = \sum_{s=-\infty}^{+\infty} \phi_s e^{is\omega t} \quad (75)$$

where

$$\phi_s = \frac{1}{2}(\phi'_s + \bar{\phi}'_{-s}) \quad (76)$$

From Eqs. (73), (74) and (76), we have

$$\begin{aligned} \phi_s \Big|_{+\infty} &= \frac{1}{2} \{ [\delta(s+|s|)f_1(-s)e^{i\lambda x} + \delta(s-|s|)f_2(s)e^{-i\lambda x}] \\ &\quad + [\delta(-s+|-s|)\bar{f}_1(s)e^{-i\lambda x} + \delta(-s-|-s|)\bar{f}_2(-s)e^{i\lambda x}] \} e^{\lambda z} \end{aligned} \quad (77)$$

$$\begin{aligned} \phi_s \Big|_{-\infty} &= \frac{1}{2} \{ [\delta(s-|s|)g_1(s)e^{i\lambda x} + \delta(s+|s|)g_2(-s)e^{-i\lambda x}] \\ &\quad + [\delta(-s-|-s|)\bar{g}_1(-s)e^{-i\lambda x} + \delta(-s+|-s|)\bar{g}_2(s)e^{i\lambda x}] \} e^{\lambda z} \end{aligned} \quad (78)$$

Substitution of Eq. (75) into Eq. (5) yields

$$\phi_s = gD / (p\omega)^2 \phi_{s,xxxx} + (g / (s\omega)^2 - K)\phi_{s,z} \quad (79)$$

$$\phi_{s,x} = gD / (p\omega)^2 \phi_{s,xxxxx} + (g / (s\omega)^2 - K)\phi_{s,zx} \quad (80)$$

Substituting Eq. (75) into Eq. (63) and using the above two equations, we obtain

$$\begin{aligned} F_{1,F}(0) &= -\frac{\rho}{2T} \int_0^T dt \int_{S_F} \left(\frac{\partial \Phi}{\partial x} \frac{\partial \Phi}{\partial n} - \frac{\partial^2 \Phi}{\partial n \partial x} \Phi \right) ds \\ &= -\frac{\rho}{2T} \int_0^T dt \sum_{s=-\infty}^{+\infty} \sum_{s_1=-\infty}^{+\infty} e^{i(s+s_1)\omega t} \int_{S_F} [(gD / (s\omega)^2)(\phi_{s,xxxxx}\phi_{s_1,z} - \phi_{s,xxxx}\phi_{s_1,zx})] ds \\ &\quad - \frac{\rho}{2T} \int_0^T dt \sum_{s=-\infty}^{+\infty} \sum_{s_1=-\infty}^{+\infty} e^{i(s+s_1)\omega t} \int_{S_F} [(g / (s\omega)^2)(\phi_{s,zx}\phi_{s_1,z} - \phi_{s,z}\phi_{s_1,zx})] ds \\ &\quad + \frac{\rho}{2T} \int_0^T dt \sum_{s=-\infty}^{+\infty} \sum_{s_1=-\infty}^{+\infty} e^{i(s+s_1)\omega t} \int_{S_F} [K(\phi_{s,zx}\phi_{s_1,z} - \phi_{s,z}\phi_{s_1,zx})] ds \end{aligned} \quad (81)$$

In each integration over S_F , s and s_1 may be exchanged in the second term. Noticing the fact that only the terms $s + s_1 = 0$ have contributions to the steady force, we can conclude that only the first integration is non zero. Through integration by parts, we have

$$\begin{aligned} F_{1,F}(0) &= -\frac{\rho}{2} \sum_{s=-\infty}^{+\infty} [gD / (s\omega)^2] \\ &\quad \times [(\phi_{s,xxxx}\phi_{-s,z} - \phi_{s,xxxx}\phi_{-s,zx}) - (\phi_{s,xxxx}\phi_{-s,zx} - \phi_{s,xxx}\phi_{-s,xxx})] \Big|_{-\infty}^{+\infty} \end{aligned} \quad (82)$$

Invoking Eq. (76), we have

1
$$\phi_{-s} = \bar{\phi}_s \quad (83)$$

2 Thus Eq. (82) can be rewritten as

3
$$F_{1,F}(0) = -\rho \operatorname{Re} \left\{ \sum_{s=1}^{+\infty} [g D / (s\omega)^2] \right.$$

4
$$\left. \times (\phi_{s,zxxx} \bar{\phi}_{s,z} - 2\phi_{s,zxxx} \bar{\phi}_{s,zx} + \phi_{s,zxx} \bar{\phi}_{s,zxx}) \Big|_{-\infty}^{+\infty} \right\} \quad (84)$$

4 Substituting Eqs. (77) and (78) into Eq. (84), we obtain

5
$$F_{1,F}(0) = -\rho \sum_{s=1}^{+\infty} [g D / (s\omega)^2] \lambda^6 (|f(s)|^2 - |g(s)|^2) \quad (85)$$

6 Finally, we have

7
$$F_1(0) = -\rho \sum_{s=1}^{+\infty} \left[\frac{g D}{(s\omega)^2} \lambda^6 + \frac{1}{4} \lambda \right] [|f(s)|^2 - |g(s)|^2] \quad (86)$$

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