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# Assessment of Wind-driven Rain Impact, Related Surface Erosion and 

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Building surface erosion is a common phenomenon obserzed on historic building façades due to wind-driven rain (WDR) impact. Recently, studies on otimate thange and the effect this might have on increased extreme rainfall events has renewed the sdientric interest on determining the risk of accelerated erosive effects. Given the fact that WD ${ }^{\text {wal }}$ s on building façades is proportional to rainfall and represents the main moisture sourcharesive physical impact for building façades, an assessment method that quantifies the severity oferosion is the first step towards recommending remedial measures. The paper discusses the jor factors escalating the gradual loss of surface material, considering value, hazard, vulne bility and exposure in order to examine the WDR drop impact on the aesthetic significance and tho tructural integrity of heritage buildings, within a parametric framework. The study in tig the effects of different size water drops, with different impact speeds on a rane of asonry materials with different surface asperities and varying moisture absorption features, $\boldsymbol{y}$ yarious impact angles. For the relative quantification of the long-term surface erosion, strzty tforward and globally adaptable experiments are proposed based on site-specific climatic data anderials. Finally, strength decline of exposed sample units proves the strength-degrading effet of erosive WDR.

Keywords: Façade eosion; Kaindrop impact; Wind-driven rain; Building conservation; Historic Buildings; Building s staingbility.

## 1. Introduction. Objedetives, State-of-the-art and Methodology

Widespread nor weather events all over the world have focussed attention on the influential effects of climate change. In June 2007, in excess of 150 mm of rain fell over much of Wales, the Midlands, Northern England, Northern Ireland and parts of Scotland and South-west England. [1]. Other notable examples include the floods in central Europe in 2002, the New Orleans flood in 2005 and numerous floods in South Asia in 2007 and 2009 [2]. Heavy rain fall, flooding and strong winds or storms have had severe impact on the social, economic and cultural spheres of the country life. Although the consequences of these extreme weather events have been investigated in various sectors (i.e. agriculture, industry, energy, transport), damaging effects on the cultural heritage have insufficiently been addressed [3]. The changing magnitude of extreme weather events has
emphasised the need to review the damaging factors in surface deterioration (Fig 1) and eventually loss of integrity of heritage buildings to identify, quantify, and control the climatic effects for evaluation of remedial strategies.

The study presents novel objectives: (i) Firstly, to define a robust risk assessment framework within which the major factors escalating the loss of surface erosion are documented and classified according to value, hazard, vulnerability and exposure. (ii) Secondly, to identify th dominant factors and their implication on the aesthetic significance and structural integrity on heritage buildings. Impact behaviour examination of varying size water drops with varyj speeds and impacting angles forms the parametric framework, wherein a range of masone paterials with different surface asperities and moisture absorption features, are being comparatrely (ested. (iii) Thirdly, a straightforward and adaptable testing regime to quantify the surfacerosion due to longterm WDR impact is proposed, relating the drop impact size and duratione local rainfall characteristics. (iv) Lastly, the study aims to measure the strength declipe of exposed sample units after erosive WDR effect using a modestly destructive technique that can be usgd on site.

## 2. Evidence of Wind-driven Rain Erosion on Historic Masonry

Several researchers emphasised the adverse effects of WDR an va ious building materials. For instance, rain has been noted to be an important contributory factor to the extent of stone erosion [4]. Importantly, clean rain is considered the cause for sureerosion of building façades [5]. For instance, the effect of WDR on surface stone erosion has been investigated at the Cathedral of Learning, a tall limestone building in Pittsburgh, Pennsywania [6, 7]. Numerical modelling and field measurements of WDR loads on the façades sh red yat white, eroded areas on the building's walls corresponded to sections receiving high amounts, of WDR fluxes. For the same building, Etyemezian et. al. [8] also reported that the calculated rain fluxes on the façade of the building were reasonably consistent with the erosion patteme Morumental brick masonry is also susceptible to WDR erosion as reported for St Hubertus, n the Netherlands, where climatic conditions are similar to the UK. Numerical simulation and opsite $W$. amount of WDR on the most dampres south-west façade [9]. Surface deterioration of granite buildings in Aberdeen, Scotland, is atso wownorthy. On the highly exposed façades of some of the buildings, WDR caused mortar erostu and dampness problems and re-pointing was performed as a remedial measure for the façad [10]. Dramatically, for earth-wall buildings, the impact becomes more critical. Heathcote [11 reported that the release of the kinetic energy associated with raindrops impacting on the quildr,s façade is the main cause for the removal of material from the surfaces. Furthermore, abse ction of raindrops on cement stabilised rammed earth wall surfaces is also evident and sta力ilising, he soil with a chemical agent such as cement was noted to eliminate, to some extent, this (rawbalck [12]. Similarly, in a study performed with compressed and cementstabilised building blow Kerali (2001), the conclusion was surface erosion due to WDR varies according to the elevatjon of the block within the wall, orientation of the façade, and the age of the building (period ornosure) [13].

## 3. Factors Affecting Façade Erosion due to Raindrop Impact

Rain with a horizontal velocity component given by the wind is called wind-driven rain (WDR) [14], which is one of the main factors being responsible for surface erosion. Erosion here means the material detachment from a masonry building façade due to the physical impingement effect of WDR. Detachment occurs through long-term, continuous, repetitive and synergetic action of WDR. The loss of surface material can be quantified by a probabilistic approach to enable variation and uncertainty, and defined as a probability function of value, hazard, vulnerability and exposure. This
can be applied to a single feature on a façade (an ornament), a complete façade, a whole building or the historic building stock in a historic centre. The major contributing factors can be expressed with regard to the respective components:

$$
\begin{equation*}
P_{\text {Surface Erosion }}(\text { Loss })=P(\text { Value }) \cdot P(\text { Hazard }) \cdot P(\text { Vulnerability }) \cdot P(\text { Exposure }) \tag{1}
\end{equation*}
$$

Equation (1) provides a probabilistic-based qualitative understanding of the predo finant and independent variables determining surface erosion. Similar approach has also been used to yaluate other natural disastrous effects by other researchers [15]. This formulation can be understood by examining different locations on the façades of the southern transept of Thesbury Abbey (Fig.1). For instance, very heavy rainfalls (high hazard) on the south east facade tron exposure) causes high physical loss (as the material is vulnerable, see loosening of thesonry) but modest overall loss as there is no ornament (low value) (Fig.1a). Similarly, heayx rafalls (high hazard) will not cause loss to a well-protected (shielded) façade (low exposure) (though the material might be just as vulnerable and has greater value (Fig.1b). However, even modrate aiain events (medium hazard) will result in relatively more erosion to the moulded unp tected façade (high exposure, high value, and high vulnerability) (Fig.1c). Besides material type Oulnerability) and façade orientation with respect to prevailing wind direction (directional expostre) (Fig.1a and Fig.1c), an important factor is also the duration of any event determining tim. exposure. Each variable will now be discussed in-detail.

### 3.1. Value

By value is intended the appreciation (cultural, socis due to its authenticity and the added value of human there is no subtance loss. However, most cultural heritage presents delicate ornamentation and exquisite crâtsmanship having historical and evidential importance. Protection of cultural identity from he threat of accelerating climatic events becomes necessary. The protection of Sueno's Stane, scylptured sandstone monument dating from the end of the first millennium AD on the nop-edastrly edge of Forres, Scotland well clarifies the concept [16]. The monument was covered prefege glass to explicitly prevent further deterioration of the carved sandstone from WDP wind erosion. In an even more extreme intervention, the original statues of the west front of Wells Cathedral have been replicated and removed to avoid further loss of material and vae (17].

### 3.2. Hazard

Hazard is the erosive effect or impinging raindrops. Paramount factor for its quantification is WDR loads delivered to th wal surfaces. As an easily measurable parameter, the total rainfall is a reliable indicato of the relative WDR loads. For instance Tang et. al., (2004) measured WDR loads on a building at 61 dations together with rainfall data [6]. The conclusion was that total WDR loads on buil fagades for each storm are increasing with the increase in rainfall.
Importantly, processes of naturally occurring surface erosion are expected to be accelerated by the changes in weather patterns observed by [1], [18] and [19]. For instance, analyses of historical weather records show that all regions of the UK have experienced an increase over the past 45 years in the contribution to winter rainfall from heavy precipitation events. Severe windstorms around the UK have become more frequent in the past few decades [20]. More importantly, projections indicate that changes in mean precipitation for the 2080s under the medium emissions scenario demonstrate that in winter, precipitation increases are in the range +10 to $+30 \%$ over the majority of the country. The biggest changes (those at the $50 \%$ probability level) in precipitation in winter,
increases up to $+33 \%$, are seen along the western side of the UK [18]. The data from Meteorological Office [1] depicted in Fig. 2 shows seasonal rainfall trends between 1910 and 2010 where increased rainfall is observed over springs and autumns. The implication from a seasonal point of view is that, the building façades are subjected to WDR longer and the moisture can penetrate deep into the walls causing potential stone decay as indicated by Smith et.al. [19].

To quantify WDR loads, there are three approaches: (i), semi empirical (ii) experimen/T and (iii) numerical analysis [14]. Due to simplicity of application, semi-empirical WDR relati<ns ins have been a widely used method, employing standard weather data (wind speed, windirection and horizontal rainfall). Primarily, a frequently used relationship to predict WDR oads $R_{\text {wdr }}$ on buildings is [21, 22]:

$$
R_{w d r}=\alpha \cdot U \cdot R_{h}{ }^{0.88} \cdot \cos \beta
$$

where $\alpha_{\text {is the adapted WDR coefficient }(\mathrm{s} / \mathrm{m}) \text { which takes into considenth the site topography }}$ and the presence of building itself. In many studies, $\alpha$ has taken values between $0.02 \mathrm{~s} / \mathrm{m}$ and $0.26 \mathrm{~s} / \mathrm{m}$ depending on the building size and location on the [22]. $U$ is the reference wind speed measured at the standard meteorological height $4 \sim \mathrm{~m} / \mathrm{m}), R_{h}$ is the unobstructed rainfall intensity and $\beta$ is the wind incidence angle betwand direction and the normal to the wall surface, requiring that component of the wind velocity nomal to the wall surface is taken [14]. Critically, Equation (2) constitutes the basis for the 100 15927-3:2009 for calculation of WDR [23].

### 3.3. Vulnerability

Lack of resistance to surface erosion constiutes he vulnerability component. Historic building wall substance and surface properties are critica Eah building material generally has a characteristic range of particle bond strength [13]. The yeaker the bond is, the easier the material detachment is to occur, disrupting the bond among the castident particles. Porosity and tortuosity is another factor in terms of rendering inner layer are valnerable to atmospheric effects. Equally important, the likelihood of particle disturbance on ough wall surface induced by rain impingement is higher than on a wall of the same mar h1 with smoother surface due to the reduced frictional forces [11]. Additionally, wall construgkion sstems comprising different materials with different construction techniques can be critic shat different erosion rates of different materials could cause more complicated surface andinte fage degradation.

### 3.4. Exposure


"Exposure" accennt fot (i) ambient conditions of the building and (ii) the effect of time. Façade orientation the direction of the prevailing winds is crucial because prevailing winds drive the rain 0 n the exposed façades. Even different locations over the façade are decisive parameters for magnitude of surface degradation. Another factor is the extent of overhanging eaves, protecting wall surfaces. Also, adjacent disturbances such as trees and neighbouring buildings could provide variable shielding against wind and rain.

Time is dominant as erosion occurs over a very long time (i.e. years). Exposure can be defined as the time span considered determining perceptible erosion. Within this time frame, it is possible to determine the joint occurrence of rain and wind, blowing in a specific direction against a wall. To reflect this, Equation 2 can be integrated over the time span to calculate the total WDR load at a
specific location on the wall, taking into account the windward rain events for a particular façade orientation.

## 4. Parametric Raindrop Impact Behaviour Testing

### 4.1. Drop Impact

Drop impact on solid and liquid surfaces has been studied by many researchers as the ohemenon has been the central issue in many applications [24]. Studies have not only concentrqu on the drop impact on liquids [25], but also placed emphasis on solid and dry surfaces, xperimentally investigating the relevance of impacting factors with high resolution digital photognowy [26] and to theoretically develop quantitative models [27]. Efforts were even extended difterent specific types of materials such as wood with different density, surface properties ar water absorbability [28] or on aluminium and glass plate surfaces [29]. Three-dimensional fiyite elem, ent modelling was also performed to determine the response of deformable surfaces du to trbitrary water drop collisions [30]. However, porous building materials have interesting beenstudied far less [31,32, 33]. The main reason for this was probably the challenge of thextemely complicated flow patterns caused by the surface texture influence. Work by Riobodet. supports this since they reported six possible drop impact behaviour for dry surfaces depo ition, prompt splash, corona splash, receding break-up, partial rebound and complete Nbourd [34]. Abuku et. al. (2009) performed the first measurements of the oblique drop imparin a smooth ceramic brick surface, with drop diameters of 2 mm and 3.9 mm [35]. However, yblique drop impact has never been investigated on different porous masonry materials Wily a wider range of drop size, in a comparative fashion. This study aims to fill this gap.

### 4.2. Testing Parameters, Considerations and MEthodology

To understand the response of the diffe, nt monry materials to WDR impact, solid hand-cut historic clay bricks ( $19^{\text {th }}$ century), fired crobricks (Berkeley Red Multi), unfired clay bricks (Ecoterre), and lime mortar were sele ed th the structures laboratory of the University of Bath. The test is designed to simulate variabil/ty on card by employing a range of water drops with a range of impact speeds under varying xpure condition by changing the specimen orientation and for different vulnerability conditionsy considering different masonry materials. There are three aims: (i) The main aim is to obserathe role of different masonry materials impacted by water drops of varying diameters, impact speeds)and impact angles. This will help to form an opinion about how the energy of the water ors tends to release on the surface with respect to related behaviour. Because the raindropo are asumed to be pure liquid water, viscosity and surface tension of a raindrop are consider d conftant and thus inconsequential for this study [35]. (ii) The second aim is to see how much arater by the specimen surface after the water drop impingement depending on masonry mat rial type. During a rain event, not all of the raindrops stick to the wall surface and therefore not all frem present a source of moisture for the wall; a portion of some of the raindrops can woys away after the drops hit the surface (bouncing or splashing). WDR gauges collect all of th raindrops therefore the amount of WDR collected in WDR gauges can be different from the amount which actually acts as a moisture source. Differences bear importance for numerical analyses. (iii) The third aim is to have an idea about the role of different surface roughness and absorption features on the impact behaviour.

Drop diameters of $1.95 \mathrm{~mm}, 3.07 \mathrm{~mm}$, and 4.06 mm were considered suitable and reasonably conservative for the testing, contributing largely to the erosive energy and being also used in other similar raindrop impact studies [35]. Importantly, existing literature on the field studies of raindrop
size distribution reveals that the maximum drop size selected for the testing is not negligible considering an extreme and erosive rain event [36, 37, 38, 39]. Additionally, Best's raindrop size distribution model [40] which was established based on the rainfalls in USA, Canada and UK confirms that heavy rain events having more than $10 \mathrm{~mm} / \mathrm{hr}$ rainfall intensity present reasonable portion of raindrops with 4 mm diameter [41]. Selected drops were released at heights of 2.10 m , 3.20 m , and 4.00 m respectively through a hypodermic needle and burettes on four different types of masonry materials. To see the effect of impact angle, the masonry specimens were aced on a table with inclined surfaces at five different angles of $5^{\circ}, 15^{\circ}, 25^{\circ}, 35^{\circ}$, and $45^{\circ}$ from the raical to simulate the fact that specific wind flow patterns developed around buildings can dive rair drops onto wall surface at varied angles. The maximum release of kinetic energy occurs angle of $90^{\circ}$ (perpendicular to the wall). However, potential removal of particles can more eastr bs \%itiated by a smaller angle owing to its prising effect. Therefore, acute impact anglesconsicered, allows incorporating both of kinetic energy release and drop prising effect to initiate ursutegration. The angles were adjusted with a digital angle finder. Fig. 3 illustrates the texing set 4 . To capture the behaviour and determine the impact speed, a high speed camera (Photro - FA) TCAM-X 1280PCI 4 K ) was used. A frame rate of 2000 frames per second was used for of trie tests. A resolution of $320 \times 256$ pixels was used as it was sufficient to capture the bur of the drops. Table 1 illustrates the testing parameters with respect to the tests on four difere masonry materials, at five different angles. Fig. 4 shows a the stain from a digital camer ${ }^{2}$ nd stil frames from the high speed camera of the bouncing behaviour after some run-off of a 4.0 ve virameter droplet from a height of 4.0 m on the solid hand-cut historic clay brick at $5^{\circ}$.

For each drop impact, 2 specimens of the same mater used and the experiments of drop impact were conducted twice to ensure repeatability Malarial selection is based on bond strength, surface roughness, unit age, and material type or fundtion. Unfired clay bricks (Ecoterre) are quite weak with respect to their bond strength [42]. Rred clay bricks (Berkeley Red Multi) have a relatively rough surface. Solid hand-cut histonic clay bricks are from the Victorian period. Finally, lime mortar is one of the most common bders historic buildings. The air lime mortar used was the one found attached to the hand-cutbrich claimed from a $19^{\text {th }}$ Century building, and hence also historic.

Although the drop behaviour is c the dominant drop behaviour milliseconds, was considered as surface wettability, rout 8 , [43] [43]. The width and the efth the drop stains on each unit were measured with a calliper and recorded. A stain aspet ratio, (AR) is defined as the ratio of drop stain width to drop stain length on material surface a ter imbingement. SARs are plotted for each test, for each material and impact angle (Fig. 5). Sytmple stains are shown in Fig. 6 through which SAR value of each drop is calculated.

### 4.3. Surface Ray, ness Determination of Masonry Materials

Effects of solid material surface topography on the drop impact phenomenon have widely been investigated. Commonly, surface topography can be classified into two groups. (i) On patterned solid surface textures, either a liquid drop fills the cavities of the surface texture (collapsed state) [44] or entrap the surrounding gas inside the surface cavities (suspended state) [45]. (ii) On the surfaces with randomly shaped asperities, characterisation of the impact becomes more difficult where the surfaces are characterized by a mean surface roughness parameter. Historic masonry material surfaces generally falls in this group and roughness plays a crucial role on the outcomes of
drop impact phenomenon [26, 46, 47]. For example, Rioboo et al. experimentally studied the impact of drops onto solid dry surfaces and concluded that, roughness has an instant effect on the possibility of prompt splash behaviour [26]. They also added that the excessive number of influencing factors such as impact velocity, drop diameter, liquid viscosity, surface tension, surface wettability, wavelength and roughness generally preclude any universal correlation with any impact behaviour. If the surfaces are highly complex, having textured or porous surfaces or non-uniform surface wettability features, the characterization of the impact becomes much more rallenging [48]. Various outcomes can be observed for drop impact onto rough and porous substran whose understanding is significantly more complex than drop impacts onto simple surfan [47]. If the impact is on the inclined surfaces as in the case of WDR rain impingement, a paran ruc study with varying impact angles can best investigates the phenomenon.
Herein, four different surface roughness features of the chosen porfuraronry materials representing relative vulnerability levels facilitate the comparison of the dro impact. A laser scanner (PROSCAN 2000) with an accuracy of $1 \mu \mathrm{~m}$ (micrometre) whil being limited to a surface roughness depth of 5 mm was used for the determination of the surfase rougnness of the materials (Fig. 7). During the preliminary measurements, the laser scanner yd gured to scan the surface in steps of $20 \mu \mathrm{~m}$ for each step, with 500 steps in the x and y direct give a scanned area of 10 mm by 10 mm . This configuration was considered as a go compromise between the holistic surface roughness at a spot and the measurement time. It yas trecided to measure the surface roughness of each masonry material at five different and take the average of these roughness measurement values. The grooves on the unfired day bricks (Ecoterre) have a curved profile and the laser scanning analysis software could neya, er the data properly to take the curve into account. Thus, only the data measured in the Nire tion was taken into account because the grooves have a fairly consistent elevation in the

The roughness values $\left(R_{a}\right)$ acquired for thon
nd $y$ directions for each material and displayed in Table 2 are the arithmetic average deviation of he assessed profile [49], as calculated by the laser scanner analysis software by taking therithry, tic average of the absolute values of the roughness profile ordinates $|Z(x)|$ all over the sing ang length $l$ using Equation (3). The roughness profile for solid hand-cut historic track in Test 4 is displayed in Fig. 7.
4.4. Water Absorption Characteristics of Masonry Materials

The absorption characteristics of masonry materials having fine-pored surfaces are identified as essential parame rs sidce they govern the immediate impact behaviour, penetration of impacting water drops retention of moisture. For instance, a material with a high absorption rate will absorb and ret min mater from WDR than a material with a low absorption rate [35] or impact behaviour woul result in different outcomes such as drop stain lengths and widths [31, 48]. Therefore, the tests to determine the "Initial rate of water absorption of clay masonry units" is regarded as appropriate and performed in accordance with BS EN 772-11:2000 [50] where the principle is to dry the material to a constant mass, immerse the relevant face of the material in water for a period of time ( 60 seconds) and determine the increase in mass according to Equation (4).

$$
\begin{equation*}
c_{w i, s}=\left[\left(m_{s o, s}-m_{d p y, s}\right) / A_{s} t\right] \times 10^{3}\left[\mathrm{~kg} /\left(\mathrm{m}^{2} \times \mathrm{min}\right)\right] \tag{4}
\end{equation*}
$$

where $m_{d y, s}$ is the mass of the specimen after drying, (g); $m_{s o, s}$ is the mass of the specimen after soaking for time $t,(\mathrm{~g}) ; A_{s}$ is the gross area of the face of the specimen immersed in water, $\left(\mathrm{mm}^{2}\right) ; \mathrm{t}$ is the time in minutes, set as 1 minute for this test. Only one face of the unit is in contact with water while the others are in contact with air. The test provides an understanding of one-dimensional water absorption into a semi-infinite medium. The flow is normal to the inflow face thropghout the wetted face to which the equipotentials are parallel [51]. Fig. 8 shows the testing of shand-cut historic clay bricks. The unfired clay bricks (Ecoterre) had to be omitted from absorntion vsts as they disintegrate when immersed in water.

To simulate the processes occurring when clay masonry units are subjected arp laryounts of WDR impact for longer period of time, the water absorption test was performed with BS EN 771-1:2003 (Annex C) [52] as this takes into account better the effects micro-structural features such as amount, size and shape of pores because this standard requires greater contact to water for longer time. Oven-dried samples having dry mass, $m_{d}$ wep plateln a tank of water for 24 hours (all faces of the units were kept in contact with water), ich the wet mass of each unit was determined, $m_{w}$. The water absorption $w_{m}$ is calculated acdrding to Equation (5).
$w_{m}=\left[\left(m_{w}-m_{d}\right) / m_{d}\right] \times 100[\%]$
The initial rate of water absorption ( $c_{w i, s}$ ) and water abourtion $\left(w_{m}\right)$ test results for each masonry material are displayed in Table 3. Increasing absorption rate represents increasing vulnerability to WDR.

## 5. Surface Erosion Testing

To make practical surface erosion predictio a straightforward and globally adaptable testing method has been presented. Although indar tests have previously been conducted, they were only used to determine the suitability of soil $\downarrow$ used in adobe structures (whether there is a need for stabilisation) based on pitting deftive to the effect of water drops or water jet [53]. Therefore, parameters, considerations andmethoology are discussed for a relative assessment of surface erosion and strength degradar $Q$

### 5.1. Testing Parameters, Considerations and Methodology

Three solid hand-cut wistorif clay bricks due to their historic nature and relatively high $c_{w i, s}$ and $w_{m}$ values and three anfired clay bricks (Ecoterre) due to their low bond strength were tested in Erosion Test 1 and in tosion Test 2, respectively to quantify surface erosion and moisture retention. Vulnerable materials can be identified in different areas with a similar fashion. Burettes were used wit and without plastic nozzles attached to their tips to produce 3.07 mm and 4.06 mm diameter drops, Łespectively, knowing their high contribution to the erosive kinetic energy [54]. The burettes were placed at a height of 4.0 m above the intended points of impact on the masonry units and supported by clamps. The water amount to impact on the surface of units was calculated based on the climatic data from Boscombe Down and Lyneham between 1971 and 2000. Fig. 9 shows monthly rainfall and wind speed averages through a year. Maximum representative WDR amounts can be quantified for the erosion tests utilizing Equation (2). The main reasons for the selection of these locations are the proximity of the places to major historic city centres such as Tewkesbury and Winchester in the south-west of the UK and the ease of the accessibility and availability of the
climatic data. It is noted from Fig. 9 that in the winter and autumn months, wind speed averages increase with the increasing rainfall.

Rain intensity vectors have been determined as a resultant vector of wind speed and falling drop terminal velocity for each drop size (Fig. 10). For the horizontal component of rain intensity vector, an average wind speed value of $4.5 \mathrm{~m} / \mathrm{s}$ for Boscombe Down and Lyneham has been used based on available monthly average data between 1971 and 2000 (Fig. 9). Impact velocities of dy 6 ps falling from 4.0 m height and impact angles based on rain intensity vectors were estimated a $31.3^{\circ}$ for 3.07 mm diameter drops and $8.0 \mathrm{~m} / \mathrm{s}$ and $29.4^{\circ}$ for 4.06 mm diameter droprespectively (Fig. 10). Drop stain areas corresponding to impact angles of $31.3^{\circ}$ and $29.4^{\circ}$ were tmated based on the previous water drop behaviour testing by interpolation between the sing area values corresponding to $25^{\circ}$ and $35^{\circ}$ angles. To calculate rain amounts for an erosive fect of 5-year time, the areas are multiplied with the approximate monthly average rainfall of amr from Lyneham (similar to Boscombe Down), 12 months and 5 years (Fig. 10, Table 4). Using Equation (2) and assuming $\alpha \cdot=0.222$ and $U=4.5 \mathrm{~m} / \mathrm{s}$, erosion test water amounts to released in the form of drops on the surface of bricks was finalised as 400 ml for 3.07 mm diamert drops and 900 ml for 4.06 mm drops (Fig. 10). In different regions with different atmosph coditions, impacting water amounts and even raindrop size can be determined using region-specife climatic data and on-site measurements ensuring the developed methodology as globaradapable. Although in reality the spatial and temporal distribution of WDR is discrete and hatom, for practicality purposes, a uniform flux onto the surface can be assumed.
The bricks were oven dried at a temperature of $110^{\circ} \mathrm{C}$ tar gays (mass change was less than $0.1 \%$ for a 24 hour interval) and the dry masses measured fole erosion testing. Then, they were placed on the angled table and positioned with a digital anole finpler (Fig. 11). Two trays were placed at the foot of the bricks to collect the run-off water. Aften whe water impacted on the surface in the form of drops, wet eroded unit masses of the brickswere measured. The bricks were dried in an oven at $110^{\circ} \mathrm{C}$ for 10 days and then dry eroded pit mas were measured. Material wetting and surface erosion values are calculated according to $\mathbf{y}$ tions (6) and (7) and presented in Table 5 and 6. During measurements, material loss bhatdling the bricks is unlikely as care was taken.


Wetting $=($ Wet Eroded Unit Mas_Dry Eroded Unit Mass)/Dry Eroded Unit Mass $\times 100$ [\%]

### 5.2. Determination of Suriscestrength Degradation

The Drilling Resistanse Measurement System (DRMS) equipment is used to measure the force required to drill hrough a solid material and correlate this to the material compressive strength. The standard set-up test building masonry materials uses a 5 mm diameter diamond tipped drill bit with a flat tre The rotational speed is normally 600 rpm and the rate of penetration is $5 \mathrm{~mm} / \mathrm{min}$. More information on DRMS applications can be found elsewhere [55]. The purpose of using the DRMS maching was to measure the drilling resistance variation between the eroded and uneroded spots on the three solid hand-cut historic clay bricks and three unfired clay bricks (Ecoterre). The testing was performed after the surface erosion test to determine whether water drops on a single spot of masonry would cause a reduction in surface material strength due to their erosive impact.

Because unfired clay bricks have relatively low level of bond strength, the DRMS testing was successfully completed on all of the units. However, during the testing of three solid hand-cut historic clay bricks, DRMS drilling machine refused to drill due to the hard nature of the material
and over-resistance of aggregate constituents of the material. As a result, only one of the three units could be tested up to almost 8 mm depth. Drilling resistance measurements of unfired clay bricks for three eroded spots were taken from the eroded surface, and measurements for three uneroded spots were mostly taken from the grooves in between the ridges (Fig. 12) for each of the three units. Fig. 13 illustrates the average drilling resistance forces required for the DRMS machine with increasing depth for the unfired clay bricks tested horizontally in the erosion test subjected to both 3.07 mm and 4.06 mm diameter water drop erosive action. The only results of one of the unfired Tay bricks are also included for some comparison in Fig. 13. The solid lines represent we weraged measurements taken from uneroded spots while the dotted lines represent measury nts from the eroded spots of each set of three units.

## 6. Results and Discussion

The raindrop impact on masonry materials was examined. To correlate dpop beha) iour and compare the effects of each parameter, a stain aspect ratio (SAR) has been defifed as an indicator of drop behaviour. The ratio provides the normalisation of the measured wid ancrength of drop stain on the surface in a way that the ratio of 1 represents a circular drop stain he deviation from 1 gives information about the deviation from circular drop stain facilitating mparison of different size of drops' impact. The SAR, however, does not provide the gatatity f water adhering, penetrating or running off, which were recorded with a high speed carner tor each drop. SARs of all of the impacting water drops and their associated behaviour on masonry materials were presented in Fig. 5. Each column of SAR represents a single wate drop

### 6.1. The effect of angle and its implications

The general trend of the SAR tends to approach Tor arger impact angles and 0 for smaller impact angles (Table 7). At $25^{\circ}, 35^{\circ}$ and $45^{\circ}$ impactenges, the dominant behaviour is splashing such that $82 \%$ of the drops released in all tests spla and $12.5 \%$ of drops run off after impingement. At $15^{\circ}$ impact angle, the number of splashin drons falls to $67 \%$ with a slight increase in bouncing behaviour ( $17 \%$ ) and no change in rumin offdrops. However, at a $5^{\circ}$ impact angle, the number of splashing drops decreases dramatic Aly $\mathbf{t} 2 \%$ with a dramatic increase in the number of bouncing drops as $63 \%$. Average SAR valus tacrease with impact angle, (Table 7), which actually puts great emphasis on the role of impact mgle on the drop behaviour.

This leads to certain deant (i) One of the physical implications is that raindrops with high speeds of wind, (impacting (der larger angles) tend mostly to splash approaching a SAR of 1 while drops with low wind speeds with smaller angles are more likely to bounce approaching a SAR of 0 (Fig. 5 (ii) econdly, in terms of the effects on building fabric, splashing causes the masonry unit suffaces to hold a greater amount of water, which could cause water to penetrate into the material more a 1 y, especially for the ones with high porosity. (iii) Thirdly, as Heathcote [11] states, maxim 7 /ease of kinetic energy takes place at an impact angle of $90^{\circ}$, however, only very strong winds calrive raindrops perpendicular to the vertical wall façades. Drops striking a wall surface at around $45^{\circ}$ with SARs close to 1 , both transfer most of their kinetic energies onto the wall and erosion takes place due to the resulting prising force [11]. For instance, a drop with a SAR less than 0.3 for the $5^{\circ}$ and $15^{\circ}$ impact angles are likely to bounce or run off and thus less water is held by the surface. (iv) Additionally, bouncing drops with very low SAR can be caught by WDR gauges and measurements are used in numerical analyses but in reality they would not increase significantly the moisture content of a wall.

### 6.2. The effect of surface roughness, material moisture absorption features and drop velocity

All of the materials are considerably rough since a relatively small roughness of $R_{a} \approx 0.5 \mu \mathrm{~m}$ can be considered a threshold between smooth and rough surfaces [56]. The influence of roughness is seen in the results. Fired clay bricks (Berkley Red Multi) have the roughest surface among the four tested materials and they cause drops to splash the most except only for the drops with impact angle $5^{\circ}$ in Tests 1 and 2. Considerable roughness of lime mortar possibly causes the adhesion of pall drops ( 1.98 mm diameter) at modest impact angles. Roughness also plays a role on the spashing behaviour of drops; such that, fingers were generated and their lengths have a pry nd effect on the variation of the SAR with impact angle. For instance, in Test 4, the stains a the clay bricks for $25^{\circ}$ and $35^{\circ}$ deviated slightly from the usual trend because two fingers of the $\mathbf{v}$ ashing drops were as long as 7 mm and 9 mm respectively of 2 mm width. Similarly, the as for deviation of the SAR of 4 mm diameter drop on lime mortar at $25^{\circ}$ impact angle in Test was due to a finger 3 mm long and 2 mm wide. Given the fact that these two materials have the fighest roughness values, surface asperities initiate splashing by disturbing the spreading mell during drop impact especially the ones with high impact speeds as a generally accept behavour on rough surfaces [56]. Additionally, the relative effect of $c_{w i, s}$ and $w_{m}$ values can pkt, pring pally, be observed in the immediate impact behaviour as it takes a few millisecond to occur superficially. However, absorption features have influence to a certain degree. For . relatively high $c_{w i, s}$ and $w_{m}$ values of lime mortar caused mostly adhesion and splassibly, due to the closeness of impact speeds of the same size of drops, their impact behavyr does not present quite noticeable difference.

### 6.3. The effect of drop size

As the drop size increases, water drops te impact angle, 4.06 mm drops splash; whi 1.95 hand-cu histo bricks. From the rosity pro , more kinetic energy of drops are released during adherence and splashing then eyteing because when a drop bounces it keeps some of its kinetic energy after hitting the surace , Continuing its move with a velocity. Therefore, bigger size drops at higher impact angles ill transfer more energy on the facades by adhering, which could cause relatively more erguion over time. Furthermore, larger drops apply larger pressures on the surface. To understand the in relationships of water drep imp, They released drops with diameters of $3.31 \mathrm{~mm}, 3.83 \mathrm{~mm}$, 4.51 mm , and 5.25 mm fro the)ght of 14.0 m onto piezoelectric transducers with rise times of 2 and $5 \mu \mathrm{~s}$. They calculated dverago, vater drop impact pressures using the force measurements and an approximation of wa er drop contact area as a function of time. Results show that larger drops do not necessarily ptygestreater impact pressures however they certainly indicate that greater pressures are preduced for longer durations. Therefore larger drops are critical for exposure of surfaces. Average sures decrease to 100 kPa after $50 \mu \mathrm{~s}$. The maximum average pressure for the 5.25 mm diampter was found as high and powerful as 1.3 MPa [57]. This repetitive and longterm impact prdssure becomes critical when compressive strength of unfired clay bricks (Ecoterre) as 3.8 MPa and fired clay bricks (Berkeley Red Multi) as about 40MPa are taken into account. Shear strength values of these materials are even less.

### 6.4. Surface erosion

Permanent actions of these pressures induced by WDR drops render material erosion. For the relative quantification of erosion, and wetting, three solid hand-cut historic clay bricks and three
unfired clay bricks (Ecoterre) were tested (Table 5 and 6). An impact angle of $30^{\circ}$ is considered to be an optimum value for erosion tests based on wind and drop terminal velocity analysis (Fig. 10) being comparable with the values used in similar tests [53]. Considering impact behaviour tests, at a $30^{\circ}$ impact angle, splashing is expected for all of the drop behaviour except for the 4.06 mm diameter drops, running off after impacting on unfired clay bricks. This will ensure maximum kinetic energy release during impingement.

It is experimentally verified that rain water drop impact causes surface erosion. Table 5prents the results of Solid hand-cut historic clay brick erosion test (Test 1). Because the bond rength of the particles of the solid hand-cut historic clay bricks is very strong, very slight andon has been observed. Hence, no substantial difference was identified in terms of drop size itvuryce erosion. The very slight variation of surface erosion between brick units might be due he helerogeneity of the brick fabric and the varying strength characteristics of the units gained mor ry uring the firing process. However, the wetting impact of the water drops was profound. Agreeabry wetting values of the brick units confirm the effect of high $c_{w i, s}$ and $w_{m}$ values for Solid nd-cyt fired bricks (Table 3 and 5 ). As high as $10.4 \%$ wetting was observed during the eron test of 4.06 mm drops. The clear implication of this is the important role of water drop impacten ne pasonry wall wetting.
The results of the unfired clay bricks (Ecoterre) in Erosion Tesur sh that surface erosion values fluctuate around $1 \%$ for both 3.07 mm and 4.06 mm diamet drops, representing material loss (Table 6). However a slightly less erosion $(0.19 \%$ ) is obssved for the 4.06 mm diameter drops. This is attributed to the fact that the impact pressures the .06 mm diameter drops were higher than the 3.07 mm diameter drops [57]. This erosive offec Caused pitting on the surface and the pits acted as a drainage after a while directing water dornint the run-off water collection trays. That is, gradual erosion of the ridges of the horizonta bricks created vertical grooves and allowed the water to run off. Moreover, impact behaviour asts agreeably imply that drops on unfired clay bricks at a $30^{\circ}$ impact angle run off. Compan of the wetting of the tested materials shows that the unfired clay bricks had lower wetting lues. In conclusion, the solid hand-cut clay bricks have a higher permeability and hence retain Are w, er as opposed to the low permeability of the unfired clay bricks that prevented more water. rm ) peing absorbed into the brick. The long-term effect of moisture embodied by WDR, espe aally ofer variable temperature spells, can be deterioration. The erosion test proposed critical andglo $y$ y practical results for the assessment of different masonry materials.

The eroded areas were not yery visible on solid hand-cut historic clay bricks. However, material detachment was quite eviant on unfired clay bricks since the erosion process started when the surface became wet nough to revert back to loose clay and erode until the aggregates had been reached. Then, the r of erosion slowed down considerably. The fact that the resulting loose surface fabric may reduce the material surface strength increased the considerations of testing strength variatio thro gh eroded and uneroded areas after erosion tests. As a result, both materials were tested ung vinhs.

### 6.5. Surface strength reduction due to drop impact

The resistance of masonry units to drilling in terms of force thorough their depth are shown in Fig. 13. They generally show that for up to a depth of about 8 mm , the eroded spots show strikingly a lower drilling resistance than the uneroded spots. Even though the eroded spots are at a lower datum (aggregates present at the surface) than the uneroded spots, they consistently exhibit a lower drilling resistance at the start of the measurement due to strength degradation throughout the layers near surface. Within the first 3 mm from the surface more than $40 \%$ strength seems to be lost. Furthermore, stronger
drop impact rendered greater strength degradation. Slightly lower strength resistance of eroded spots by 4.06 mm diameter drops than by 3.07 mm diameter drops is evident in Fig. 13. This indicates that larger drops have larger impacts on the surface degradation applying larger forces for longer durations. Measurements taken from uneroded spots agree quite well, demonstrating the testing reliability. From the surface to a point of 8 mm depth, material resistance increases significantly due to the substance graining and resultant weakened bond strength of material near surface. After that point, however, resistance to drilling follows a very slightly increasing trend due to the side friction of theprobe. In addition, no strength degradation due to the water drop impact is observed and resistance the eroded and uneroded spots behave similar throughout the rest of the depth (Fig. 13).

Comparison of the testing results of hand-cut historic clay bricks and unfired clay britererre) shows that if the material bond strength is higher, the surface strength reduction decreases an consequently less erosion takes place. For example, within the first 3 mm from the surface whi the water drop impact causes considerable and consistent strength reduction on unfired clay bricls (baterre), a slight and varying reduction is seen on solid hand-cut historic clay bricks. Furthermore, he fact that fired bricks show 2.3 times more resistance than the unfired ones at the point of 7 mm deep proves vidert.

## 7. Conclusions

Widespread and severe recent flood events have raised concerns, in the in in rainfall intensities. Winters and autumns have become wetter relative to previous decades. The increase in rainfall and changes in wind speed patterns have serious implications one strface erosion of cultural heritage. Particle detachment occurs through long-term, continuows rep yitive and synergetic action of WDR. The major factors of gradual surface erosion can be clas $\sqrt{1}$ ed into four components of loss: values, hazard, vulnerability and exposure in predictions. wsty, a parametric WDR impact behaviour testing was performed where solid hand-cut hidaric clay bricks, fired clay bricks (Berkeley Red Multi), unfired clay bricks (Ecoterre), and lime mytar with various vulnerabilities were tested in order to better understand masonry material onse to varying hazard intensities employing three drop diameters, six impact speeds and fix impat angles. The behaviour is quite complicated for exact correlation. Therefore a stain aspet ran (SAR) is defined as the ratio of drop stain width to stain length. SAR tends to approach tor lor arger impact angles (strong winds) denoting splashing drops and to 0 denoting bouncing rops. gives information about the erosive effect of a drop in terms of energy release, wall surace vetting effect and potential error for WDR measurements. It was seen that the behaviour 9 ops changes depending on the drop characteristics and masonry material. The rougher the surnce, the more splashing occurred and even fingering was observed. Additionally, as the drop ©̂̀ gety bigger, water drops tend to do more splashing and run-off after striking the surface. The a straightforward and globally adaptable testing method has been introduced for an erpsion watherability assessment to evaluate the performance of three solid hand-cut historic cla brick and three unfired clay bricks (Ecoterre) for two impacting drop sizes. Surface erosion rat resurng wetting were quantified. Unfired clay bricks showed relatively much higher erosion o ving ty their low bond strength and solid hand-cut historic clay bricks were wetted more due to their permeability. The drilling resistance measurement system equipment is used to measure thf strigth variation through the eroded and uneroded spots on masonry units after erosion tests. F $y$ up to a depth of about 8 mm , the eroded spots showed a lower drilling resistance than the uneroded spots for unfired clay bricks. While the above work experimentally validates and attempts to quantify the extent of surface erosion and material strength degradation as a function of rain and material properties, it is necessary to apply this approach to a larger variety of masonry materials to be more confident in predictions and develop preventive measures for sustainability of cultural heritage.

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## Figure Captions

Fig. 1. Various locations on SE and Stacades of the southern transept of Tewkesbury Abbey
Fig. 2. Seasonal rainfall variation in the
Fig. 3. Schematic illustration of stin set-up
Fig. 3. Schematic illustration of thet-up
Fig. 4. Impact behaviour of the 06 mr diameter droplet from a height of 4.0 m on the solid handcut historic clay brick at $5^{\circ}$
Fig. 5. Stain aspect ratios $\not \subset$ wate drops on masonry material with respect to impact angle
Fig. 6. Water drop stains the jested masonry materials after impact
Fig. 7. Laser scanning for suly ace roughness determination and resultant roughness profile of solid hand-cut historic clay brick)
Fig. 8. Initial rate wormabsorption testing of solid hand-cut historic clay bricks
Fig. 9. Monthly ainfaly and wind speed averages in Boscombe Down and Lyneham between 1971 and 2000
Fig. 10. Rain nyemp vectors (RIV) and water volume representation for erosion test
Fig. 11. Brick units on angled table during erosion test
Fig. 12. Testing strength variation of eroded unfired clay brick (Ecoterre) by DRMS machine through the depth of the units
Fig. 13. Averaged DRMS data from eroded and uneroded spots of unfired clay bricks (Ecoterre) tested with 3.07 mm and 4.06 mm diameter drops and DRMS data from eroded and uneroded spots of solid hand-cut historic clay brick tested with $3.07 \mathrm{~mm} \square$ drops (only one tested unit results)

## Table Captions

Table 1 Water drop behaviour tests combination.
Table 2 Surface roughness of tested materials.
Table 3 Absorption test results of the materials.
Table 4 Interpolated stain areas.
Table 5 Erosion Test 1 - Solid hand-cut historic clay brick erosion test results Table 6 Erosion Test 2 - Unfired clay bricks (Ecoterre) erosion test results. Table 7 Number of drops in the impact behaviour tests.


Table1 Water drop behaviour tests combination.

| Parameters | Test 1 | Test 2 | Test 3 | Test 4 | Test 5 | Test 6 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Drop Diameter $(\mathrm{mm})$ | 1.95 | 3.07 | 3.07 | 4.06 | 4.06 | 4.06 |
| Falling Height $(\mathrm{m})$ | 2.10 | 2.10 | 3.20 | 2.10 | 3.20 | 4.00 |
| Drop Speed $(\mathrm{m} / \mathrm{s})$ | 5.75 | 6.00 | 6.92 | 6.06 | 7.29 | 00 |



Table 2 Surface roughness of tested materials.

| Masonry Type |  | Roughness ( $\mu \mathrm{m}$ ) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Test 1 | Test 2 | Test 3 | Test 4 | Test 5 | Direction Average | Standard Deviation | Final Average |
| Solid hand-cuthistoric clay brick (Victorian) | x | 58.30 | 48.40 | 46.40 | 47.50 | 56.00 | 51.32 |  | 50.37 |
|  | y | 48.60 | 54.20 | 45.80 | 50.60 | 47.90 | 49.42 | 3.18 |  |
| Fired clay brick (Berkeley Red Multi) | x | 272.70 | 131.40 | 280.10 | 198.80 | 335.10 | 243.62 | 79.2 | 231.88 |
|  | y | 173.70 | 200.70 | 139.70 | 316.30 | 270.30 | 220 | 2.0 |  |
| Unfired clay bricks (Ecoterre) | x | 32.50 | 29.90 | 31.50 | 28.50 | 32.40 |  | 1.73 | 13 |
|  | y | 12.40 | 14.40 | 12.20 | 12.40 | 13.60 | 13.09 | 0.96 |  |
| Lime Mortar | x | 105.60 | 105.90 | 43.10 | 70.00 | 45.10 | 18.94 | 30.91 | 76.62 |
|  | y | 106.40 | 89.00 | 49.70 | 86.10 | 653 | 79.30 | 22.06 |  |

Table 3 Absorption test results of the materials.


Table 4 Interpolated stain areas.

| Drop <br> Diameter <br> $(\mathbf{m m})$ | Fired clay bricks (Berkeley Red <br> Multi) |  | Unfired clay bricks (Ecoterre) |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Stain area <br> $\left(\mathbf{c m}^{2}\right)$ | $\mathbf{5}$ year volume <br> $\left(\mathbf{c m}^{3}\right)$ | Stain area <br> $\left(\mathbf{c m}^{2}\right)$ | $\mathbf{5}$ year volume <br> $\left(\mathbf{c m}^{\mathbf{3}}\right.$ |
| 3.07 | 1.142 | 411.12 | 1.11 | 300.6 |
| 4.06 | 2.50 | 900 | 3.04 | $\mathbf{Y}$ |



Table 5 Erosion Test 1 - Solid hand-cut historic clay brick erosion test results

| Drop <br> Diameter <br> (mm) | Unit No | Dry <br> Unit <br> Mass <br> (grams) | Wet Eroded Unit Mass (grams) |  | Wetting (\%) |  |  | Surface Erosion (\%) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $x$ | $\bar{x}$ | $\sigma$ | $x$ |  | $\sigma$ |
| 3.07 | 1 | 2937.8 | 3196.5 | 2937.6 | 8.81 | 9.17 | 0.33 | 0.0068 |  |  |
|  | 2 | 3063.8 | 3346.8 | 3063.5 | 9.25 |  |  | 0.0098 | 145 | ${ }^{7} 0.011$ |
|  | 3 | 2957.7 | 3236.4 | 2956.9 | 9.45 |  |  | 0.0278 |  |  |
| 4.06 | 4 | 3003.5 | 3316.5 | 3003.4 | 10.42 | 8.90 | 1.40 | 0.0033 |  | 0.005 |
|  | 5 | 3075.3 | 3310.9 | 3074.9 | 7.68 |  |  | 0.13 | 0086 |  |
|  | 6 | 3164.7 | 3436.2 | 3164.4 | 8.59 |  |  | O0 |  |  |



Table 6 Erosion Test 2 - Unfired clay bricks (Ecoterre) erosion test results

| DropDiameter$(\mathrm{mm})$ | $\begin{aligned} & \text { Unit } \\ & \text { No } \end{aligned}$ | Dry Unit Mass (grams) | Wet Eroded Unit Mass (grams) | Dry Eroded Unit Mass (grams) | Wetting (\%) |  |  | Surface Erosion (\%) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $x$ | $\bar{x}$ | $\sigma$ | $x$ |  | $\sigma$ |
| 3.07 | 1 | 3114.6 | 3158.5 | 3081.6 | 2.50 | 3.24 | 0.66 | 1.0595 |  | $Y_{0.102}$ |
|  | 2 | 3115.9 | 3194.6 | 3087.6 | 3.47 |  |  | 0.9082 |  |  |
|  | 3 | 3105.4 | 3186.6 | 3071.2 | 3.76 |  |  | 1.1012 |  |  |
| 4.06 | 4 | 3123.5 | 3181.8 | 3096.6 | 2.75 | 3.49 | 0.69 | 0.8612 |  | 0.061 |
|  | 5 | 3111.9 | 3195.9 | 3084.9 | 3.60 |  |  | $0 \times 66$ | . 8293 |  |
|  | 6 | 3148.3 | 3253.2 | 3124.4 | 4.12 |  |  | \% 75 |  |  |



Table 7 Number of drops in the impact behaviour tests.

| Units | Impact Behaviour | Impact Angle |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $5^{\circ}$ | $15^{\circ}$ | $25^{\circ}$ | $35^{\circ}$ | $45^{\circ}$ |
| Solid hand-cut <br> historic clay <br> bricks  | Bouncing | 5 | 3 | 1 | 1 | 0 |
|  | Running-off | 0 | 0 | 0 | 0 | 0 |
|  | Adhesion | 0 | 0 | 0 | 0 | 0 |
|  | Splashing | 1 | 3 | 5 | 5 | 6 |
|  | Average SAR | 0.04 | 0.21 | 0.37 | 0.53 | 0.76 |
| Fired clay bricks (Berkeley Red Multi) | Bouncing | 2 | 0 | 0 | 0 | 0 |
|  | Running-off | 0 | 0 | 0 | 0 | 0 |
|  | Adhesion | 0 | 0 | 0 | 0 | 0 |
|  | Splashing | 4 | 6 | 6 | 6 | 6 |
|  | Average SAR | 0.07 | 0.25 | 0.42 | 0.59 |  |
| Unfired clay bricks (Ecoterre) | Bouncing | 4 | 1 | 0 | 0 | Q |
|  | Running-off | 2 | 3 | 3 | $\lambda$ | 3 |
|  | Adhesion | 0 | 0 | 0 |  | 0 |
|  | Splashing | 0 | 2 |  |  | 3 |
|  | Average SAR | 0.09 | 0.22 | 40.3 | 0.53 | 0.72 |
| Lime mortar | Bouncing | 4 | 0 | 0 | 0 | 0 |
|  | Running-off | 0 |  |  | 0 | 0 |
|  | Adhesion | 0 |  | 1 | 1 | 0 |
|  | Splashing | 2 |  | 5 | 5 | 6 |
|  | Average SAR | 4 | 0.25 | 0.44 | 0.58 | 0.66 |
| Total of the 4 type of units | $\text { Bouncing } / 15 \times 1$ |  |  | 1 | 1 | 0 |
|  | Running-of |  | 3 | 3 | 3 | 3 |
|  | Adhesic 6 | 0 | 1 | 1 | 1 | 0 |
|  | Splashing | 7 | 16 | 19 | 19 | 21 |
|  | Avores | 0.07 | 0.23 | 0.40 | 0.56 | 0.70 |




Fig. 2. Seasonal rainfall viation in the UK





Fig. 5. Stain aspect ratios dratu drops on masonry material with respect to impact angle






Fig. 9. Monthly rainfall and wind speed averages in Boscombe Down andiymam between 1971 and 2000







Fig. 13. Averaged DRMS data from eroded and uneroded spots of unfired (Ecoterre) tested with 3.07 mm and 4.06 mm diameter drops and DRMS data from groded ayd uneroded spots of solid hand-cut historic clay brick tested with $3.07 \mathrm{~mm} \square$ drops (only one testgd unit results)


## Highlights

- Raindrop erosion considering hazard, vulnerability exposure and values is defined
- Physical raindrop impact on masonry surface erosion is comprehensively docune
- Impact angle, drop size, surface roughness are crucial factors in surface response
- Bond strength in erosion and moisture absorption features in wetting are ceray
- Raindrop induced surface erosion causes strength degradation of building ntaterls


