# Three-dimensional interaction between uniform current and a submerged horizontal cylinder in an ice-covered channel 

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The problem of interaction of a uniform current with a submerged horizontal circular cylinder in an ice-covered channel is considered. The fluid flow is described by linearized velocity potential theory and the ice sheet is treated as a thin elastic plate. The potential due to a source or the Green function satisfying all boundary conditions apart from that on the body surface is first derived. This can be used to derive the boundary integral equation for a body of an arbitrary shape. It can also be used to obtain the solution due to multipoles by differentiating the Green function with its position directly. For a transverse circular cylinder, through distributing multipoles along its centre line, the velocity potential can be written in an infinite series with unknown coefficients, which can be determined from the impermeable condition on body surface. A major feature here is that different from the free surface problem, or a channel without the ice sheet cover, this problem is fully three dimensional because of the constraints along the intersection of the ice sheet with the channel wall. It has been also confirmed that there is an infinite number of critical speeds. Whenever the current speed passes a critical value, the force on the body and wave pattern change rapidly, and two more wave components are generated at the far-field. Extensive results are provided for hydroelastic waves and hydrodynamic forces when the ice sheet is under different edge conditions, and the insight of their physical features is discussed.

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## 1. Introduction

In ocean engineering and naval architecture, it is common to undertake model tests in a wave/towing tank. It can be expected that if the width of the tank is not sufficiently larger than the structure dimension of the model or the wavelength, it can greatly affect the interactions between the fluid flow and the structure. The measured result may not truly reflect that of the prototype in the real ocean. Furthermore, the tank has its natural frequencies, at which resonance of the fluid motion can occur. Such a resonance due to the side wall effect on the other hand does not occur in the ocean. It is then important to understand how the tank wall affects the desired results. Therefore, there has been an extensive amount of work

[^0]on body / flow / tank or channel interactions. By employing linearized velocity potential theory, Linton (1993) derived the Green function for free surface wave in a channel. The diffraction/radiation and the forward speed problem of a submerged sphere in a channel were studied by $\mathrm{Wu}(1998 a, b)$ where multipole expansion was applied. In addition to submerged bodies, wave interaction with surface piercing structures in a channel was also well studied. Linton et al. (1992) provided an analytical solution for wave diffraction and radiation by a vertical circular cylinder using cylindrical system. Later, Evans \& Porter (1997) and Utsunomiya \& Eatock Taylor (1999) solved wave diffraction by multiple cylinders and discussed the effect of trapped mode (Ursell 1951). A more recent work by Newman (2017) provided some detailed description of the trapped mode phenomenon in a channel for various structures, including submerged bodies and bottom-mounted cylinders with different cross sections.

In recent years, there has been an increasing interest in the hydrodynamic problems in polar and other icy water regions. One of the typical features in these regions is the ice in many different forms, one of which is ice sheet covering water surface over very large extent. In the experiment, the ice sheet will meet the tank wall. At their intersection, the physical constrain of the ice sheet edge, including whether the edge is clamped, simply supported or free, can significantly affect the result. In fact, it has been shown in Korobkin et al. (2014) and Ren et al. (2020) that while the transverse modes of the fluid flow are mathematically orthogonal, they are still completely coupled. One consequence of this coupling is that unlike the free surface problem, the purely two-dimensional wave propagating along the channel is impossible when there is an ice sheet cover. The work by Ren et al. (2020) is mainly for wave propagation without any structures in its path, although the case of an ice sheet with a crack is considered. Here we shall consider a problem of a body submerged in a channel below an ice sheet in a uniform current. This is similar to a submerged body moving forward with constant speed. Although the problem may seem to be conventional for the free surface flow, when there is an ice sheet the physics of the fluid flow and the resistance and lift on the body is very different. The present work aims to shed some light into this.

When there is no channel wall, or for the open ice sheet problem, there has been a large volume of work on interactions of fluid flow and ice sheet. In mathematical modelling, the ice sheet is treated as a thin elastic plate and the fluid flow is described by the linearized velocity potential theory. A review of some early works based on this method can be found in Squire (2007). Typical three-dimensional works on wave interaction with ice sheets/floes include those by Fox \& Squire (1994) and Balmforth \& Craster (1999) for oblique wave diffraction by a semi-infinite ice sheet, Meylan \& Squire (1996) for wave interaction with a circular ice floe, Bennetts \& Williams (2010) for wave diffraction by an ice floe of arbitrary shapes, and by Porter (2019) for wave interaction with a rectangular ice floe floating on ocean. There are also works on imperfect ice sheets including cracks, such as Evans \& Porter (2003) for hydroelastic waves propagating by a single straight-line crack, and Porter \& Evans (2007) for multiple straight-line cracks parallel to each other, and a recent work by Li et al. (2020c) for multiple cracks with arbitrary shapes on ice sheet.

The work mentioned above is mainly about interaction between wave and ice sheet. In polar engineering, it is also important to consider their interaction with structures. For threedimensional submerged bodies, Das \& Mandal (2008) studied wave radiation by a submerged sphere in a fluid with an ice-cover by multipole expansion method. Sturova (2013) derived the time domain Green function due to a source undergoing arbitrary three-dimensional motion in water below an ice sheet with infinite extent, and further considered the wave radiation by a submerged sphere with a forward speed. For wave interaction with structures piercing ice plate or water surface, Brocklehurst et al. (2011) investigated the diffraction problem of hydroelastic wave beneath ice sheet by a single bottom-mounted circular cylinder based on
the Weber transform. Later, Dişibüyük et al. (2020) further extended it to vertical cylinder of non-circular cross section by applying the perturbation method at the mean position of the section. Hydroelastic wave diffraction problems by multiple vertical cylinders are solved by Ren et al. (2018a). When the ice sheet is not directly contacted with the surface of the structures, such as structures are located in a polynya or open water confined by ice sheets, mixed upper surface boundary conditions need to be considered. Ren et al. (2018b) investigated wave diffraction and radiation by a vertical circular cylinder standing arbitrarily in a circular polynya, while Li et al. (2020a) employed a hybrid numerical method and considered floating structure of arbitrary shapes in a polynya with various shapes.

Compared with unbounded sea covered by an ice sheet, the hydrodynamic features in an ice-covered channels are quite different. Korobkin et al. (2014) studied hydroelastic wave propagating along a rectangular channel with homogeneous ice cover clamped to the side walls. The velocity potential and ice sheet deflection are first expanded into different eigenfunctions. Each term in the expression of the ice sheet deflection satisfies the edge condition and is further expanded into Fourier series used for the velocity potential. The dispersion relations of the channel can be obtained through finding non-trivial solutions of the homogeneous linear equations. Based on the procedure of Korobkin et al. (2014), Shishmarev et al. (2016) and Khabakhpasheva et al. (2019) investigated the hydroelastic waves due to a load moving with a constant speed along a frozen channel through frequency domain and time domain methods, respectively. However, their results and conclusion are only for the clamped edges. Ren et al. (2020) explicitly discussed the merit and weakness of the method in Korobkin et al. (2014), and then they proposed a more efficient and flexible approach to investigate the propagation of hydroelastic waves in a channel with an ice cover subject to various edge constrains at the side walls, and also the effect of a longitudinal line crack on the ice. In their work, both the velocity potential and the fourth transverse-derivative of the ice deflection are expanded into a series of cosine functions in the transverse direction. The expression of deflection itself is obtained through integration, which contains a series of cosine functions and a quartic polynomial with four additional unknown constants. Using the kinematic and dynamic conditions on the ice sheet, the system of linear equations in terms of these four constants can be obtained by imposing edge conditions at the channel wall. Based on this method, the solution procedure is very much simplified, and it is very convenient to consider different combinations of edge conditions and the effect of the crack.

In the present work, the interaction of a uniform current with a submerged body in an ice-covered rectangular channel is considered. The three-dimensional Green function, or the velocity potential due to a source is first derived. This can then be used to derive the integral equation over the body surface of arbitrary shapes. In particular, we shall consider a submerged horizontal circular cylinder with its axis in the transverse direction. Multipole (Ursell 1949, 1950) is distributed along the centre line. As a result, the velocity potential can be written explicitly in terms of basic and special functions, involving integrals and unknown coefficients which can be obtained from the impermeable condition on the body surface. For the two-dimensional free surface case, the problem of current passing a submerged cylinder has been considered extensively. Lamb (1932) used an approximation where the linearized free surface boundary condition is satisfied exactly but the body surface boundary conditions approximately. Havelock (1936) solved the linear problem exactly in the sense the infinite series can be truncated at a sufficiently large number to achieve the desired accuracy. Tuck (1965) considered the second-order effects, while Haussling \& Coleman (1979), Scullen \& Tuck (1995) and Semenov \& Wu (2020) further solved the fully nonlinear problem. However, with the ice sheet, this will be a fully three-dimensional problem and the flow is much more complex. In particular, there will be an infinite number of critical speeds, and multi-wave components at both sides of the channel. Based on the dispersion relationship, extensive


Figure 1: Sketch of the problem. (a) Three-dimensional view; (b) A cross section view of the channel in the negative $x$-direction.
analyses are made for the physical behaviours of the deflection of the ice cover and the hydrodynamic forces on the cylinder. Compared with the two-dimensional case, it is found that the confined channel walls and the constraints at the ice edge have a significant influence on the hydrodynamic features.

The paper is arranged as follows. The governing equation and boundary conditions for a submerged horizontal circular cylinder in an ice-covered channel in current is presented in Section 2. The Green function or potential due to a single source is derived in Section 3.1. The multipole expansion is constructed in Section 3.2. The formulas of hydrodynamic forces on the cylinder and ice deflection are obtained in sections Section 3.3 and Section 3.4 respectively. The numerical procedure is briefly introduced in Section 3.5. The numerical results are shown in Section 4, followed by the conclusions in Section 5. The expression of some essential coefficients is given in Appendix A. The symmetry property of the Green function is proved in Appendix B, while the far-field formula of the resistance is derived in Appendix C.

## 2. Governing Equation and boundary conditions

We consider the problem of a horizontal circular cylinder submerged in an infinitely long rectangular channel covered by an ice sheet. A sketch of the problem is shown in figure 1 . The channel has half-width $b$ and calm water depth $H$. The density and thickness of the homogeneous ice sheet are assumed to be constant and represented by $\rho_{i}$ and $h_{i}$ respectively, and the density of the water is $\rho$. A Cartesian coordinate system $O-x y z$ is defined with the origin located on the central line of the water surface, the $x$-axis along the longitudinal direction of the channel, and $z$-axis pointing upwards. The centre line of the cylinder is located at $x=x_{0} \& z=z_{0}$ and its radius is equal to $r_{0}$.

It is assumed that the fluid is ideal, incompressible, its motion is irrotational, and the linearized velocity potential theory is employed. As discussed in the introduction, due to the ice sheet and its edge conditions, the problem will be three dimensional. The total velocity potential is written as

$$
\begin{equation*}
\Phi=-U x+\phi \tag{2.1}
\end{equation*}
$$

where $U$ denotes the velocity of the uniform current from $x=+\infty, \phi(x, y, z, t)$ is the disturbed velocity potential by the cylinder, which satisfies the Laplace equation in the entire
fluid domain,

$$
\begin{equation*}
\nabla^{2} \phi=\frac{\partial^{2} \phi}{\partial x^{2}}+\frac{\partial^{2} \phi}{\partial y^{2}}+\frac{\partial^{2} \phi}{\partial z^{2}},-\infty<x<+\infty,-b \leqslant y \leqslant b,-H \leqslant z \leqslant 0 \tag{2.2}
\end{equation*}
$$

The deflection of the ice plate $\eta(x, y, t)$ should satisfy the Euler-Bernoulli equation on $z=0$,

$$
\begin{equation*}
\rho_{i} h_{i} \eta_{t t}+L \nabla^{4} \eta=p, \quad z=0 \tag{2.3}
\end{equation*}
$$

where $L=E h_{i}^{3} /\left[12\left(1-v^{2}\right)\right]$ denotes the flexural rigidity of the ice sheet, $E$ and $v$ represent its Young's modulus and Poisson's ratio respectively. The fluid pressure $p$ on the right-hand side of (2.3) is the excessive fluid pressure and does not include the weight of ice. It can be calculated through the linearized Bernoulli equation

$$
\begin{equation*}
p=-\rho\left(\frac{\partial \phi}{\partial t}-U \frac{\partial \phi}{\partial x}+g \eta\right), \quad z=0 \tag{2.4}
\end{equation*}
$$

where $g$ is the acceleration due to gravity. The kinematic boundary condition can be written as

$$
\begin{equation*}
\left(\frac{\partial}{\partial t}-U \frac{\partial}{\partial x}\right) \eta=\frac{\partial \phi}{\partial z}, \quad z=0 \tag{2.5}
\end{equation*}
$$

For steady flow, $\partial / \partial t=0$. We have

$$
\begin{gather*}
\left(L \nabla^{4}+\rho g\right) \eta=\rho U \frac{\partial \phi}{\partial x}, \quad z=0  \tag{2.6}\\
-U \frac{\partial \eta}{\partial x}=\frac{\partial \phi}{\partial z}, \quad z=0 \tag{2.7}
\end{gather*}
$$

in which (2.6) is obtained by substituting (2.4) into (2.3). The impermeable condition on the cylinder surface $S_{B}$ can be written as

$$
\begin{equation*}
\frac{\partial \phi}{\partial n}=U n_{x}, \quad \text { on } S_{B}, \tag{2.8}
\end{equation*}
$$

where $\boldsymbol{n}=\left(n_{x}, 0, n_{z}\right)$ is the unit normal vector of $S_{B}$, which is pointing out of the fluid domain. Similarly, the impermeable boundary conditions on the rigid side walls and bottom of the channel can be expressed as

$$
\begin{align*}
& \frac{\partial \phi}{\partial y}=0, \quad y= \pm b  \tag{2.9}\\
& \frac{\partial \phi}{\partial z}=0, \quad z=-H \tag{2.10}
\end{align*}
$$

Different from free surface problems, there are also edge conditions at intersection lines of the ice sheet with the two side walls, or $y= \pm b, z=0$, which can be written as (Timoshenko \& Woinowsky-Krieger 1959)

$$
\left.\begin{array}{rlrl}
\eta=0, & \frac{\partial \eta}{\partial y}=0, & \text { Clamped edge } \\
\eta & =0, & \frac{\partial^{2} \eta}{\partial y^{2}}+v \frac{\partial^{2} \eta}{\partial x^{2}}=0, & \text { Simply supported edge } \\
\frac{\partial^{2} \eta}{\partial y^{2}}+v \frac{\partial^{2} \eta}{\partial x^{2}}=0, & \frac{\partial^{3} \eta}{\partial y^{3}}+(2-v) \frac{\partial^{3} \eta}{\partial x^{2} \partial y}=0, & \text { Free edge } \tag{2.11}
\end{array}\right\} .
$$

The radiation condition at far-field $x \rightarrow \pm \infty$ can be written as

$$
\begin{equation*}
\frac{\partial \phi}{\partial x}=\mathcal{U}_{ \pm}(x, y, z), \quad x \rightarrow \pm \infty \tag{2.12}
\end{equation*}
$$

where $\mathcal{U}_{ \pm}(x, y, z)$ represents waves generated by the cylinder. The waves can have multi components and group velocities of the waves at $x \rightarrow+\infty$ and $x \rightarrow-\infty$ are larger and smaller than $U$ respectively. This will be discussed in detail later.

## 3. Solution procedures

### 3.1. The Green function: velocity potential due to a single source

The Green function $G\left(x, y, z, x_{0}, y_{0}, z_{0}\right)$ is the velocity potential at point $P(x, y, z)$ due to a source at $P_{0}\left(x_{0}, y_{0}, z_{0}\right)$, which satisfies the following equation,

$$
\begin{equation*}
\nabla^{2} G=\delta\left(x-x_{0}\right) \delta\left(y-y_{0}\right) \delta\left(z-z_{0}\right) \tag{3.1}
\end{equation*}
$$

where $\delta(x)$ is the Dirac delta-function. $\xi\left(x, y, x_{0}, y_{0}, z_{0}\right)$ is defined as the wave elevation at point $(x, y)$ induced by the source at $P_{0}\left(x_{0}, y_{0}, z_{0}\right) . G\left(x, y, z, x_{0}, y_{0}, z_{0}\right)$ and $\xi\left(x, y, x_{0}, y_{0}, z_{0}\right)$ also need to satisfy the boundary conditions (2.6), (2.7) and (2.9)~(2.12).
Performing the Fourier transform for $G$ and $\xi$ in the $x$-direction

$$
\left.\begin{array}{l}
\hat{G}=\int_{-\infty}^{+\infty} G e^{-i k x} d x  \tag{3.2}\\
\hat{\xi}=\int_{-\infty}^{+\infty} \xi e^{-i k x} d x
\end{array}\right\}
$$

and applying (3.2) to (3.1), we have

$$
\begin{equation*}
-k^{2} \hat{G}+\frac{\partial^{2} \hat{G}}{\partial y^{2}}+\frac{\partial^{2} \hat{G}}{\partial z^{2}}=\delta\left(y-y_{0}\right) \delta\left(z-z_{0}\right) e^{-\mathrm{i} k x_{0}} \tag{3.3}
\end{equation*}
$$

Based on the impermeable condition in (2.9), $\hat{G}$ can be further expanded into an orthogonal series of cosine functions in the $y$-direction. Using the condition in (2.10), the solution of (3.3) can be written in the following form

$$
\begin{equation*}
\hat{G}=\sum_{n=0}^{+\infty} Z_{n}\left(k, z, x_{0}, y_{0}, z_{0}\right) \cos \sigma_{n}(y+b), \tag{3.4}
\end{equation*}
$$

where

$$
\begin{align*}
Z_{n}\left(k, z, x_{0}, y_{0}, z_{0}\right)= & -\frac{\left[e^{-K_{n}\left|z-z_{0}\right|}+e^{-K_{n}\left(z+z_{0}+2 H\right)}\right] e^{-\mathrm{i} k x_{0}} \cos \sigma_{n}\left(y_{0}+b\right)}{2\left(1+\delta_{n 0}\right) b K_{n}}  \tag{3.5}\\
& +b_{n} \frac{e^{-\mathrm{i} k x_{0}} \cosh K_{n}(z+H)}{\cosh K_{n} H}
\end{align*}
$$

$\sigma_{n}=n \pi / 2 b, K_{n}=\sqrt{k^{2}+\sigma_{n}^{2}}$, and $\delta_{i j}$ denotes the Kronecker delta function. The terms of $b_{n}$ in (3.5) correspond to the general solution of (3.3) when the right-hand side is zero. $b_{n}$ are to be determined by the conditions on the ice sheet. Applying Fourier transform to (2.6) and (2.7), we have

$$
\begin{equation*}
\left(\rho g+L k^{4}\right) \hat{\xi}-2 L k^{2} \frac{\partial^{2} \hat{\xi}}{\partial y^{2}}+L \frac{\partial^{4} \hat{\xi}}{\partial y^{4}}=\mathrm{i} k \rho U \hat{G}, \tag{3.6}
\end{equation*}
$$

$$
\begin{equation*}
-\mathrm{i} k U \hat{\xi}=\frac{\partial \hat{G}}{\partial z} \tag{3.7}
\end{equation*}
$$

We choose to follow the approach taken by Ren et al. (2020), which involves expanding $\partial^{4} \hat{\xi} / \partial y^{4}$ rather than $\hat{\xi}$ into a cosine series, thus

$$
\begin{equation*}
\frac{\partial^{4} \hat{\xi}}{\partial y^{4}}=e^{-\mathrm{i} k x_{0}} \sum_{n=0}^{+\infty} a_{n} \cos \sigma_{n}(y+b) \tag{3.8}
\end{equation*}
$$

Then through integration four times, $\hat{\xi}$ can be obtained as

$$
\begin{equation*}
\hat{\xi}=e^{-\mathrm{i} k x_{0}}\left[c_{0}+c_{1} y+c_{2} y^{2}+c_{3} y^{3}+\frac{a_{0}}{24} y^{4}+\sum_{n=1}^{+\infty} \frac{a_{n}}{\sigma_{n}^{4}} \cos \sigma_{n}(y+b)\right] \tag{3.9}
\end{equation*}
$$

where $a_{n}(n=0,1,2 \ldots)$ are unknown coefficients and are functions of $k, c_{i}(i=0 \sim 3)$ are four constants which can be linked to $a_{n}$ through edge conditions. It should be mentioned here that $c_{0}, c_{2}$ and $a_{2 n}$ correspond to symmetric component, while $c_{1}, c_{3}$ and $a_{2 n+1}$ correspond to anti-symmetric component. Substituting (3.4), (3.5) and (3.9) into (3.6) and (3.7), we have

$$
\begin{align*}
& \left(\rho g+L k^{4}\right)\left[c_{0}+c_{1} y+c_{2} y^{2}+c_{3} y^{3}+\frac{a_{0}}{24} y^{4}+\sum_{n=1}^{+\infty} \frac{a_{n}}{\sigma_{n}^{4}} \cos \sigma_{n}(y+b)\right] \\
& -2 k^{2} L\left[2 c_{2}+6 c_{3} y+\frac{a_{0}}{2} y^{2}-\sum_{n=1}^{+\infty} \frac{a_{n}}{\sigma_{n}^{2}} \cos \sigma_{n}(y+b)\right]+L \sum_{n=0}^{+\infty} a_{n} \cos \sigma_{n}(y+b)  \tag{3.10}\\
& =\mathrm{i} k \rho U \sum_{n=0}^{+\infty}\left[-\frac{e^{-K_{n} H} \cosh K_{n}\left(z_{0}+H\right)}{\left(1+\delta_{n 0}\right) b K_{n}} \cos \sigma_{n}\left(y_{0}+b\right)+b_{n}\right] \cos \sigma_{n}(y+b),
\end{align*}
$$

$$
\begin{align*}
& -\mathrm{i} k U\left[c_{0}+c_{1} y+c_{2} y^{2}+c_{3} y^{3}+\frac{a_{0}}{24} y^{4}+\sum_{n=1}^{+\infty} \frac{a_{n}}{\sigma_{n}^{4}} \cos \sigma_{n}(y+b)\right] \\
& =\sum_{n=0}^{+\infty}\left[\frac{e^{-K_{n} H} \cosh K_{n}\left(z_{0}+H\right)}{\left(1+\delta_{n 0}\right) b} \cos \sigma_{n}\left(y_{0}+b\right)+b_{n} K_{n} \tanh K_{n} H\right] \cos \sigma_{n}(y+b) \tag{3.11}
\end{align*}
$$

The term $y^{j}(j=0 \sim 4)$ can be further expanded into the orthogonal series of cosine functions as

$$
\begin{equation*}
y^{j}=\sum_{n=0}^{+\infty} d_{n}^{(j)} \cos \sigma_{n}(y+b) \tag{3.12}
\end{equation*}
$$

Then, (3.10) and (3.11) can be written as

$$
\begin{align*}
& \left(\rho g+L k^{4}\right)\left[c_{0} d_{n}^{(0)}+c_{1} d_{n}^{(1)}+c_{2} d_{n}^{(2)}+c_{3} d_{n}^{(3)}+\frac{a_{0}}{24} d_{n}^{(4)}+\frac{\left(1-\delta_{n 0}\right) a_{n}}{\sigma_{n}^{4}}\right] \\
& -2 k^{2} L\left[2 c_{2} d_{n}^{(0)}+6 c_{3} d_{n}^{(1)}+\frac{a_{0}}{2} d_{n}^{(2)}-\frac{\left(1-\delta_{n 0}\right) a_{n}}{\sigma_{n}^{2}}\right]+L a_{n}  \tag{3.13}\\
& =\mathrm{i} k \rho U\left[-\frac{e^{-K_{n} H} \cosh K_{n}\left(z_{0}+H\right)}{\left(1+\delta_{n 0}\right) b K_{n}} \cos \sigma_{n}\left(y_{0}+b\right)+b_{n}\right], n=0,1,2 \ldots
\end{align*}
$$

$$
\begin{align*}
& -\mathrm{i} k U\left[c_{0} d_{n}^{(0)}+c_{1} d_{n}^{(1)}+c_{2} d_{n}^{(2)}+c_{3} d_{n}^{(3)}+\frac{a_{0}}{24} d_{n}^{(4)}+\frac{\left(1-\delta_{n 0}\right) a_{n}}{\sigma_{n}^{4}}\right] \\
& =\left[\frac{e^{-K_{n} H} \cosh K_{n}\left(z_{0}+H\right)}{\left(1+\delta_{n 0}\right) b} \cos \sigma_{n}\left(y_{0}+b\right)+b_{n} K_{n} \tanh K_{n} H\right], n=0,1,2 \ldots \tag{3.14}
\end{align*}
$$

From (3.13) and (3.14), $a_{n}$ and $b_{n}$ can be expressed as

$$
\begin{equation*}
a_{n}=\alpha_{n, 0} c_{0}+\alpha_{n, 1} c_{1}+\alpha_{n, 2} c_{2}+\alpha_{n, 3} c_{3}+\mathrm{i} R_{n}, \quad n=0,1,2 \ldots \tag{3.15}
\end{equation*}
$$

$$
\begin{equation*}
b_{n}=\frac{1}{K_{n} \tanh K_{n} H}\left(\mathrm{i} \beta_{n, 0} c_{0}+\mathrm{i} \beta_{n, 1} c_{1}+\mathrm{i} \beta_{n, 2} c_{2}+\mathrm{i} \beta_{n, 3} c_{3}+S_{n}\right), \quad n=0,1,2 \ldots \tag{3.16}
\end{equation*}
$$

where

$$
\alpha_{n, j}=\frac{1}{\Delta_{n}}\left\{\begin{array}{l}
d_{n}^{(j)}\left[\left(\rho g+L k^{4}\right) K_{n} \tanh K_{n} H-\rho k^{2} U^{2}\right]  \tag{3.17a}\\
-4 \delta_{2 j} k^{2} L d_{n}^{(0)} K_{n} \tanh K_{n} H-12 \delta_{3 j} k^{2} L d_{n}^{(1)} K_{n} \tanh K_{n} H
\end{array}\right\}+\left(1-\delta_{n 0}\right) \gamma_{n} \alpha_{0, j},
$$

$$
\begin{equation*}
\beta_{n, j}=-U k\left[d_{n}^{(j)}+\frac{d_{n}^{(4)}}{24} \alpha_{0, j}+\frac{\left(1-\delta_{n 0}\right)}{\sigma_{n}^{4}} \alpha_{n, j}\right], \quad j=0 \sim 3 \tag{3.17b}
\end{equation*}
$$

and

$$
\begin{equation*}
R_{n}=\frac{\rho U k \cosh K_{n}\left(z_{0}+H\right) \cos \sigma_{n}\left(y_{0}+b\right)}{\left(1+\delta_{n 0}\right) b \Delta_{n} \cosh K_{n} H}+\left(1-\delta_{n 0}\right) \gamma_{n} R_{0}, \tag{3.18a}
\end{equation*}
$$

$$
\begin{equation*}
S_{n}=k U\left(\frac{d_{n}^{(4)}}{24} R_{0}+\frac{1-\delta_{n 0}}{\sigma_{n}^{4}} R_{n}\right)-\frac{e^{-K_{n} H} \cosh K_{n}\left(z_{0}+H\right) \cos \sigma_{n}\left(y_{0}+b\right)}{\left(1+\delta_{n 0}\right) b}, \tag{3.18b}
\end{equation*}
$$

with

$$
\Delta_{n}=\left\{\begin{array}{l}
\delta_{n 0}\left[-\left(\frac{\rho g+L k^{4}}{24} d_{n}^{(4)}-k^{2} L d_{n}^{(2)}+L\right) K_{n} \tanh K_{n} H+\frac{\rho k^{2} U^{2} d_{n}^{(4)}}{24}\right]  \tag{3.19a}\\
+\left(1-\delta_{n 0}\right)\left[-\left(\frac{\rho g+L k^{4}}{\sigma_{n}^{4}}+\frac{2 k^{2} L}{\sigma_{n}^{2}}+L\right) K_{n} \tanh K_{n} H+\frac{\rho k^{2} U^{2}}{\sigma_{n}^{4}}\right]
\end{array}\right\},
$$

$$
\begin{equation*}
\gamma_{n}=\frac{1}{\Delta_{n}}\left[\left(\frac{\rho g+L k^{4}}{24} d_{n}^{(4)}-k^{2} L d_{n}^{(2)}\right) K_{n} \tanh K_{n} H-\frac{\rho k^{2} U^{2} d_{n}^{(4)}}{24}\right] . \tag{3.19b}
\end{equation*}
$$

If we consider the symmetry or anti symmetry nature of $y^{j}(j=0 \sim 3)$, we will have $d_{2 n}^{(2 j+1)}=d_{2 n+1}^{(2 j)}=0(j=0,1)$. This leads to $\gamma_{2 n+1}=0$ and $\alpha_{2 n+1,2 j}=\alpha_{2 n, 2 j+1}=0$ $(j=0,1, n \geqslant 0)$. As a result, $a_{n}$ can be further expressed as

$$
\left.\begin{array}{rl}
a_{2 n} & =\alpha_{2 n, 0} c_{0}+\alpha_{2 n, 2} c_{2}+\mathrm{i} R_{2 n}  \tag{3.20}\\
a_{2 n+1} & =\alpha_{2 n+1,1} c_{1}+\alpha_{2 n+1,3} c_{3}+\mathrm{i} R_{2 n+1}
\end{array}\right\} .
$$

It can be seen from (3.20) that $a_{2 n}$ depend on two unknown coefficients $c_{0}$ and $c_{2}$, while $a_{2 n+1}$ depend on $c_{1}$ and $c_{3}$. $b_{n}$ can be also treated in a similar way. The four unknown coefficients $c_{i}(i=0 \sim 3)$ can be determined from the four edge conditions, including those in (2.11). As a result, a system of linear equations of the following form can be established,

$$
\begin{equation*}
[\mathcal{A}][\boldsymbol{C}]=[\mathcal{B}], \tag{3.21}
\end{equation*}
$$

where $[\mathcal{A}]$ is a $4 \times 4$ coefficient matrix, $[\boldsymbol{C}]$ is a column containing $c_{i}$, and $[\mathcal{B}]$ is a known
column. For a specific case, this $4 \times 4$ matrix equation may be further simplified. If the edge conditions at $y= \pm b$ are same, (3.21) may be split into two independent $2 \times 2$ submatrix equations, one for $c_{0}$ and $c_{2}$, and the other for $c_{1}$ and $c_{3}$. As a result, the symmetric and anti-symmetric transverse waves in (3.9) become independent to each other. In general, $c_{i}$ $(i=0 \sim 3)$ are fully coupled to each other, which leads $a_{2 n}$ and $a_{2 n+1}$ in (3.20) to become coupled. The solution of $c_{i}$ can be written as

$$
\begin{equation*}
c_{i}=-\mathrm{i} \sum_{m=0}^{+\infty} \frac{\rho U k \cosh K_{m}\left(z_{0}+H\right) \cos \sigma_{m}\left(y_{0}+b\right)}{\left(1+\delta_{m 0}\right) b|\mathcal{A}| \Delta_{m} \cosh K_{m} H} c_{m, i}^{\prime}, \quad i=0 \sim 3 \tag{3.22}
\end{equation*}
$$

where $|\mathcal{A}|$ is the determinant of $[\mathcal{A}]$. The elements of $[\mathcal{A}]$ and $[\mathcal{B}]$, as well as coefficient $c_{m, i}^{\prime}$ are related to edge conditions, an example of these under clamped-clamped edges are given in appendix A.

Once $c_{i}(i=0 \sim 3), a_{n}$ and $b_{n}(n=0,1,2 \ldots)$ are obtained, then $G$ and $\xi$ can be obtained through the inverse Fourier transform. Using (Linton \& McIver 2001)

$$
\begin{gather*}
\int_{-\infty}^{+\infty} \frac{\delta_{n 0} e^{-K_{n} H}-e^{-K_{n}\left|z-z_{0}\right|+\mathrm{i} k\left(x-x_{0}\right)}}{2 K_{n}} d k=\delta_{n 0} \ln \left(\frac{r}{H}\right)-\left(1-\delta_{n 0}\right) \mathscr{K}_{0}\left(\sigma_{n} r\right),  \tag{3.23a}\\
\int_{-\infty}^{+\infty} \frac{\delta_{n 0} e^{-K_{n} H}-e^{-K_{n}\left(z+z_{0}+2 H\right)+\mathrm{i} k\left(x-x_{0}\right)}}{2 K_{n}} d k=\delta_{n 0} \ln \left(\frac{r^{\prime}}{H}\right)-\left(1-\delta_{n 0}\right) \mathscr{K}_{0}\left(\sigma_{n} r^{\prime}\right), \tag{3.23b}
\end{gather*}
$$

where $r=\sqrt{\left(x-x_{0}\right)^{2}+\left(z-z_{0}\right)^{2}}$ and $r^{\prime}=\sqrt{\left(x-x_{0}\right)^{2}+\left(z+z_{0}+2 H\right)^{2}}, \mathscr{K}_{n}$ denotes the $n$ th-order modified Bessel function of second kind. We have the Green function

$$
\begin{gather*}
G\left(x, y, z, x_{0}, y_{0}, z_{0}\right)=\sum_{n=0}^{+\infty} \mathcal{Z}_{n}\left(x, z, x_{0}, y_{0}, z_{0}\right) \cos \sigma_{n}(y+b)  \tag{3.24}\\
\mathcal{Z}_{n}\left(x, z, x_{0}, y_{0}, z_{0}\right)=\mathcal{Z}_{n, 1}+\mathcal{Z}_{n, 2} \tag{3.25}
\end{gather*}
$$

and

$$
\begin{equation*}
\mathcal{Z}_{n, 1}=\frac{1}{4 \pi b}\left\{\delta_{n 0}\left[\ln \left(\frac{r}{H}\right)+\ln \left(\frac{r^{\prime}}{H}\right)\right]-2\left(1-\delta_{n 0}\right)\left[\mathscr{K}_{0}\left(\sigma_{n} r\right)+\mathscr{K}_{0}\left(\sigma_{n} r^{\prime}\right)\right]\right\} \cos \sigma_{n}\left(y_{0}+b\right) \tag{3.26a}
\end{equation*}
$$

$$
\begin{align*}
& \mathcal{Z}_{n, 2}=\frac{\rho U}{2 \pi b} \times \\
& \quad \int_{\mathscr{L}} \sum_{m=0}^{+\infty} \frac{k \mu_{n, m} \cosh K_{m}\left(z_{0}+H\right) \cos \sigma_{m}\left(y_{0}+b\right)\left[\cosh K_{n}(z+H) e^{\mathrm{i} k\left(x-x_{0}\right)}-\delta_{n 0}\right]}{\left(1+\delta_{m 0}\right)|\mathcal{A}| \Delta_{m} K_{n} \sinh K_{n} H \cosh K_{m} H} d k, \tag{3.26b}
\end{align*}
$$

where

$$
\begin{align*}
\mu_{n, m}= & \sum_{j=0}^{3} \beta_{n, j} c_{m, j}^{\prime}+\delta_{m 0}|\mathcal{A}| U k\left(\frac{d_{n}^{(4)}}{24}+\frac{\left(1-\delta_{n 0}\right) \gamma_{n}}{\sigma_{n}^{4}}\right)  \tag{3.27}\\
& +\delta_{n m}|\mathcal{A}|\left[\frac{U k\left(1-\delta_{n 0}\right)}{\sigma_{n}^{4}}-\frac{\Delta_{n}\left(1+e^{-2 K_{n} H}\right)}{2 \rho U k}\right] .
\end{align*}
$$

It should be mentioned that a constant term is respectively added to $\mathcal{Z}_{n, 1}$ and $\mathcal{Z}_{n, 2}$ in (3.26) to remove the high-order singularity at $k=0$, which will not affect the results as all the equations for $G$ involve only its spatial derivatives. The term $\mathcal{Z}_{n, 1}$ is obtained based on the derivation in Li et al. (2020b). There will be singularities in $\mathcal{Z}_{n, 2}$ when $|\mathcal{A}|(k)=0$. Because $|\mathcal{A}|(k)$ is an even function, all its real roots can be represented by $\pm k_{s}(s=1 \ldots S)$,
with $k_{s}>0$. In the case of Ren et al. (2020) for wave propagation in the channel, each $k_{s}$ corresponds to the dispersion relationship between the wave frequency and wave number. Here mathematically each $k_{s}$ corresponds to a singularity in the integrand of the inverse Fourier transform. Physically, it corresponds to each wave generated by the source in the channel. The number of singularities can be more than one, and the value of $S$ depends on the current speed $U$ when other parameters are fixed, which will be discussed in detail later. The way to treat each singularity will depend on the group velocity of its corresponding wave. The wave will be at upstream $(x=+\infty)$ or downstream $(x=-\infty)$ if its group velocity is larger or smaller than $U$ respectively. Based on this, the integration route $\mathscr{L}$ in $\mathcal{Z}_{n, 2}$ from $-\infty$ to $+\infty$ can be defined as follows. We may consider the integration of $f(k) e^{i k x} /\left(k-k_{s}\right)$ along the path $\mathscr{L}$. This can be split into the principle integration and a contribution from the pole. If the group velocity of the wave component $k_{s}$ is larger (smaller) than $U, \mathscr{L}$ passes over (under) the singularities at $\pm k_{s}$. Thus, the contribution from the pole at $k=k_{s}$ will be $-\mathrm{i} \pi f\left(k_{s}\right) e^{\mathrm{i} k_{s} x}$ or $+\mathrm{i} \pi f\left(k_{s}\right) e^{\mathrm{i} k_{s} x}$. If we use Wehausen \& Laitone (1960)

$$
\begin{equation*}
\lim _{|x| \rightarrow+\infty} \text { P.V. } \int_{-\infty}^{+\infty} \frac{f(k)}{k-k_{s}} e^{\mathrm{i} k x} d k=\operatorname{sgn}(x) \mathrm{i} \pi f\left(k_{s}\right) e^{\mathrm{i} k_{s} x} \tag{3.28}
\end{equation*}
$$

in the integral in $\mathcal{Z}_{n, 2}$, we can find that the radiation condition (2.12) is satisfied.
It may be of interest to see that the Green function is symmetric about source and filed points, or $G\left(x, y, z, x_{0}, y_{0}, z_{0}\right)=G\left(x_{0}, y_{0}, z_{0}, x, y, z\right)$, which is shown in appendix B .

### 3.2. Multipole expansion for the horizontal circular cylinder

The Green function derived above can be used to convert the governing equation to an integral equation over the surface of a body with arbitrary shape. For some special geometries, such as a sphere, the solution may be found through expansion in terms of the multipole obtained through differentiating the Green function with respect to the position of the source (see example $\mathrm{Wu}(1998 b)$ ). For a horizontal circular cylinder, the potential can be expanded into the cosine series as used for $G$. The governing Laplace equation in $(x, y, z)$ then becomes the modified Helmholtz equation in $(x, z)$ for a circular section. Subsequently, the solution of the modified Helmholtz equation can be obtained from a two-dimensional multipole expansion. To construct that, we may apply a source distribution $\varsigma\left(y_{0}\right)$ along the centre line of the cylinder. This is then expanded into the cosine series, or $\varsigma\left(y_{0}\right)=\sum_{n=0}^{+\infty} \mathcal{V}_{n} \cos \sigma_{n}\left(y_{0}+b\right)$. It will create the following potential

$$
\begin{equation*}
\varphi=\sum_{n=0}^{+\infty} \mathcal{V}_{n} \int_{-b}^{b} G \cos \sigma_{n}\left(y_{0}+b\right) d y_{0} \tag{3.29}
\end{equation*}
$$

where $G$ is the Green function derived in the previous section. Substituting (3.24) into (3.29), we obtain

$$
\begin{equation*}
\varphi=\sum_{n=0}^{+\infty} \varphi_{n} \cos \sigma_{n}(y+b) \tag{3.30}
\end{equation*}
$$

where

$$
\begin{equation*}
\varphi_{n}=\varphi_{n, 1}+\varphi_{n, 2} \tag{3.31}
\end{equation*}
$$

with

$$
\begin{equation*}
\varphi_{n, 1}=\frac{\mathcal{V}_{n}}{2 \pi}\left\{\delta_{n 0}\left[\ln \left(\frac{r}{H}\right)+\ln \left(\frac{r^{\prime}}{H}\right)\right]-\left(1-\delta_{n 0}\right)\left[\mathscr{K}_{0}\left(\sigma_{n} r\right)+\mathscr{K}_{0}\left(\sigma_{n} r^{\prime}\right)\right]\right\} \tag{3.32a}
\end{equation*}
$$

$$
\begin{equation*}
\varphi_{n, 2}=\frac{\rho U}{2 \pi} \int_{\mathscr{L}} \sum_{n^{\prime}=0}^{+\infty} \frac{\mathcal{V}_{n^{\prime}} \mu_{n, n^{\prime}} k \cosh K_{n^{\prime}}\left(z_{0}+H\right)\left[\cosh K_{n}(z+H) e^{i k\left(x-x_{0}\right)}-\delta_{n 0}\right]}{|\mathcal{A}| \Delta_{n^{\prime}} K_{n} \sinh K_{n} H \cosh K_{n^{\prime}} H} d k \tag{3.32b}
\end{equation*}
$$

We may use the operator

$$
\begin{equation*}
\left(D_{ \pm}\right)^{m}=-\frac{1}{2^{m-1}(m-1)!}\left(\frac{\partial}{\partial z_{0}} \pm \mathrm{i} \frac{\partial}{\partial x_{0}}\right)^{m} . \tag{3.33}
\end{equation*}
$$

This gives (Linton \& McIver 2001)

$$
\begin{equation*}
\left(D_{ \pm}\right)^{m} \ln r=\frac{e^{ \pm \mathrm{i} m \theta}}{r^{m}} \tag{3.34a}
\end{equation*}
$$

$$
\begin{equation*}
\left(D_{ \pm}\right)^{m} \mathscr{K}_{0}\left(\sigma_{n} r\right)=\frac{(-1)^{m} \sigma_{n}^{m}}{2^{m-1}(m-1)!} e^{ \pm \mathrm{i} m \theta} \mathscr{K}_{m}\left(\sigma_{n} r\right), \tag{3.34b}
\end{equation*}
$$

where $z-z_{0}=r \cos \theta$ and $x-x_{0}=r \sin \theta$. As mentioned in Li et al. (2019), because $\varphi$ is a real function, we may apply only the operator $\left(D_{+}\right)^{m}$. Using

$$
\begin{equation*}
\left(D_{+}\right)^{m} e^{ \pm K_{n} z_{0} \pm i k x_{0}}=-\frac{( \pm 1)^{m}\left(K_{n}-k\right)^{m}}{2^{m-1}(m-1)!} e^{ \pm K_{n} z_{0} \pm i k x_{0}} \tag{3.35a}
\end{equation*}
$$

$$
\begin{equation*}
\left(D_{+}\right)^{m} e^{ \pm K_{n} z_{0} \mp \mathrm{i} k x_{0}}=-\frac{( \pm 1)^{m}\left(K_{n}+k\right)^{m}}{2^{m-1}(m-1)!} e^{ \pm K_{n} z_{0} \mp \mathrm{i} k x_{0}} \tag{3.35b}
\end{equation*}
$$

as weel as (3.34), the velocity potential of the multipole can be expressed as

$$
\begin{gather*}
\left(\varphi_{+}\right)^{m}=\left(D_{+}\right)^{m} \varphi=\sum_{n=0}^{+\infty}\left(\varphi_{+}\right)_{n}^{m} \cos \sigma_{n}(y+b),  \tag{3.36}\\
\left(\varphi_{+}\right)_{n}^{m}=\left(\varphi_{+}\right)_{n, 1}^{m}+\left(\varphi_{+}\right)_{n, 2}^{m}+\left(\varphi_{+}\right)_{n, 3}^{m}, \tag{3.37}
\end{gather*}
$$

where $\left(\varphi_{+}\right)_{n, 1}^{m}+\left(\varphi_{+}\right)_{n, 2}^{m}=\left(D_{+}\right)^{m} \varphi_{n, 1},\left(\varphi_{+}\right)_{n, 3}^{m}=\left(D_{+}\right)^{m} \varphi_{n, 2}$ and can be obtained as

$$
\begin{equation*}
\left(\varphi_{+}\right)_{n, 1}^{m}=\frac{\mathcal{V}_{n}}{2 \pi}\left[\delta_{n 0} \frac{e^{\mathrm{i} m \theta}}{r^{m}}-\left(1-\delta_{n 0}\right) \frac{(-1)^{m} \sigma_{n}^{m}}{2^{m-1}(m-1)!} e^{\mathrm{i} m \theta} \mathscr{K}_{m}\left(\sigma_{n} r\right)\right], \tag{3.38a}
\end{equation*}
$$

$$
\begin{gather*}
\left(\varphi_{+}\right)_{n, 2}^{m}=\frac{(-1)^{m} \mathcal{V}_{n}}{2^{m+1}(m-1)!\pi} \int_{-\infty}^{+\infty} \frac{\left(K_{n}-k\right)^{m} e^{-K_{n}\left(z+z_{0}+2 H\right)+\mathrm{i} k\left(x-x_{0}\right)}}{K_{n}} d k,  \tag{3.38b}\\
\left(\varphi_{+}\right)_{n, 3}^{m}=\frac{-\rho U}{2^{m+1}(m-1)!\pi} \int_{\mathscr{L}} \sum_{n^{\prime}=0}^{+\infty} \frac{\mathcal{V}_{n^{\prime}} \mu_{n, n^{\prime}} k \cosh K_{n}(z+H) E_{n^{\prime}, m}\left(k, z_{0}\right) e^{\mathrm{i} k\left(x-x_{0}\right)}}{|\mathcal{A}| \Delta_{n^{\prime}} K_{n} \sinh K_{n} H \cosh K_{n^{\prime}} H} d k, \tag{3.38c}
\end{gather*}
$$

with

$$
\begin{equation*}
E_{n^{\prime}, m}\left(k, z_{0}\right)=\left(K_{n^{\prime}}+k\right)^{m} e^{K_{n^{\prime}}\left(z_{0}+H\right)}+(-1)^{m}\left(K_{n^{\prime}}-k\right)^{m} e^{-K_{n^{\prime}}\left(z_{0}+H\right)} . \tag{3.39}
\end{equation*}
$$

The potential $\phi$ due to the cylinder then can be written in a multipole expansion form as

$$
\begin{equation*}
\phi=\operatorname{Re}\left\{\sum_{m=1}^{+\infty} r_{0}^{m} g_{m}\left(\varphi_{+}\right)^{m}\right\} . \tag{3.40}
\end{equation*}
$$

It then satisfies all the boundary conditions met by the Green function and the only remaining

It should be mentioned here that a new unknown coefficient $f_{n, m}$ is defined as $f_{n, m}=\mathcal{V}_{n} g_{m}$. The impermeable condition on the body surface in (2.8) gives

$$
\begin{align*}
& Q_{n, m}^{\prime}\left(r_{0}\right) f_{n, m}+T_{n, m}^{\prime}\left(r_{0}\right) \sum_{m^{\prime}=1}^{+\infty}\left(f_{n, m^{\prime}} C_{n, m, m^{\prime}}^{+}+\bar{f}_{n, m^{\prime}} \bar{C}_{n, m, m^{\prime}}^{-}\right) \\
& +T_{n, m}^{\prime}\left(r_{0}\right) \sum_{n^{\prime}=0}^{+\infty} \sum_{m^{\prime}=1}^{+\infty}\left(f_{n^{\prime}, m^{\prime}} \mathcal{D}_{n, n^{\prime}, n, m^{\prime}}^{+}+\bar{f}_{n^{\prime}, m^{\prime}} \overline{\mathcal{D}}_{n, n^{\prime}, m, m^{\prime}}^{-}\right)=-\mathrm{i} \delta_{n 0} \delta_{m 1} U \tag{3.45}
\end{align*}
$$

and $\mathscr{I}_{m}$ denotes the $m$ th-order modified Bessel function of first kind, we have

$$
\phi=\operatorname{Re}\left\{\begin{array}{c}
\sum_{n=0}^{+\infty} \sum_{m=0}^{+\infty}\left[\begin{array}{l}
Q_{n, m} f_{n, m} e^{\mathrm{i} m \theta}+T_{n, m} \sum_{m^{\prime}=1}^{+\infty} f_{n, m^{\prime}}\left(C_{n, m, m^{\prime}}^{+} e^{\mathrm{i} m \theta}+C_{n, m, m^{\prime}}^{-} e^{-\mathrm{i} m \theta}\right) \\
+T_{n, m} \sum_{n^{\prime}=0}^{+\infty} \sum_{m^{\prime}=1}^{+\infty} f_{n^{\prime}, m^{\prime}}\left(\mathcal{D}_{n, n^{\prime}, m, m^{\prime}}^{+} e^{\mathrm{i} m \theta}+\mathcal{D}_{n, n^{\prime}, m, m^{\prime}}^{-} e^{-\mathrm{i} m \theta}\right)
\end{array}\right]  \tag{3.43}\\
\times \cos \sigma_{n}(y+b)
\end{array}\right\},
$$

where

$$
\begin{equation*}
Q_{n, m}(r)=\frac{\left(1-\delta_{m 0}\right)}{2 \pi}\left[\delta_{n 0}\left(\frac{r_{0}}{r}\right)^{m}-\frac{\left(1-\delta_{n 0}\right)\left(-\sigma_{n} r_{0}\right)^{m}}{2^{m-1}(m-1)!} \mathscr{K}_{m}\left(\sigma_{n} r\right)\right], \tag{3.44a}
\end{equation*}
$$

$$
\begin{equation*}
C_{n, m, m^{\prime}}^{ \pm}=\frac{(-1)^{m+m^{\prime}} r_{0}^{m^{\prime}}}{2^{m^{\prime}+1}\left(m^{\prime}-1\right)!\pi} \int_{-\infty}^{+\infty} \frac{\left(K_{n}-k\right)^{m^{\prime}} e^{-2 K_{n}\left(z_{0}+H\right)} A_{n, m}^{\mp}}{K_{n}} d k \tag{3.44b}
\end{equation*}
$$

$\mathcal{D}_{n, n^{\prime}, m, m^{\prime}}^{ \pm}=\frac{-\rho U r_{0}^{m^{\prime}}}{2^{m^{\prime}+2}\left(m^{\prime}-1\right)!\pi} \int_{\mathscr{L}} \frac{\mu_{n, n^{\prime} k} k\left[e^{K_{n}\left(z_{0}+H\right)} A_{n, m}^{ \pm}+(-1)^{m} e^{-K_{n}\left(z_{0}+H\right)} A_{n, m}^{\mp}\right] E_{n^{\prime}, m^{\prime}}}{\Delta_{n^{\prime}}|\mathcal{A}| K_{n} \sinh K_{n} H \cosh K_{n^{\prime}} H} d k$.
where $n=0,1,2 \ldots$ and $m=1,2,3 \ldots Q_{n, m}^{\prime}\left(T_{n, m}^{\prime}\right)$ represents the derivative of $Q_{n, m}\left(T_{n, m}\right)$
with respect to $r$, which can be obtained as

$$
\left.\begin{array}{rl}
Q_{n, m}^{\prime}(r) & =\frac{\left(1-\delta_{m 0}\right)}{2 \pi}\left[-\delta_{n 0} m \frac{r_{0}^{m}}{r^{m+1}}+\frac{\left(1-\delta_{n 0}\right)\left(-\sigma_{n} r_{0}\right)^{m+1}}{2^{m-1}(m-1)!r_{0}} \mathscr{K}_{m}^{\prime}\left(\sigma_{n} r\right)\right]  \tag{3.46}\\
T_{n, m}^{\prime}(r) & =\delta_{n 0} \frac{r^{m-1}}{(m-1)!}+\left(1-\delta_{n 0}\right) \sigma_{n} \mathscr{I}_{m}^{\prime}\left(\sigma_{n} r\right)
\end{array}\right\} .
$$

After (3.45) is solved, $\phi$ can then be obtained.

### 3.3. Hydrodynamic forces

Once the potential is found, the lift $F_{L}$ and resistance $F_{R}$ on the cylinder can be calculated through the integration of hydrodynamic pressure over the body surface. Thus, we have:

$$
\begin{equation*}
-\mathrm{i} F_{R}+F_{L}=-\iint_{S_{B}} p e^{\mathrm{i} \theta} d S \tag{3.47}
\end{equation*}
$$

The pressure $p$ can be obtained by the Bernoulli equation

$$
\begin{equation*}
p=-\frac{1}{2} \rho \nabla(\phi-U x) \cdot \nabla(\phi-U x) . \tag{3.48}
\end{equation*}
$$

We may notice that the product term is kept here, as it may be small on the ice sheet but may not be on the body surface. When determining the gradient term in (3.48) in the cylindrical coordinate system, we may substitute (3.45) into (3.43) and have

$$
\begin{align*}
& \left.\frac{\partial(\phi-U x)}{\partial r}\right|_{r=r_{0}}=0 \\
& \left.\frac{\partial(\phi-U x)}{\partial \theta}\right|_{r=r_{0}}=\operatorname{Re}\left\{\sum_{n=0}^{+\infty} \sum_{m=0}^{+\infty} \mathrm{i} m \psi_{n, m} e^{\mathrm{i} m \theta} \cos \sigma_{n}(y+b)\right\}  \tag{3.49}\\
& \left.\frac{\partial(\phi-U x)}{\partial y}\right|_{r=r_{0}}=-\operatorname{Re}\left\{\sum_{n=0}^{+\infty} \sum_{m=0}^{+\infty} \sigma_{n} \psi_{n, m} e^{\mathrm{i} m \theta} \sin \sigma_{n}(y+b)\right\},
\end{align*}
$$

where

$$
\begin{equation*}
\psi_{n, m}=f_{n, m}\left[Q_{n, m}\left(r_{0}\right)-Q_{n, m}^{\prime}\left(r_{0}\right) \frac{T_{n, m}\left(r_{0}\right)}{T_{n, m}^{\prime}\left(r_{0}\right)}\right] . \tag{3.50}
\end{equation*}
$$

Substituting (3.48) and (3.49) into (3.47), we obtain

$$
\begin{equation*}
-\mathrm{i} F_{R}+F_{L}=\frac{\pi \rho b}{2 r_{0}} \sum_{n=0}^{+\infty} \sum_{m=0}^{+\infty}\left[\left(1+\delta_{n 0}\right) m(m+1)+\left(1-\delta_{n 0}\right) \sigma_{n}^{2} r_{0}^{2}\right] \psi_{n, m} \bar{\psi}_{n, m+1} \tag{3.51}
\end{equation*}
$$

Similar to that in Wu (1995), the resistance can be also obtained by far-field integral, which is shown in appendix C.

### 3.4. Deflection of the ice sheet

We may use (2.7) to obtain the expression of $\eta$, and this gives

$$
\begin{equation*}
\eta(x, y)=-\frac{1}{U} \int \frac{\partial \phi}{\partial z} d x+C(y) \tag{3.52}
\end{equation*}
$$

Substituting (3.36) (3.40) into (3.52), we have

$$
\begin{equation*}
\eta=\operatorname{Im}\left\{\sum_{m=1}^{+\infty} \sum_{n=0}^{+\infty} r_{0}^{m} \eta_{n, m} \cos \sigma_{n}(y+b)\right\}+C(y) \tag{3.53}
\end{equation*}
$$

$$
\begin{equation*}
\eta_{n, m}=\eta_{n, m}^{(1)}+\eta_{n, m}^{(2)}+\eta_{n, m}^{(3)}, \tag{3.54}
\end{equation*}
$$

where

$$
\begin{equation*}
\eta_{n, m}^{(1)}=\frac{f_{n, m}}{2^{m+1}(m-1)!\pi U} \int_{-\infty}^{+\infty} \frac{\left(K_{n}+k\right)^{m} e^{K_{n} z_{0}+i k\left(x-x_{0}\right)}}{k} d k, \tag{3.55a}
\end{equation*}
$$

$$
\begin{equation*}
\eta_{n, m}^{(1)}=\frac{(-1)^{m} f_{n, m}}{2^{m+1}(m-1)!\pi U} \int_{-\infty}^{+\infty} \frac{\left(K_{n}-k\right)^{m} e^{-K_{n}\left(z_{0}+2 H\right)+\mathrm{i} k\left(x-x_{0}\right)}}{k} d k \tag{3.55b}
\end{equation*}
$$

$$
\begin{equation*}
\eta_{n, m}^{(3)}=\frac{\rho}{2^{m+1}(m-1)!\pi} \int_{\mathscr{L}} \sum_{n^{\prime}=0}^{+\infty} \frac{f_{n^{\prime}, m} \mu_{n, n^{\prime}} E_{n^{\prime}, m}\left(k, z_{0}\right) e^{i k\left(x-x_{0}\right)}}{\Delta_{n^{\prime}}|\mathcal{A}| \cosh K_{n^{\prime}} H} d k . \tag{3.55c}
\end{equation*}
$$

$C(y)$ in (3.52) is the integration constant. As $C(y)$ is not function of $x$, we may determine it at $x \rightarrow+\infty$. Based on (3.36) (3.40) and (3.53) $\sim(3.55)$, the asymptotic expressions of $\phi$ and $\eta$ at $x \rightarrow+\infty$ can be written as

$$
\begin{equation*}
\phi=\operatorname{Re}\left\{\sum_{j=1}^{S} \phi^{(j)}(y, z) e^{-\mathrm{i} k_{j} x}\right\}+\operatorname{sgn}(x) \phi^{(0)}(y, z), \tag{3.56}
\end{equation*}
$$

$$
\begin{equation*}
\eta=\frac{1}{U} \operatorname{Re}\left\{\sum_{j=1}^{S} \frac{1}{\mathrm{i} k_{j}} \frac{\partial \phi^{(j)}(y, 0)}{\partial z} e^{-\mathrm{i} k_{j} x}\right\}+C(y), \tag{3.57}
\end{equation*}
$$

where $\phi^{(0)}$ is due to the singularity of the integrand at $k=0$ and is related to the 'blockage parameter' (Mei \& Chen 1976). It should be noted that the summation in (3.56) and (3.57) should include only those terms with group velocity larger than $U$. It can be shown $\partial \phi^{(0)} / \partial z=$ 0 on $z=0$ and therefore it does not contribute to $\eta$. We may combine (2.6) and (2.7) to eliminate $\eta$, and then use (3.56) in the result. We have

$$
\begin{equation*}
\left[L\left(k_{j}^{4}-2 k_{j}^{2} \frac{\partial^{2}}{\partial y^{2}}+\frac{\partial^{4}}{\partial y^{4}}\right)+\rho g\right] \frac{\partial \phi^{(j)}}{\partial z}-\rho U^{2} k_{j}^{2} \phi^{(j)}=0, z=0, x \rightarrow+\infty, j=1 \sim S . \tag{3.58}
\end{equation*}
$$

Substituting (3.56) and (3.57) into (2.6), and using (3.58), we obtain

$$
\begin{equation*}
L \frac{d^{4} C}{d y^{4}}+\rho g C=0 \tag{3.59}
\end{equation*}
$$

The boundary conditions of $C$ can be established by substituting (3.57) into (2.11) which is satisfied by the first term on the right-hand side of (3.57). This gives at $y= \pm b$

$$
\left.\begin{array}{l}
C=0, \quad C_{y}=0, \quad \text { Clamped edge }  \tag{3.60}\\
C=0, \quad C_{y y}=0, \quad \text { Simply supported edge } \\
C_{y y}=0, \quad C_{y y y}=0, \quad \text { Free edge }
\end{array}\right\} .
$$

It can be found that $C(y)=0$ under any of these edge conditions.

### 3.5. Numerical procedure

Although the expression for the potential has been written explicitly, some of the computations still have to be performed numerically. Taking into account the integrand decays exponentially, the integration for $k$ from $-\infty$ to $+\infty$ is truncated at a sufficiently large
value $K_{T}$ and treated as

$$
\begin{equation*}
\text { P.V. } \int_{-\infty}^{+\infty} F(k) d k=\text { P.V. } \int_{0}^{+\infty}[F(k)+F(-k)] d k \approx \text { P.V. } \int_{0}^{K_{T}}[F(k)+F(-k)] d k, \tag{3.61}
\end{equation*}
$$

P.V. indicates the Cauchy principal value. The range $\left(0, K_{T}\right)$ is divided into many small steps and Gaussian method is used in each step for integration. To deal with multiple singularities in the integrand, the following numerical procedures are applied. Assume that $F(k)$ contains $n$ first-order singularities in $k \in(0,+\infty)$. We may write

$$
\begin{equation*}
F(k)=\frac{G(k)}{\prod_{i=1}^{n}\left(k-k_{i}\right)}=G(k) \sum_{i=1}^{n} \frac{1}{\prod_{j=1(j \neq i)}^{n}\left(k_{i}-k_{j}\right)} \times \frac{1}{k-k_{i}}, \tag{3.62}
\end{equation*}
$$

Its Cauchy principal value can be calculated through

$$
\begin{equation*}
\text { P.V. } \int_{0}^{+\infty} F(k) d k=\sum_{i=1}^{n} \frac{1}{\prod_{j=1(j \neq i)}^{n}\left(k_{i}-k_{j}\right)}\left[\int_{0}^{2 k_{i}} \frac{G(k)-G\left(k_{i}\right)}{k-k_{i}} d k+\int_{2 k_{i}}^{+\infty} \frac{G(k)}{k-k_{i}} d k\right], \tag{3.63}
\end{equation*}
$$

where $G(k)$ can be calculated by

$$
\left.\begin{array}{ll}
G(k)=F(k) \prod_{i=1}^{n}\left(k-k_{i}\right), & k \neq k_{i}  \tag{3.64}\\
G\left(k_{i}\right)=\prod_{j=1(j \neq i)}^{n}\left(k_{i}-k_{j}\right) \lim _{k \rightarrow k_{i}}\left(k-k_{i}\right) F(k), & i=1 \sim n
\end{array}\right\} .
$$

By doing that, all the singularity effects are eliminated. Although the original one integral is split into $n$ integrals, it is still more computationally efficient, as the integration step $\Delta k$ can be appropriately chosen for each integral. For a single integral of $F(k), \Delta k$ has to be very small, especially when some of the singularities are very close to each other.

When determining the residue at the singularities caused by $|\mathcal{A}|\left(k_{s}\right)=0$, the value of $|\mathcal{A}|^{\prime}\left(k_{s}\right)$ needs to be calculated numerically. Here we adopt the method proposed by Li et al. (2020b). Assuming $f(k) /\left(k-k_{s}\right)=g(k) /|\mathcal{A}|(k)=P(k)$, we have

$$
\begin{equation*}
f\left(k_{s}\right)=\frac{g\left(k_{s}\right)}{|\mathcal{A}|^{\prime}\left(k_{s}\right)}=\lim _{k \rightarrow k_{s}}\left(k-k_{s}\right) P(k) \approx \frac{P\left(k_{s}+\Delta k\right)-P\left(k_{s}-\Delta k\right)}{2} \Delta k \tag{3.65}
\end{equation*}
$$

## 4. Numerical Results

The typical values of the physical parameters of the ice sheet and the channel are chosen as below

$$
\left.\begin{array}{l}
\rho_{i}=917 \mathrm{~kg} \mathrm{~m}^{-3}, \quad h_{i}=0.15 \mathrm{~m}, \quad E=4.2 \times 10^{9} \mathrm{~N} \mathrm{~m}^{-2}, \quad v=0.3  \tag{4.1}\\
\rho=1000 \mathrm{~kg} \mathrm{~m}^{-3}, \quad g=9.81 \mathrm{~m} \mathrm{~s}^{-2}, \quad H=10 \mathrm{~m}, \quad b=20 \mathrm{~m}, \quad r_{0}=1 \mathrm{~m}
\end{array}\right\} .
$$

The calculations undertaken below are based on these parameters unless otherwise specified. All the numerical results are presented in the dimensionless form, based on a characteristic length, the fluid density $\rho$ and acceleration $g$ due to gravity. The depth-based Froude number is defined as $F n=U / \sqrt{g H}$.

### 4.1. Analysis of the dispersion relationship of an ice-covered channel

As discussed after (3.24), there are singularities in the integrand when $|\mathcal{A}|(k, U)=$ $|\mathcal{A}|(\hat{k}, F n)=0($ where $\hat{k}=k H)$, which are equivalent to the dispersion relationship


Figure 2: Purely positive solution of $|\mathcal{A}|(\hat{k}, F n)=0$ for clamped-clamped edges condition.
and correspond to the waves at infinity. Compared with the two-dimensional case of a homogeneous ice plate with infinite extent (Li et al. 2019), the dispersion relationship here is more complicated. The root $F n$ of the equation at each given $\hat{k}$ is not unique. In fact, there is an infinite number of solutions of $F n$ at a given $\hat{k}$, we denote this these solutions as $F n^{(i)}(i=1,2,3 \ldots)$, with $F n^{(1)}<F n^{(2)}<F n^{(3)}<\ldots$ Similar to that in Khabakhpasheva et al. (2019), the curves $F n^{(i)}$ against $\hat{k}$ are shown in figure 2. It can be seen that for each curve there is a minimum value at $\hat{k}=\hat{k}_{c}^{(i)}$, which can be called $i$ th-order critical Froude number and be denoted by $F n_{c}^{(i)}$. The value of each $F n_{c}^{(i)}$ can be found in figure 2. When $F n<F n_{c}^{(1)}$, there will be no solution in $|\mathcal{A}|(\hat{k}, F n)=0$, or there will be no waves propagating to $x= \pm \infty$. When $F n_{c}^{(1)}<F n<F n_{c}^{(2)}$, there will be two solutions in $|\mathcal{A}|(\hat{k}, F n)=0$. The ones corresponding to $\hat{k}<\hat{k}_{c}^{(1)}$ and $\hat{k}>\hat{k}_{c}^{(1)}$ will lead to waves at $x=-\infty$ and $x=+\infty$ respectively, because their group velocities are smaller and larger than $U$ respectively, as in Li et al. (2019). In fact, the nondimensionalized group velocity $c_{g}^{(s)}$ can be obtained as

$$
\begin{equation*}
c_{g}^{(s)}=F n\left(\hat{k}_{s}\right)+\hat{k}_{s} \frac{d}{d \hat{k}} F n\left(\hat{k}_{s}\right), \tag{4.2}
\end{equation*}
$$

where $\left(\hat{k}_{s}, F n\left(\hat{k}_{s}\right)\right)$ is an intersection point of $F n$ with curve $\hat{k}-F n^{(i)}$. It can be observed from figure 2 and (4.2) that when $\hat{k}_{s}<\hat{k}_{c}^{(i)}\left(\hat{k}_{s}>\hat{k}_{c}^{(i)}\right)$, the slope $d F n\left(\hat{k}_{s}\right) / d \hat{k}<0\left(d F n\left(\hat{k}_{s}\right) / d \hat{k}>\right.$ 0 ), and the group velocity $c_{g}^{(s)}<F n\left(\hat{k}_{s}\right)\left(c_{g}^{(s)}>F n\left(\hat{k}_{s}\right)\right)$ and wave will be at $x=-\infty$ $(x=+\infty)$.

As $F n$ increases, it will reach the second critical point $F n_{c}^{(2)}$. Beyond that, there will be two more solutions in $|\mathcal{A}|(\hat{k}, F n)=0$. This leads to two more waves and one each at $x=-\infty$ and $x=+\infty$. In general, when $F n_{c}^{(i)}<F n<F n_{c}^{(i+1)}$, there will be $2 i$ roots in $|\mathcal{A}|(\hat{k}, F n)=0$, and $i$ waves at $x=-\infty$ and $x=+\infty$ respectively. However, the curve $F n_{c}^{(1)}$ is different from


Figure 3: Purely positive solution of $|\mathcal{A}|(\widetilde{k}, \widetilde{F n})=0$ for clamped-clamped edges at $H \rightarrow+\infty$.
others. When $\hat{k} \rightarrow 0$, we can see that $\lim _{\hat{k} \rightarrow 0} F n^{(1)}=F n_{0}$ while $\lim _{\hat{k} \rightarrow 0} F n^{(i)}=+\infty(i \geqslant 2)$. Therefore, when $F n>F n_{0}$, corresponding to curve $F n^{(1)}$, there will be only one solution in $|\mathcal{A}|(\hat{k}, F n)=0$, and there will be $2 i-1$ solutions when $F n_{c}^{(i)}<F n<F n_{c}^{(i+1)}$.

As mentioned in Section 3.1, the symmetric and anti-symmetric transverse waves are completely independent in the cases of symmetric edges. If we further write $|\mathcal{A}|$ as in (A 10), it can be found that all the intersection points on curve $F n^{(2 i-1)}(i \geqslant 1)$ in figure 2 are solutions of $\left|\mathcal{A}^{S}\right|(\hat{k}, F n)=0$ and correspond to symmetric mode, while those on curve $F n^{(2 i)}$ are solutions of $\left|\mathcal{A}^{\boldsymbol{A}}\right|(\hat{k}, F n)=0$ and correspond to anti-symmetric mode.
In the two-dimensional case, there is only one critical $F n_{c}$, below which there will be no wave, and above which will be two waves at $x=-\infty$ and $x=+\infty$ respectively. When $F n>1$, the wave at downstream region will disappear but the one at upstream region will remain. Here when there is an infinite number of critical Froude number. $F n_{c}^{(1)}$ is similar to $F n_{c}$ in the two-dimensional case, below which there will be no wave. As $F n$ increases and passes through each $F n^{(i)}(i=1,2,3 \ldots)$ each pair of waves will be created, with one at downstream and the other at upstream regions. $F n_{0}$ here corresponds to $F n=1$ in the two-dimensional case. When $F n>F n_{0}$, the wave at $x=-\infty$ due to $F n^{(1)}$ will disappear, which is similar to the two-dimensional case. However, different from the two-dimensional case, there will be still waves at downstream region.
The feature of wave in infinite water depth or $H \rightarrow+\infty$ is also investigated, other parameters in (4.1) are kept the same. For the two-dimensional case, the critical value $F n=1$ will disappear if $H \rightarrow+\infty$. There will always be two waves, with one each at downstream and upstream regions respectively when the current speed is larger than the critical value. Here, using $\lim _{H \rightarrow+\infty} \tanh K_{n} H=1, \alpha_{n, j}$ in (3.17a) can be further expressed



Figure 4: Critical Froude number under clamped-clamped edges condition. (a) $F n_{c}^{(i)}$ $(i=1 \sim 4)$ at different channel widths; $(b) F n_{c}^{(i)}(i=1 \sim 4)$ at different ice sheet thickness.
as

$$
\alpha_{n, j}=\frac{1}{\Delta_{n}^{\infty}}\left\{\begin{array}{l}
d_{n}^{(j)}\left[\left(\rho g+L k^{4}\right) K_{n}-\rho k^{2} U^{2}\right]  \tag{4.3}\\
-4 k^{2} L d_{n}^{(0)} K_{n} \delta_{2 j}-12 k^{2} L d_{n}^{(1)} K_{n} \delta_{3 j}
\end{array}\right\}+\left(1-\delta_{n 0}\right) \gamma_{n}^{\infty} \alpha_{0 j},
$$

where

$$
\Delta_{n}^{\infty}=\left\{\begin{array}{l}
\delta_{n 0}\left[-\left(\frac{\rho g+L k^{4}}{24} d_{n}^{(4)}-k^{2} L d_{n}^{(2)}+L\right) K_{n}+\frac{\rho k^{2} U^{2}}{24} d_{n}^{(4)}\right]  \tag{4.4a}\\
+\left(1-\delta_{n 0}\right)\left[-\left(\frac{\rho g+L k^{4}}{\sigma_{n}^{4}}+\frac{2 k^{2} L}{\sigma_{n}^{2}}+L\right) K_{n}+\frac{\rho k^{2} U^{2}}{\sigma_{n}^{4}}\right]
\end{array}\right\},
$$

$$
\begin{equation*}
\gamma_{n}^{\infty}=\frac{1}{\Delta_{n}^{\infty}}\left[\left(\frac{\rho g+L k^{4}}{24} d_{n}^{(4)}-k^{2} L d_{n}^{(2)}\right) K_{n}-\frac{\rho k^{2} U^{2}}{24} d_{n}^{(4)}\right] . \tag{4.4b}
\end{equation*}
$$

If the half-width $b$ is chosen as the characteristic length, two new parameters can be defined as $\widetilde{F n}=U / \sqrt{g b} \& \widetilde{k}=k b$. Substituting (4.3) into (A2), the solution of $|\mathcal{A}|(k, U)=$ $|\mathcal{A}|(\widetilde{k}, \widetilde{F n})=0$ at $H \rightarrow+\infty$ can be obtained and shown in figure 3. Similar to $F n_{c}^{(i)}$ in figure 2, the $i$ th-order critical value at $\widetilde{k}=\widetilde{k}_{c}^{(i)}$ is defined as $\widetilde{F n}_{c}^{(i)}$. It can be seen that the curve $\widetilde{F n}^{(1)}$ is different from $F n^{(1)}$ in figure 2. When $\widetilde{k} \rightarrow 0$, we can find that $\lim _{\widetilde{k} \rightarrow 0} \widetilde{F n}^{(1)}=+\infty$. Therefore, there will always be $2 i$ solutions in $|\mathcal{A}|(\widetilde{k}, \widetilde{F n})=0$ when $\widetilde{F n}_{c}^{(i)}<\widetilde{F n}<\widetilde{F n}_{c}^{(i+1)}$, which leads to $i$ waves at $x=-\infty$ and $x=+\infty$ respectively.

We may also investigate how the above results vary with the channel width $b$, ice sheet thickness $h_{i}$ and various edge constraints while other values in (4.1) remain unchanged. $F n_{c}^{(i)}$ $(i=1,2,3,4)$ at different $b$ and $h_{i}$ under clamped-clamped edges is shown in figure 4. It can be seen from figure $4(a)$ that all the $F n_{c}^{(i)}$ decrease when $b$ increases, and they all tend to the value of $F n_{c}$ without the tank wall, or the two-dimensional case (Li et al. 2019). In other words, when $b$ increases, the curves $F n^{(i)}$ in figure 2 will gradually move towards each other and approach the curve in the two-dimensional case. figure $4(b)$ shows that at $h_{i}=0$ all the $F n_{c}^{(i)}=0$, which is expected as this is a free surface problem. As $h_{i}$ increases, all the $F n_{c}^{(i)}$ increase. As $h_{i}$ becomes very large or $L$ is very large, the ice sheet becomes a rigid plate and there will be no waves, which means $F n_{c}^{(i)} \rightarrow+\infty$. In figures 5(a) and (b),


Figure 5: $F n_{c}^{(1)}$ and $F n_{0}$ under different edges conditions. (a) $F n_{c}^{(1)}$ at different channel widths; (b) $F n_{c}^{(1)}$ at different ice thickness; (c) $F n_{0}$ at different channel widths; (d) $F n_{0}$ at different ice thickness. The clamped edge is denoted by C , simply supported edge is by SS and the free edge is by F (The same below).
$F n_{c}^{(1)}$ under other edges conditions show variation trends similar to that in figure 4, apart from the free-free one where $F n_{c}^{(1)}$ increases slightly with $b$. $F n_{0}$ versus with $b$ and $h_{i}$ are given in figures $5(c)$ and $(d)$ respectively. It can be shown that under free-free condition, $F n_{0}=1$, which can be confirmed by substituting $\hat{k}=0$ and $F n=1$ in $|\mathcal{A}|(\hat{k}, F n)$. This is the same as the two-dimensional case. Under other edges conditions $F n_{0}$ is different from 1. It varies with $b$ in a way similar to that of $F n_{c}^{(1)}$. It can be observed that at sufficiently large $b$, $F n_{0} \rightarrow 1$, which is consistent with the two-dimensional case. Also $\lim _{h_{i} \rightarrow 0} F n_{0}=1$, which is consistent with the free surface problem. Another interesting feature in figure 5 is when all the parameters are fixed, $F n_{c}^{(1)}$ and $F n_{0}$ increase in the following sequence: free-free, simply supported-free, clamped-free, simply supported-simply supported, clamped-simply supported and clamped-clamped.

### 4.2. Deflection of the ice sheet due to an underwater source

Numerical results are first given for the ice sheet deflection $\xi$ induced by a submerged source. The quiescent water depth $H$ is used as the characteristic length. $\xi$ can be obtained directly by using inverse Fourier transformation

$$
\begin{equation*}
\xi=\frac{1}{2 \pi} \int_{-\infty}^{+\infty} \hat{\xi} e^{i k x} d k \tag{4.5}
\end{equation*}
$$



Figure 6: Ice sheet deflection $\xi$ along $y$-axis due to a source when the ice sheet is under clamped-clamped edges condition. (a) $\left(x_{0}, y_{0}, z_{0}\right)=(0,0,-H / 5) ;(b)$ $\left(x_{0}, y_{0}, z_{0}\right)=(0,-b / 2,-H / 5)$. The Froude numbers in the figure are in the following order: $F n_{c}^{(1)}=0.8180<0.9<F n_{c}^{(2)}=0.9981<1.05<F n_{c}^{(3)}=1.3177<1.4<$ $F n_{c}^{(4)}=1.7682<1.8<F n_{c}^{(5)}=2.3230<2.4<F n_{c}^{(6)}=2.9600$.

Substituting (3.7) into (4.5) and using (3.24) ~ (3.27), we have

$$
\begin{equation*}
\xi=\frac{\mathrm{i} \rho}{2 \pi b} \int_{\mathscr{L}} \sum_{n=0}^{+\infty} \sum_{m=0}^{+\infty} \frac{\lambda_{n, m} \cosh K_{m}\left(z_{0}+H\right) e^{\mathrm{i} k\left(x-x_{0}\right)} \cos \sigma_{n}(y+b) \cos \sigma_{m}\left(y_{0}+b\right)}{\left(1+\delta_{m 0}\right) \Delta_{m}|\mathcal{A}| \cosh K_{m} H} d k \tag{4.6}
\end{equation*}
$$

where

$$
\begin{equation*}
\lambda_{n, m}=\sum_{j=0}^{3} \beta_{n, j} c_{m, j}^{\prime}+\delta_{m 0}|\mathcal{A}| U k\left(\frac{d_{n}^{(4)}}{24}+\frac{\left(1-\delta_{n 0}\right) \gamma_{n}}{\sigma_{n}^{4}}\right)+\delta_{n m}\left(1-\delta_{n 0}\right) \frac{U k|\mathcal{A}|}{\sigma_{n}^{4}} . \tag{4.7}
\end{equation*}
$$

It should be noted that the infinite series in (4.6) is truncated at $n=m=15$ in the calculation, and the results have been found to have converged.
The ice deflection along $y$-axis induced by a source at two different locations under clamped-clamped edges are shown in figure 6 . In figure $6(a)$, the source is located at $\left(x_{0}, y_{0}, z_{0}\right)=(0,0,-H / 5)$. This corresponds to a symmetric case about $y=0$, or only waves correspond to even $m$ in (4.6) will exist. As $x=0$ and $x_{0}=0$, the principal integral of $\xi$ in (4.6) is zero. The remaining waves are those components which will propagate to $x=+\infty$ and $x=-\infty$ respectively. It can be seen from figure $6(a)$ that the wave profiles at $F n=0.9$ and $F n=1.05$ are dominated by the first symmetric transverse mode $\left(\sigma_{2}\right)$, and at $F n=1.4$ and $F n=1.8$ the second symmetric mode $\left(\sigma_{4}\right)$ become important, further at $F n=2.4$, the third symmetric mode $\left(\sigma_{6}\right)$ also becomes important. In general, when $F n_{c}^{(2 m-1)}<F n<F n_{c}^{(2 m+1)}$ ( $m \geqslant 1$ ), the contribution of the $m$-th symmetric mode ( $\sigma_{2 m}$ ) to the overall wave profile will be significant. As $c_{1}=c_{3}=0$ and $a_{2 n-1}=b_{2 n-1}=0$ in (3.5) and (3.9), the singularities corresponding to $\left|\mathcal{A}^{\boldsymbol{A}}\right|(\hat{k}, F n)=0$ (see (A 6)) in the solution for these coefficients are no longer relevant. Therefore, the curves of $F n^{(2 i)}(\hat{k})$ can be removed from figure 2 in such a case. The transverse wave profiles with the source located at $\left(x_{0}, y_{0}, z_{0}\right)=(0,-b / 2,-H / 5)$ are shown in figure $6(b)$, which is an asymmetric case. In such a case, all the longitudinal wave components of $k_{j}(j=1 \sim S)$ in (4.6) exist, and all the transverse waves of $\sigma_{m}$ will also exist. The overall transverse wave in this case is asymmetric.
The hydroelastic waves along the $x$-axis due to a source at $\left(x_{0}, y_{0}, z_{0}\right)=(0,0,-H / 5)$ are given in figure 7. It can be seen from figure 7(a) that there is no wave propagating to $x= \pm \infty$ when $F n<F n_{c}^{(1)}$. The longitudinal wave profile is anti-symmetric about $x=x_{0}$, which can


Figure 7: Ice sheet deflection $\xi$ along $x$-axis due to a source located at $\left(x_{0}, y_{0}, z_{0}\right)=(0,0,-H / 5)$ when the ice sheet is under clamped-clamped edges condition.
(a) $F n<F n_{c}^{(1)}$; (b) $F n_{c}^{(1)}<F n<F n_{0}$; (c) $F n_{c}^{(2)}<F n<F n_{c}^{(3)}$; (d) $F n_{c}^{(4)}<F n<F n_{c}^{(5)}$. $F n_{c}^{(1)}=0.8180, F n_{c}^{(2)}=0.9981, F n_{c}^{(3)}=1.3177, F n_{c}^{(4)}=1.7682, F n_{c}^{(5)}=2.3230$, $F n_{0}=1.1469$.


Figure 8: Ice sheet deflection $\xi$ along $y=-b / 2$ due to a source located at $\left(x_{0}, y_{0}, z_{0}\right)=(0,-b / 2,-H / 5)$ when the ice sheet is under clamped-clamped edges condition. (a) $F n<F n_{c}^{(1)}$; (b) $F n_{c}^{(1)}<F n<F n_{c}^{(2)}$; (c) $F n_{c}^{(2)}<F n<F n_{0}$; (d)
$F n_{0}<F n<F n_{c}^{(3)} . F n_{c}^{(1)}=0.8180, F n_{c}^{(2)}=0.9981, F n_{c}^{(3)}=1.3177, F n_{0}=1.1469$.
be confirmed by the sine function in the principal integral of (4.6). As Fn increases, it can be seen that in figures $7(a),(c)$ and $(d)$ that the wave elevation near $x=x_{0}$ is significantly increased when $F n$ is near each of $F n_{c}^{(i)}(i=1,3,5 \ldots)$. When $F n \rightarrow F n_{0}$, the smaller root of $F n^{(1)}(\hat{k})=F n$ tends to zero, and the corresponding wavelength at $x=-\infty$ tends to infinity, which can be observed from figure $7(b)$. Since waves corresponds the roots of $F n^{(2 i)}=F n$ will not appear in the symmetric case, there is no wave propagating to $x=-\infty$ when $F n_{0}<F n<F n_{c}^{(3)}$. As discussed in appendix B, due to the net mass outflow from the source, or mathematically the singularity of function $\hat{\xi}$ at $k=0$, opposite mean wave elevations can be observed at $x \rightarrow+\infty$ and $x \rightarrow-\infty$, which are denoted by $\bar{\xi}_{+\infty}$ and $\bar{\xi}_{-\infty}$ respectively. It is also interesting to see that $\bar{\xi}_{+\infty}>0\left(\bar{\xi}_{-\infty}<0\right)$ when $F n<F n_{0}$, while $\bar{\xi}_{+\infty}<0\left(\bar{\xi}_{-\infty}>0\right)$ when $F n>F n_{0}$. In figure 8, the ice sheet deflection along $y=-b / 2$ generated by a source located at $\left(x_{0}, y_{0}, z_{0}\right)=(0,-b / 2,-H / 5)$ is plotted. Phenomena similar to those in figure 7 may be also observed, apart from that waves corresponding to the roots of $F n^{(2 i)}=F n$ also appear. In fact, from figures $8(c)$ and $(d)$, we can see clearly the wave behind the source due to $F n^{(2)}$ when $F n>F n_{c}^{(2)}$.

### 4.3. Hydrodynamic forces on a submerged horizontal cylinder

A submerged horizontal cylinder with its centre line located at $x_{0}=0$ and $z_{0}=-H / 5$ in the current is considered next. The radius of the cylinder $r_{0}$ is employed as the characteristic length. For the free surface problem, the two-dimensional steady solution may no longer be possible within some ranges of the Froude number at a given submergence (Haussling \& Coleman 1979; Scullen \& Tuck 1995; Semenov \& Wu 2020). This is mainly because the steady wave amplitude reaches its limit and may break, which is reflected by the fact the velocity magnitude at the wave peak tends to zero (Semenov \& Wu 2020). This can occur when the submergence of the circular cylinder is chosen as $\left|z_{0}\right| / r_{0}=2$ for the free surface flow. However, here the problem is quite different. On the one hand, the covering ice sheet will obviously affect the behavior of the waves. More importantly, the case considered here is a three-dimensional one. For clamped edges as an example, the wave elevation along the edge is in fact zero. Indeed, it also has been found from the obtained numerical results that the magnitude of the velocity on the far-field is always positive for the case considered apart from that near the critical Froude numbers. This suggests that the linear model is valid apart from when Froude number is near one of the critical values. At the critical value, the results of the linear model become inaccurate while its prediction for the critical value itself may be accurate. In such a case, nonlinearity and other effects may need to be taken into account.
To conduct numerical computations, the infinite series in (3.45) is truncated at $n^{\prime}=N$ and $m^{\prime}=M$. A convergence test is then carried out for $N$ and $M$ through the hydrodynamic forces on the cylinder when the ice sheet is under clamped-clamped edges condition. The far-field formula (C7) is also employed to check the resistance $F_{R}$ obtained by the near field formula (3.51). The convergence test results are shown in figure 9. An excellent agreement can be seen between the results obtained by $N=15 \& M=15$ and $N=25 \& M=25$, which means the convergence has been achieved. Therefore, $N=15 \& M=15$ is applied in the following calculation. We can also observe that the resistance calculated by (C7) is graphically the same as that from (3.51). It can be seen from figure 9(a) that $F_{R}=0$ when $F n<F n_{c}^{(1)}$, which is similar to the two-dimensional case considered by Li et al. (2019). As mentioned above, there is no wave at infinity when $F n<F n_{c}^{(1)}$, which leads to that coefficients $Q_{n, m}$, $C_{n, m, m^{\prime}}^{ \pm}$and $\mathcal{D}_{n, n^{\prime}, m, m^{\prime}}^{ \pm}$in (3.45) are all real, and correspondingly the solution $f_{n, m}$ should be imaginary. Together with (3.50), $\psi_{n, m}$ is imaginary, then from (3.51), we have $F_{R}=0$.

We then consider the resistance and lift under three different symmetric edge constraints. The results are provided in figure 10. In figure $10(a)$. When the Froude number passes $F n=$


Figure 9: Convergence study for the hydrodynamic forces on the cylinder when the ice sheet is under clamped-clamped edge condition. (a) Resistance; (b) Lift.


Figure 10: Hydrodynamic forces on the cylinder when the ice sheet is under symmetric edges conditions. (a) Resistance; (b) Lift.


Figure 11: Hydrodynamic forces on the cylinder when the ice sheet is under asymmetric edges conditions. (a) Resistance; (b) Lift.
$F n_{c}^{(i)}(i \geqslant 1)$ or $F n_{0}$, the resistance changes rapidly. The number of the wave components at far-field will also change. In particular the resistance $F_{R}$ is zero initially. When $F n$ passes $F n=F n_{c}^{(1)}, F_{R}$ jumps up rapidly. As $F n$ passes $F n_{0}, F_{R}$ will have a sharp drop. These are similar to the behaviours of the resistance at $F n=F n_{c}$ and $F n=1$ in the two-dimensional case considered by Li et al. (2019). When $F n$ continues to increase and pass $F n_{c}^{(2 i-1)}(i \geqslant 2)$, $F_{R}$ will first increase rapidly and then decrease rapidly, and return to almost original trajectory of the curve. In the two-dimensional case, critical values of $F n_{c}^{(i)}(i>1)$ do not exist. For the $F_{L}$ given in figure $10(b)$, it is nonzero even at small $F n$ because the flow is not symmetric about $z=z_{0}$. The lift near $F n_{c}^{(2 i-1)}(i \geqslant 1)$ and $F n_{0}$ have a behaviour similar to that of resistance. At large Fn the lift may become negative similar to the free surface problem (Wu \& Eatock Taylor 1987). It is also interesting to see from figure 10 that the differences in $F_{R}$ or $F_{L}$ under various edge conditions are much more obvious in the region near $F n_{c}^{(i)}$ and $F n_{0}$. When $F n$ passes $F n_{c}^{(2 i-1)}(i>1)$, we can see that the jump phenomena of $F_{L}$ and $F_{R}$ in the case of free-free edges are not as obvious as those of clamped-clamped and simply supported-simply supported edges. The variation of $F_{R}$ and $F_{L}$ against $F n$ under three asymmetric edges constraints are shown in figure 11. Behaviours similar to those in figure 10 may be observed. However, as mentioned in Section 4.2, in addition to $F n_{c}^{(2 i-1)}$, $F n_{c}^{(2 i)}$ will also affect the waves in these asymmetric cases about $y=0$. As a result, more sudden changes can be observed in figure 11.

Computations are then carried out to investigate the effect of the channel width $b$. A comparison with the two-dimensional cases given in Fig. 4 of Li et al. (2019) is also conducted. It should be noted that apart from there are channel width $b$ and edge conditions


Figure 12: Hydrodynamic forces on the cylinder in an ice-covered channel with clamped clamped edges at different channel widths. (a) Resistance; (b) Lift.


Figure 13: Hydrodynamic forces on the cylinder in an ice-covered channel with Free -
Free edges at different channel widths. (a) Resistance; (b) Lift.


Figure 14: Ice sheet deflection $\eta$ along $x$-axis due to a submerged horizontal cylinder
when the ice sheet is under clamped-clamped edges. (a) $F n<F n_{c}^{(1)}$; (b)
$F n_{c}^{(1)}<F n<F n_{0} ;(c) F n_{c}^{(2)}<F n<F n_{c}^{(3)} ;(d) F n_{c}^{(4)}<F n<F n_{c}^{(5)}$. $F n_{c}^{(1)}=0.8180, F n_{c}^{(2)}=0.9981, F n_{c}^{(3)}=1.3177, F n_{c}^{(4)}=1.7682, F n_{c}^{(5)}=2.3230$, $F n_{0}=1.1469$.
here, all the physical parameters are chosen as the same as those in Fig. 4 of Li et al. (2019). The results of clamped - clamped edges are given in figure 12 . When $b=2 H$, it can be seen that the resistance and lift are significantly different from those in the two-dimensional case. As $b$ increases, the results tend to those without the tank wall. When $b=8 H$, very good agreement can be observed between these two cases. It ought to point out at large $b$, for example $b=8 H$, the critical Froude numbers $F n_{c}^{(i)}(i \geqslant 2)$ still have effects. However the effects are obvious only when $F n$ is very close to $F n_{c}^{(i)}$, or the sharp variation of the force is almost like a vertical line, although this line is not included in the figure. In figure 13, the resistance and lift on the cylinder with free - free edges are provided. It can be observed that even at small $b$, the results are already close to the two-dimensional ones. This suggests that when doing model tests in an ice-covered tank, the free - free edges may resemble an unbounded ocean better.

### 4.4. Deflection of the ice sheet induced by a submerged horizontal cylinder

We may also investigate the deflection of the ice sheet $\eta$ due to a submerged horizontal cylinder. The position of the centre line of cylinder is the same as that in Section 4.3. In figure 14 , the hydroelastic waves along $x$-axis when the ice sheet is under clamped-clamped edges are plotted. As can be observed, many features of the waves in figure 14 are similar to those in figure 7, but there are also some differences. In figure 14(a), we can see that the wave profile for $F n<F n_{c}^{(1)}$ is symmetric about $x=x_{0}$, which can be shown through (3.53)~(3.55), while this is opposite to that in figure $7(a)$. As $F n$ increases, the ice deflection $\eta$ at $x=0$ reaches a large negative value when $F n$ is close to $F n_{c}^{(1)}$, then it will rapidly become a positive value


Figure 15: Wave patterns under different edge conditions when $F n_{c}^{(3)}<F n<F n_{c}^{(4)}$.
(a) Clamped-Clamped edges, $F n=1.5\left(F n_{c}^{(3)}=1.3177 \& F n_{c}^{(4)}=1.7682\right)$;
(b) Clamped-Simply supported edges, $F n=1.4\left(F n_{c}^{(3)}=1.2569 \& F n_{c}^{(4)}=1.6824\right)$;
(c) Simply supported-Simply supported edges, $F n=1.3$

$$
\left(F n_{c}^{(3)}=1.2012 \& F n_{c}^{(4)}=1.6005\right) ;
$$

(d) Clamped-Free edges, $F n=1.3\left(F n_{c}^{(3)}=1.0962 \& F n_{c}^{(4)}=1.4454\right)$;
(e) Free-Free edges, $F n=1.05\left(F n_{c}^{(3)}=0.9486 \& F n_{c}^{(4)}=1.1997\right)$;
(f) Simply supported-Free edges, $F n=1.1\left(F n_{c}^{(3)}=1.0561 \& F n_{c}^{(4)}=1.3782\right)$.
after $F n$ passes the critical point. Similar sudden change can be also observed in figures $14(c)$ and (d) when $F n$ is near $F n_{c}^{(3)}$ and $F n_{c}^{(5)}$. However, the deflection above the cylinder in figure $14(c)$ before the critical point is a crest, while it is a trough in figures 14(a) and (d). As discussed previously, the results from the linear theory might become qualitatively inaccurate near one of the critical Froude numbers and the nonlinearity and other effects may need to be taken into account. However, this is only within a very small range. Also the critical Froude numbers predicted from the linear theory are still valid.
In order to show the influence of edge conditions on the wave profiles, typical wave patterns under six different edge constraints when $F n_{c}^{(3)}<F n<F n_{c}^{(4)}$ are presented in figure 15. For
waves under symmetric edges given in figures $15(a),(c)$ and $(e)$, there is only one component in the longitudinal wave at the downstream region, while the waves at the upstream region contain two components. It can be found that the wave patterns under clamped-clamped and simply supported-simply supported edges are relatively similar, but the wave pattern under free-free is quite different. In figure $15(e)$, we can see that the downstream wave under freefree edges is not as obvious as that under other two symmetric edges. Similar phenomenon can be also observed from the transverse waves in figures 15(a), (c) and (e). From these result, we may infer that an ice-coved channel with free-free edges is closer to an unbounded region when compared to those with other edge restraints. For waves under asymmetric edge restraints shown in figures $15(b),(d)$ and $(f)$. The longitudinal waves at the downstream region consist of two components, while the waves at the upstream region containing three components. Compared with the wave profiles under clamped-simply supported edges shown in figure $15(b)$, the asymmetry of the transverse waves is more obvious in cases with a free edge given in figures $15(d)$ and $(f)$.

## 5. Conclusions

The interaction of a uniform current with a submerged horizontal circular cylinder in an ice-covered channel is studied analytically, the solution procedure is applicable to various edge conditions. The three-dimensional Green function satisfying all boundary conditions apart from that on the body surface is first derived. By using this, the potential due to sources distributed along the centre line of the cylinder is obtained, from which the potentials due to multipoles are constructed directly by differentiating the position of the centre line. The velocity potential for a submerged circular cylinder is written in terms of a summation of these multipoles.

From the solution of the Green function, it is confirmed that there is an infinite number of critical Froude numbers $F n_{c}^{(i)}(i=1,2,3 \ldots)$ and another one $F n_{0}$ which corresponds to the case of $F n_{0}=1$ in the two-dimensional problem where there are only two critical Froude numbers. This is consistent with what has been noticed in the previous work (Khabakhpasheva et al. 2019). There will be no wave propagating to infinity when $F n<F n_{c}^{(1)}$. When $F n_{c}^{(i)}<$ $F n<F n_{c}^{(i+1)}$ and $F n<F n_{0}$, there will be $2 i$ waves, and $i$ waves are at downstream region with group velocity smaller than the current speed, while the other $i$ waves with larger group velocity are at upstream region. When $F n_{c}^{(i)}<F n<F n_{c}^{(i+1)}$ and $F n>F n_{0}$, one downstream wave will disappear, and there will be $i-1$ waves at downstream. For infinite water depth, the effect of $F n_{0}$ does not exist. When $\widetilde{F n_{c}}{ }_{c}^{(i)}<F n<\widetilde{F n}{ }_{c}^{(i+1)}$, there are always $2 i$ waves, with $i$ waves at $x=-\infty$ and $x=+\infty$ respectively.

In the cases of symmetric edge constraints, the symmetric and anti-symmetric transverse modes are completely independent. The former corresponds to the roots of $F n=F n^{(2 i-1)}$ $(i \geqslant 1)$, while the latter corresponds to the solutions of $F n=F n^{(2 i)}$. If the flow itself is symmetric about $y=0$, the waves due to the roots of $F n=F n^{(2 i)}$ will not exist. In general, the symmetric and anti-symmetric modes always coexist, and they are fully coupled. When $F n_{c}^{(i-1)}<F n<F n_{c}^{(i)}(i \geqslant 2)$, the contribution of the $i$-th transverse wave mode to the overall wave profile will be significant.

The ice sheet deflection due to a submerged horizontal circular cylinder shows that the hydroelastic wave is symmetric about $x=x_{0}$ and only exists in the region near the cylinder when $F n<F n_{c}^{(1)}$. When $F n$ is near $F n_{c}^{(1)}$, there is a sudden change in the wave above the cylinder, which is similar to that of the two-dimensional case (Li et al. 2019). As Fn increases, this phenomenon can be also observed near other critical points which do not
exist in the two-dimensional case. The three-dimensional wave pattern also indicates that it can be greatly affected by the edge conditions. The resistance on the cylinder is zero when $F n<F n_{c}^{(1)}$, while the lift is nonzero. When $F n$ increases and passes each critical Froude number, the forces change very rapidly. Under different edge conditions, the change is very different.

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## Declaration of interests

The authors report no conflict of interest.

## Appendix A. Elements of the matrix $[\mathcal{F}]$ and columns $[\mathcal{B}] \&[C]$ under clamped-clamped edges condition

Applying the clamped-clamped edge condition shown in (2.11) to $\xi$, we have

$$
\left.\begin{array}{r}
c_{0}-b c_{1}+b^{2} c_{2}-b^{3} c_{3}+\frac{a_{0}}{24} b^{4}+\sum_{m=1}^{+\infty} \frac{a_{m}}{\sigma_{m}^{4}}=0 \\
c_{1}-2 b c_{2}+3 b^{2} c_{3}-\frac{a_{0}}{6} b^{3}=0  \tag{A1}\\
c_{0}+b c_{1}+b^{2} c_{2}+b^{3} c_{3}+\frac{a_{0}}{24} b^{4}+\sum_{m=1}^{+\infty}(-1)^{m} \frac{a_{m}}{\sigma_{m}^{4}}=0 \\
c_{1}+2 b c_{2}+3 b^{2} c_{3}+\frac{a_{0}}{6} b^{3}=0
\end{array}\right\} .
$$

Substituting (3.15) and (3.18a) into (A 1), the coefficients of matrix [ $\mathcal{A}]$ and column $[\mathcal{B}]$ can then be given as

$$
\left.\begin{array}{l}
\mathcal{A}_{0 j}=\delta_{0 j}-\delta_{1 j} b+\delta_{2 j} b^{2}-\delta_{3 j} b^{3}+\frac{\alpha_{0, j}}{24} b^{4}+\sum_{m=1}^{+\infty} \frac{\alpha_{m, j}}{\sigma_{m}^{4}}  \tag{A2}\\
\mathcal{A}_{1 j}=\delta_{1 j}-\delta_{2 j} 2 b+\delta_{3 j} 3 b^{2}-\frac{\alpha_{0, j}}{6} b^{3} \\
\mathcal{A}_{2 j}=\delta_{0 j}+\delta_{1 j} b+\delta_{2 j} b^{2}+\delta_{3 j} b^{3}+\frac{\alpha_{0, j}}{24} b^{4}+\sum_{m=1}^{+\infty}(-1)^{m} \frac{\alpha_{m, j}}{\sigma_{m}^{4}} \\
\mathcal{A}_{3 j}=\delta_{1 j}+\delta_{2 j} 2 b+\delta_{3 j} 3 b^{2}+\frac{\alpha_{0, j}}{6} b^{3}
\end{array}\right\},
$$

$$
\begin{equation*}
\mathcal{B}_{j}=-\mathrm{i} \sum_{m=0}^{+\infty} \frac{\rho U k \cosh K_{m}\left(z_{0}+H\right) \cos \sigma_{m}\left(y_{0}+b\right)}{\left(1+\delta_{m 0}\right) b \Delta_{m} \cosh K_{m} H} v_{m, j}, \quad j=0 \sim 3, \tag{A3}
\end{equation*}
$$

where

$$
v_{m, j}=\left\{\begin{array}{l}
\delta_{0 j}\left[\frac{1-\delta_{m 0}}{\sigma_{m}^{4}}+\delta_{m 0}\left(\frac{b^{4}}{24}+\sum_{l=1}^{+\infty} \frac{\gamma_{l}}{\sigma_{l}^{4}}\right)\right]-\delta_{1 j} \delta_{m 0} \frac{b^{3}}{6}  \tag{A4}\\
+\delta_{2 j}\left[\frac{\left(1-\delta_{m 0}\right)(-1)^{m}}{\sigma_{m}^{4}}+\delta_{m 0}\left(\frac{b^{4}}{24}+\sum_{l=1}^{+\infty} \frac{(-1)^{l} \gamma_{l}}{\sigma_{l}^{4}}\right)\right]+\delta_{3 j} \delta_{m 0} \frac{b^{3}}{6}
\end{array}\right\} .
$$

Based on (A 2) and (A 3), the solution of $c_{i}$ of (3.21) can be obtained as

$$
\begin{equation*}
c_{i}=-\mathrm{i} \sum_{m=0}^{+\infty} \frac{\rho U k \cosh K_{m}\left(z_{0}+H\right) \cos \sigma_{m}\left(y_{0}+b\right)}{\left(1+\delta_{m 0}\right) b|\mathcal{A}| \Delta_{m} \cosh K_{m} H} c_{m, i}^{\prime}, \quad i=0 \sim 3, \tag{A5}
\end{equation*}
$$

where $c_{m, i}^{\prime}=|\mathcal{A}| \times c_{m, i}, c_{m, i}$ is the solution of $[\mathcal{A}]\left[\boldsymbol{C}_{\boldsymbol{m}}\right]=\left[v_{\boldsymbol{m}}\right],\left[v_{\boldsymbol{m}}\right]$ is a known column containing coefficients $v_{m, i}(i=0 \sim 3),\left[\boldsymbol{C}_{\boldsymbol{m}}\right]$ is an unknown column containing $c_{m, i}$.

Substituting (3.20) into (A 1), the matrix equation (3.21) can be further split into two $2 \times 2$ sub ones, which can be written as

$$
\left[\mathcal{A}^{\boldsymbol{S}}\right]\left[\begin{array}{l}
c_{0}  \tag{A6}\\
c_{2}
\end{array}\right]=\left[\begin{array}{lll}
\left.\mathcal{B}^{\boldsymbol{S}}\right] & \text { and } & {\left[\mathcal{A}^{\boldsymbol{A}}\right]}
\end{array}\right]\left[\begin{array}{l}
c_{1} \\
c_{3}
\end{array}\right]=\left[\mathcal{B}^{\boldsymbol{A}}\right]
$$

or symmetric and anti-symmetric modes are fully uncoupled. The elements of matrixes $\left[\mathcal{A}^{\boldsymbol{S}}\right],\left[\mathcal{A}^{\boldsymbol{A}}\right]$ and column $\left[\mathcal{B}^{\boldsymbol{S}}\right],\left[\mathcal{B}^{\boldsymbol{A}}\right]$ can be expressed as

$$
\left.\left.\begin{array}{l}
\mathcal{A}_{0 j}^{S}=\delta_{0 j}+\delta_{1 j} b^{2}+\frac{\alpha_{0,2 j}}{24} b^{4}+\sum_{m=1}^{+\infty} \frac{\alpha_{2 m, 2 j}}{\sigma_{2 m}^{4}}  \tag{A7}\\
\mathcal{A}_{1 j}^{S}=2 \delta_{1 j} b+\frac{\alpha_{0,2 j}}{6} b^{3}
\end{array}\right\} \text { and } \begin{array}{l}
\mathcal{A}_{0 j}^{A}=\delta_{0 j} b+\delta_{1 j} b^{3}+\sum_{m=1}^{+\infty} \frac{\alpha_{2 m-1,2 j+1}}{\sigma_{2 m-1}^{4}} \\
\mathcal{A}_{1 j}^{A}=\delta_{0 j}+3 \delta_{1 j} b^{2}
\end{array}\right\},
$$

and

$$
\left.\begin{array}{l}
\mathcal{B}_{j}^{S}=-\mathrm{i} \sum_{m=0}^{+\infty} \frac{\rho U k(-1)^{m} \cosh K_{2 m}\left(z_{0}+H\right) \cos \sigma_{2 m} y_{0}}{\left(1+\delta_{m 0}\right) b \Delta_{2 m} \cosh K_{2 m} H} v_{m, j}^{S}  \tag{A8}\\
\mathcal{B}_{j}^{A}=\mathrm{i} \sum_{m=0}^{+\infty} \frac{\rho U k(-1)^{m} \cosh K_{2 m+1}\left(z_{0}+H\right) \sin \sigma_{2 m+1} y_{0}}{b \Delta_{2 m+1} \cosh K_{2 m+1} H} v_{m, j}^{A}
\end{array}\right\}, \quad j=0,1
$$

with

$$
\begin{equation*}
v_{m, j}^{S}=\delta_{0 j}\left[\frac{1-\delta_{m 0}}{\sigma_{2 m}^{4}}+\delta_{m 0}\left(\frac{b^{4}}{24}+\sum_{l=1}^{+\infty} \frac{\gamma_{2 l}}{\sigma_{2 l}^{4}}\right)\right]+\delta_{1 j} \delta_{m 0} \frac{b^{3}}{6} \quad \text { and } \quad v_{m, j}^{A}=\frac{\delta_{0 j}}{\sigma_{2 m+1}^{4}} . \tag{A9}
\end{equation*}
$$

$|\mathcal{A}|$ can be further obtained as

$$
\begin{equation*}
|\mathcal{A}|=4\left|\mathcal{A}^{\boldsymbol{S}}\right|\left|\mathcal{A}^{\boldsymbol{A}}\right| . \tag{A10}
\end{equation*}
$$

## Appendix B. Symmetry property of the Green function

Let $G_{0}=G\left(x, y, z, x_{0}, y_{0}, z_{0}\right)$ and $G_{1}=G\left(x, y, z, x_{1}, y_{1}, z_{1}\right)$, and correspondingly, $\xi_{0}=$ $\xi\left(x, y, x_{0}, y_{0}, z_{0}\right)$ and $\xi_{1}=\xi\left(x, y, x_{1}, y_{1}, z_{1}\right)$. At $x \rightarrow+\infty$, the Green function and the wave elevation can be expressed as

$$
\begin{equation*}
G_{0}=\operatorname{Re}\left\{\sum_{j=0}^{S} \frac{G_{0}^{(j)}(y, z)}{k_{j}} e^{-\mathrm{i} k_{j}\left(x-x_{0}\right)}\right\} \quad \text { and } \quad G_{1}=\operatorname{Re}\left\{\sum_{l=0}^{S} \frac{G_{1}^{(l)}(y, z)}{k_{l}} e^{-\mathrm{i} k_{l}\left(x-x_{1}\right)}\right\}, \tag{B1}
\end{equation*}
$$

Employing (A.3) in Ren et al. (2018b), $I_{I}$ can be written as

$$
\begin{align*}
I_{I}= & -\left.U \int_{-b}^{b}\left(G_{0} \xi_{1}-G_{1} \xi_{0}\right)\right|_{x \rightarrow-\infty} ^{x \rightarrow+\infty} d y  \tag{B7}\\
& +\frac{L}{\rho} \oint_{\mathcal{T}}\left(\xi_{1} \frac{\partial}{\partial n} \nabla^{2} \xi_{0}-\frac{\partial \xi_{1}}{\partial n} \nabla^{2} \xi_{0}-\xi_{0} \frac{\partial}{\partial n} \nabla^{2} \xi_{1}+\frac{\partial \xi_{0}}{\partial n} \nabla^{2} \xi_{1}\right) d l,
\end{align*}
$$

where $k_{0} \rightarrow 0$ and $k_{j}(j=1 \sim S)$ denotes the wave number of the component $j$, and the summation contains only those wave components with group velocity larger than $U$. The terms of $j=0$ in (B1) are related to the 'blockage parameter', as discussed for example in Newman (1969) and Mei \& Chen (1976) in the two dimensional case. However, for a submerged body, there should be no net flow being created. Therefore, the blockage parameter is a constant. Its derivative will be zero and no flow will be created by the constant. (B1) is due to a source which does generate flow into the fluid domain. This will continue at infinity, which is reflected by the terms of $j=0$ of (B 1).

Although $G$ and $\xi$ involve only the real part, we may use the whole complex function here. Substituting (B 1) and (B 2) into (2.6) and (2.7), the boundary conditions of $G_{0}^{(j)}$ and $\xi_{0}^{(j)}$ on ice sheet can be established as

$$
\begin{equation*}
\mathrm{i}\left[\left(L k_{j}^{4}+\rho g\right) \xi_{0}^{(j)}-2 L k_{j}^{2} \frac{d^{2} \dot{\xi}_{0}^{(j)}}{d y^{2}}+L \frac{d^{4} \xi_{0}^{(j)}}{d y^{4}}\right]=\rho U G_{0}^{(j)}, \quad j=0 \sim S, \quad z=0 \tag{B3}
\end{equation*}
$$

Similar procedure can be adopted at $x \rightarrow-\infty$. Applying the Green's second identity, we have

$$
\begin{equation*}
G\left(x_{1}, y_{1}, z_{1}, x_{0}, y_{0}, z_{0}\right)-G\left(x_{0}, y_{0}, z_{0}, x_{1}, y_{1}, z_{1}\right)=\oiint_{\mathcal{S}}\left(G_{0} \frac{\partial G_{1}}{\partial n}-G_{1} \frac{\partial G_{0}}{\partial n}\right) d S \tag{B5}
\end{equation*}
$$

where $\mathcal{S}$ is comprised of the seabed $S_{H}$, two vertical channel walls $S_{w}$, ice sheet $S_{I}$ and two vertical surfaces $S_{ \pm \infty}$ at $x \rightarrow \pm \infty$, respectively. As $\partial G_{0} / \partial n$ and $\partial G_{1} / \partial n$ are zero on $S_{H}$ and $S_{w}$, only the integrals over $S_{I}$ and $S_{ \pm \infty}$ need to be kept.
For the integral at $S_{I}$, using (2.6), (2.7) and integration by parts, this gives

$$
\begin{align*}
I_{I} & =\iint_{S_{I}}\left(G_{0} \frac{\partial G_{1}}{\partial z}-G_{1} \frac{\partial G_{0}}{\partial z}\right) d S \\
& =-\left.U \int_{-b}^{b}\left(G_{0} \xi_{1}-G_{1} \xi_{0}\right)\right|_{x \rightarrow-\infty} ^{x \rightarrow+\infty} d y+\frac{L}{\rho} \iint_{S_{I}}\left(\xi_{1} \nabla^{4} \xi_{0}-\xi_{0} \nabla^{4} \xi_{1}\right) d S . \tag{B6}
\end{align*}
$$

where $\mathcal{T}$ is comprised of the lines $x= \pm \infty$ and $y= \pm b$. Using (B 1)~(B 3 ) and integrating

$$
\xi_{0}=\operatorname{Re}\left\{\sum_{j=0}^{S} \xi_{0}^{(j)}(y) e^{-\mathrm{i} k_{j}\left(x-x_{0}\right)}\right\} \quad \text { and } \quad \xi_{1}=\operatorname{Re}\left\{\sum_{l=0}^{S} \xi_{1}^{(l)}(y) e^{-\mathrm{i} k_{l}\left(x-x_{1}\right)}\right\}
$$

by parts again, $I_{I}$ can be further written as

$$
I_{I}=\left\{\begin{array}{l}
\left.\left.\frac{\mathrm{i} L}{\rho} \sum_{j=0}^{S} \sum_{l=0}^{S} e^{-\mathrm{i}\left[k_{j}\left(x-x_{0}\right)+k_{l}\left(x-x_{1}\right)\right]}\left[\begin{array}{l}
\left(\xi_{1 y}^{(l)} \xi_{0 y y}^{(j)} / k_{j}-\xi_{0 y}^{(j)} \xi_{1 y y}^{(l)} / k_{l}\right) \\
-\left(\xi_{1}^{(l)} \xi_{0 y y y}^{(j)} / k_{j}-\xi_{0}^{(j)} \xi_{1 y y y}^{(l)} / k_{l}\right)
\end{array}\right]\right|_{y=-b} ^{y=b}\right|_{x \rightarrow-\infty} ^{y \rightarrow+\infty}  \tag{B8}\\
+\left.\frac{L}{\rho} \int_{-\infty}^{+\infty}\left[\begin{array}{c}
\left(\xi_{1} \xi_{0 y y y}-\xi_{0} \xi_{1 y y y}\right)-\left(\xi_{1 y} \xi_{0 y y}-\xi_{0 y} \xi_{1 y y}\right) \\
-2\left(\xi_{1 x} \xi_{0 x y}-\xi_{0 x} \xi_{1 x y}\right)
\end{array}\right]\right|_{y=-b} ^{y=b} d x
\end{array}\right\}
$$

Invoking the edge conditions in (2.11), it can be shown $I_{I}=0$ for any of these edge constraints.
For the integral over $S_{ \pm \infty}$, we can consider $S_{+\infty}$ as an example, as the integral over $S_{-\infty}$ can be done in a similar way. Substituting (B1) into the integral at $S_{+\infty}$, we have

$$
I_{+\infty}=\iint_{S_{+\infty}}\left(G_{0} \frac{\partial G_{1}}{\partial x}-G_{1} \frac{\partial G_{0}}{\partial x}\right) d S
$$

$$
\begin{equation*}
=\sum_{j=0}^{S} \sum_{l=0}^{S} e^{-\mathrm{i}\left[k_{j}\left(x-x_{0}\right)+k_{l}\left(x-x_{1}\right)\right]} \iint_{S_{+\infty}}\left(\frac{1}{k_{l}}-\frac{1}{k_{j}}\right) G_{0}^{(j)} G_{1}^{(l)} d S . \tag{B9}
\end{equation*}
$$

Since $G_{0}$ and $G_{1}$ is governed by the Laplace equation at $x \rightarrow+\infty$, we have

$$
\left.\begin{array}{l}
\nabla_{y z}^{2} G_{0}^{(j)}=k_{j}^{2} G_{0}^{(j)}  \tag{B10}\\
\nabla_{y z}^{2} G_{1}^{(l)}=k_{j}^{2} G_{1}^{(l)}
\end{array}\right\}
$$

where $\nabla_{y z}^{2}=\partial^{2} / \partial y^{2}+\partial^{2} / \partial z^{2}$ denotes the Laplace operator at $O-y z$ plane. Substituting (B 10) into (B 9) and applying Green's second identity, we obtain

$$
\begin{align*}
I_{+\infty} & =\mathrm{i} \sum_{j=0}^{S} \sum_{l=0}^{S} \frac{e^{-\mathrm{i}\left[k_{j}\left(x-x_{0}\right)+k_{l}\left(x-x_{1}\right)\right]}}{k_{j} k_{l}\left(k_{j}+k_{l}\right)} \iint_{S_{+\infty}}\left(G_{1}^{(l)} \nabla_{y z}^{2} G_{0}^{(j)}-G_{0}^{(j)} \nabla_{y z}^{2} G_{1}^{(l)}\right) d S  \tag{B11}\\
& =\mathrm{i} \sum_{j=0}^{S} \sum_{l=0}^{S} \frac{e^{-\mathrm{i}\left[k_{j}\left(x-x_{0}\right)+k_{l}\left(x-x_{1}\right)\right]}}{k_{j} k_{l}\left(k_{j}+k_{l}\right)} \oint_{\mathcal{L}_{+\infty}}\left(G_{1}^{(l)} \frac{\partial G_{0}^{(j)}}{\partial n}-G_{0}^{(j)} \frac{\partial G_{1}^{(l)}}{\partial n}\right) d S,
\end{align*}
$$

where $\mathcal{L}_{+\infty}$ is comprised of lines $y= \pm b, z=0$ and $z=-H$. Applying (2.9), (2.10), (B 3) and (B4), we have

$$
I_{+\infty}=\frac{\mathrm{i} L}{\rho} \sum_{j=0}^{S} \sum_{l=0}^{S} \frac{e^{-\mathrm{i}\left[k_{j}\left(x-x_{0}\right)+k_{l}\left(x-x_{1}\right)\right]}}{k_{l}+k_{j}} \int_{-b}^{+b}\left[\begin{array}{c}
2 k_{j} k_{l}\left(\xi_{0}^{(j)} \xi_{1 y y}^{(l)}-\xi_{1}^{(l)} \xi_{0 y y}^{(j)}\right)  \tag{B12}\\
-\left(k_{j} \xi_{0}^{(j)} \xi_{1 y y y y}^{(l)} / k_{l}-k_{l} \xi_{1}^{(l)} \xi_{0 y y y y}^{(j)} / k_{j}\right)
\end{array}\right] d y
$$

Using integration by parts and edge conditions in (2.11), we have $I_{+\infty}=0$, similarly $I_{-\infty}=0$.
This means that the integral at right hand-side of (B5) is equal to 0 , which gives

$$
\begin{equation*}
G\left(x_{1}, y_{1}, z_{1}, x_{0}, y_{0}, z_{0}\right)=G\left(x_{0}, y_{0}, z_{0}, x_{1}, y_{1}, z_{1}\right) \tag{B13}
\end{equation*}
$$

Appendix C. Far-field formula of the resistance
The formula of the resistance can be written as (Wu 1995)

$$
\begin{equation*}
F_{R}=-\frac{1}{2} \rho \iint_{S_{B}}\left(\phi_{x} \frac{\partial \phi}{\partial n}-\phi \frac{\partial \phi_{x}}{\partial n}\right) d S, \tag{C1}
\end{equation*}
$$

$$
\begin{equation*}
F_{R}=\frac{1}{2} \rho \iint_{S_{I}+S_{ \pm \infty}}\left(\phi_{x} \frac{\partial \phi}{\partial n}-\phi \frac{\partial \phi_{x}}{\partial n}\right) d S . \tag{C2}
\end{equation*}
$$

818 It is noted that the integral in (C2) can be treated in a similar way as shown in appendix B. 819 This gives
where

$$
\begin{align*}
\phi= & \operatorname{Re}\left\{\sum_{s=1}^{S} \sum_{n=0}^{+\infty} \chi_{s} \frac{\left[\mathscr{B}_{s, n}^{(1)} e^{\mathrm{i} k_{s}\left(x-x_{0}\right)}+\mathscr{B}_{s, n}^{(2)} e^{-\mathrm{i} k_{s}\left(x-x_{0}\right)}\right] k_{s} \cosh K_{n, s}(z+H) \cos \sigma_{n}(y+b)}{K_{n, s} \sinh K_{n, s} H}\right\} \\
& +\operatorname{sgn}(x) \phi^{(0)}, \\
& \eta=-\frac{1}{U} \operatorname{Im}\left\{\sum_{s=1}^{S} \sum_{n=0}^{+\infty} \chi_{s}\left[\mathscr{B}_{s, n}^{(1)} e^{\mathrm{i} k_{s}\left(x-x_{0}\right)}-\mathscr{B}_{s, n}^{(2)} e^{-\mathrm{i} k_{s}\left(x-x_{0}\right)}\right] \cos \sigma_{n}(y+b)\right\},
\end{align*}
$$

$$
\left.\begin{array}{l}
\mathscr{B}_{s, n}^{(1)}=-\mathrm{i} \rho U \sum_{n^{\prime}=0}^{+\infty} \sum_{m=1}^{+\infty} \frac{r_{0}^{m} f_{n^{\prime}, m} \tau_{n, n^{\prime}}\left(k_{s}\right) E_{n^{\prime}, m}\left(k_{s}, z_{0}\right)}{2^{m}(m-1)!\Delta_{n^{\prime}}\left(k_{s}\right)|\mathcal{A}|^{\prime}\left(k_{s}\right) \cosh K_{n^{\prime}, s} H}  \tag{C6}\\
\mathscr{B}_{s, n}^{(2)}=\mathrm{i} \rho U \sum_{n^{\prime}=0}^{+\infty} \sum_{m=1}^{+\infty} \frac{r_{0}^{m} f_{n^{\prime}, m} \tau_{n, n^{\prime}}\left(k_{s}\right) E_{n^{\prime}, m}\left(-k_{s}, z_{0}\right)}{2^{m}(m-1)!\Delta_{n^{\prime}}\left(k_{s}\right)|\mathcal{A}|^{\prime}\left(k_{s}\right) \cosh K_{n^{\prime}, s} H}
\end{array}\right\},
$$

Based on (3.40) and (3.53), the asymptotic expression of $\phi$ and $\eta$ at $x \rightarrow+\infty$ can be written as:

$$
834 \quad F_{R}=-\frac{b}{4 U^{2}} \sum_{s=1}^{S} \sum_{n=0}^{+\infty}\left(1+\delta_{n 0}\right) \chi_{s}\left|\mathscr{B}_{s, n}^{(1)}+\overline{\mathscr{B}}_{s, n}^{(2)}\right|^{2} \times\left[\begin{array}{l}
\rho U^{2} k_{s}^{2}\left(H+\sinh 2 K_{n, s} H / 2 K_{n, s}\right) \\
-\rho U^{2} K_{n, s} \sinh 2 K_{n, s} H  \tag{C7}\\
+4 L K_{n, s}^{4} \sinh ^{2} K_{n, s} H
\end{array}\right] \text {, }
$$

$$
F_{R}=\left[\begin{array}{l}
\left.\frac{1}{2} \rho \int_{-H}^{0} d z \int_{-b}^{b}\left(\frac{\partial \phi}{\partial x} \frac{\partial \phi}{\partial x}-\phi \frac{\partial^{2} \phi}{\partial x^{2}}\right)\right|_{x \rightarrow-\infty} ^{x \rightarrow+\infty} d y  \tag{C3}\\
-\left.\left.\frac{1}{2} \rho U \int_{-b}^{b}\left(\eta \frac{\partial \phi}{\partial x}-\phi \frac{\partial \eta}{\partial x}\right)\right|_{x \rightarrow-\infty} ^{x \rightarrow+\infty}\right|_{z=0} d y \\
+\left.\frac{L}{2} \int_{-b}^{b}\left[\left(\eta \frac{\partial^{4} \eta}{\partial x^{4}}-\frac{\partial \eta}{\partial x} \frac{\partial^{3} \eta}{\partial x^{3}}\right)-\left(\frac{\partial \eta}{\partial x} \frac{\partial^{3} \eta}{\partial x^{3}}-\frac{\partial^{2} \eta}{\partial x^{2}} \frac{\partial^{2} \eta}{\partial x^{2}}\right)\right]\right|_{x \rightarrow-\infty} ^{x \rightarrow+\infty} d y \\
+\left.\frac{L}{U} \int_{-b}^{b}\left(\eta \frac{\partial^{4} \phi}{\partial x^{3} \partial z}+\eta \frac{\partial \phi}{\partial x \partial z^{3}}-\frac{\partial \eta}{\partial x} \frac{\partial^{3} \phi}{\partial x^{2} \partial z}-\frac{\partial \eta}{\partial x} \frac{\partial \phi}{\partial z^{3}}\right)\right|_{x \rightarrow-\infty} ^{x \rightarrow+\infty} d y
\end{array}\right] .
$$

It should be noted that the summation of $s$ in (C7) contains all the wave components.

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