

PROCEEDINGS A

rspa.royalsocietypublishing.org

Research



Check for
updates

Article submitted to journal

Subject Areas:

Computer modelling and simulation,
Geophysics, Statistics

Keywords:

hazard assessment, sediment
amplification, unstructured mesh,
Karachi port, Makran subduction
zone, coastal engineering

Author for correspondence:

Devaraj Gopinathan
e-mail: d.gopinathan@ucl.ac.uk

Probabilistic Quantification of Tsunami Current Hazard using Statistical Emulation

Devaraj Gopinathan¹, Mohammad

Heidarzadeh² and Serge Guillas¹

¹Department of Statistical Science, University College London, Gower Street, London WC1E 6BT, UK.

²Department of Civil & Environmental Engineering, Brunel University London, Uxbridge UB8 3PH, UK.

In this paper, statistical emulation is shown to be an essential tool for the end-to-end physical and numerical modelling of local tsunami impact, *i.e.* from the earthquake source to tsunami velocities and heights. In order to surmount the prohibitive computational cost of running a large number of simulations, the emulator, constructed using 300 training simulations from a validated tsunami code, yields 1 million predictions. This constitutes a record for any realistic tsunami code to date, and is a leap in tsunami science since high risk but low probability hazard thresholds can be quantified. For illustrating the efficacy of emulation, we map probabilistic representations of maximum tsunami velocities and heights at around 200 locations about Karachi port. The 1 million predictions comprehensively sweep through a range of possible future tsunamis originating from the Makran Subduction Zone (MSZ). We rigorously model each step in the tsunami life-cycle: first use of the 3-D subduction geometry Slab2 in MSZ, most refined fault segmentation in MSZ, first sediment enhancements of seabed deformation (up to 60% locally), and bespoke unstructured meshing algorithm. Owing to the synthesis of emulation and meticulous numerical modelling, we also discover substantial local variations of currents and heights.

1. Introduction

Following the unexpected damage incurred at ports from the tsunamis of 2004 (Indian Ocean), 2010 (Chile) and 2011 (Japan) [1,2], it is paramount to investigate the associated hazard. Despite recent studies [1,3–5] and advances in high-fidelity modelling [6], probabilistic

© The Authors. Published by the Royal Society under the terms of the Creative Commons Attribution License <http://creativecommons.org/licenses/by/4.0/>, which permits unrestricted use, provided the original author and source are credited.

methods for quantification of future tsunami hazard due to strong flows in harbors are sparse [7,8]. The need for such a quantification is further accentuated by certain peculiarities related with the phenomena of large tsunami currents in ports, *e.g.* the drifting of the 285 m ship *Maersk Mandraki* on 26 December 2004 at the Omani port of Salalah [2], despite small wave heights. It is deceptive to associate high wave amplitudes with high velocities. The treacherous nature of the currents was aggravated by the fact that strong currents in harbors continued for hours after the waves with maximum amplitude had arrived. This is all the more consequential since conventional tsunami warnings may be lifted after visibly perceptible signs of the tsunami (*i.e.* vertical displacement) have disappeared, whereas the strong currents may manifest later on.

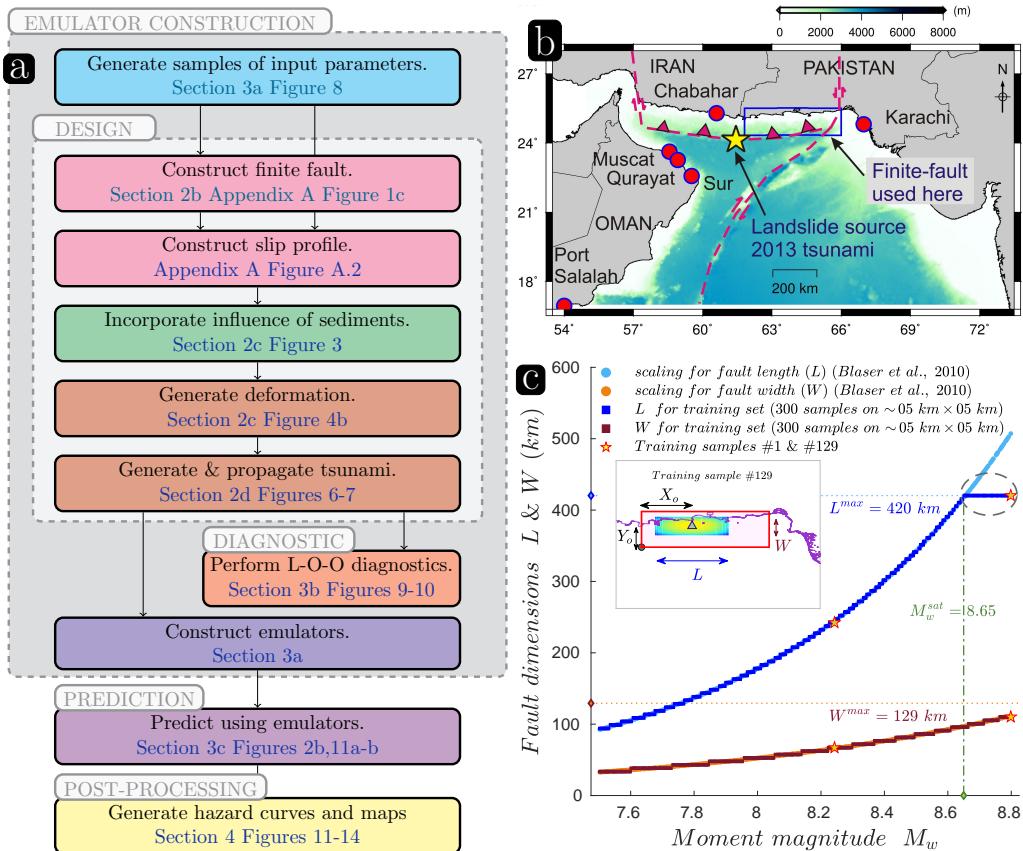


Figure 1. a) Global workflow describing the integration of different work components in this study (see Electronic Supplementary Material (ESM) for a more detailed workflow). b) The Makran subduction zone (MSZ). c) Fault dimensions, *i.e.* length (L) and width (W) of 300 earthquake scenarios plotted over the scaling relation with respect to the moment magnitude (M_w). It shows the maximum length (L_{max}), width (W_{max}) and moment magnitude (M_w^{sat}) accommodable in the eastern MSZ. L saturates (ellipse) after M_w 8.65 (green line). The inset shows the fault dimensions (L , W) and epicenter co-ordinates (X_o , Y_o) for scenario no. 129.

Probabilistic scenario-based tsunami hazard assessment (PTHA) delivers *a priori* critical data to buttress tsunami disaster planning and practice. Scenario-based assessment scores over its catalogue-based counterpart through a more comprehensive exploration of plausible scenarios. Probabilistic scenario-based assessment surpasses deterministic scenario-based assessment in its assignment of probabilities and weighed integration of the different plausible scenarios. There exist variants in the probabilistic methodologies employed in PTHA – Monte Carlo [9], logic-tree [10], and Bayesian [11]. For an in-depth discussion on PTHA, the reader is referred to the recent review of Grezio *et al* [12]. However, apart from the difficulties in assigning probabilities

to scenarios, the computational burden expended for simulating each scenario prohibits an exhaustive sweep over the entire range of plausible scenarios. In this work, we pursue another probabilistic route via statistical emulation to quantify uncertainties in future tsunamis due to the uncertain earthquake sources (see workflow in Figure 1a). Since the probability of large magnitude events is small, a comprehensive coverage of the Gutenberg-Richter relation requires a large number of runs for the diversity of plausible events to be well represented across magnitudes and source locations (at least thousands for a coarse quantification and orders more for realistic assessments). Due to the considerable computational complexity of each simulation of coastal tsunami currents, such a probabilistic endeavor is made feasible by essentially replacing the numerical tsunami model by a statistical surrogate – the emulator. To our knowledge, this is the first time that Gaussian process (GP) emulation has been marshalled to generate future earthquake-generated tsunami currents; it has been employed only once in the past for currents, for a single source of landslide-generated tsunamis with considerable benefits in terms of computational costs and hazard assessment [13]. Here, with a design of only 300 full-fledged training runs, we fit an emulator to rapidly predict the impact of 1 million plausible tsunamis at prescribed locations. These emulated runs enable us to characterize uncertainty in future tsunami currents. A recent work by Kotani *et al* [14] adopts a similar strategy of approximating the input-output response surface, albeit using non-linear regression. Zhang and Niu [15] showcase a comparable 1.38 million scenarios, although using linear combination of waves from unit sources. Another recent strategy for reduction of the number of tsunami simulations employs an event-tree coupled with cluster filtering of sources [16,17].

Additionally, formidable computational challenges must be addressed in order to accurately represent both the actual geophysical processes and their uncertainties. Despite possible issues arising from handling fine resolutions, our main challenge lies in encapsulating a large number of these high-definition simulations within a statistical framework. This is an essential requirement for PTHA and stretches the limit of current High-performance Computing (HPC) facilities, even with the latest GPU (Graphics Processing Unit) acceleration [18]. Often, the trade-off between capability and capacity in HPC is left unresolved by either radically simplifying the physics (*e.g.* a linear tsunami propagation till say 100 m depth with the use of an empirical relationship thereafter), or running very few fine resolution simulations as scenarios. Given a validated tsunami model, we argue that our emulation framework, in this context of currents that are nonlinear and very sensitive to near shore bathymetry, attempts a solution to this trade-off between precision and coverage of uncertainties. It requires manipulation of very large data sets on HPC, as well as complex post-processing on diverse software and data platforms. Overall, this work pushes the boundaries of current state-of-the-art in quantifying port hazard – with multi-threaded emulation platform for large-scale (1 million) predictions, built on 300 high-definition simulations on smart unstructured meshes (10 m), using massively parallel multi-GPU-enabled simulations of validated tsunami model VOLNA, and hierarchical file formats – all integrated in an overarching workflow. We illustrate the emulation framework for the Karachi port in the Makran Subduction Zone (MSZ).

The MSZ has given rise to tsunamis in 1524 [19], 1945 [20,21] and 2013 [22]. Recent studies estimate the mega-thrust potential for the eastern part of the MSZ (blue rectangle in Figure 1b) to be M_w 8.8 – 9.0 [23]. Thus, here is a pressing need for a comprehensive quantification of tsunami hazard, especially port velocities, and associated uncertainties. However, the accurate simulation of tsunami currents at shallow depths requires accurate coastline definition and bathymetry, with adequately refined meshes, over a long duration to capture the maximum. Thus, in this study, we employ spatial resolutions of 10 m for the computational mesh, 30 m for bathymetry, and 10 m for coastline, locally in the vicinity of Karachi port (Pakistan), for a total simulation time of 12 hours. Further, we employ here an earthquake source designed with segments of size 5 km \times 5 km with carefully constructed positive slip kernels to preserve fidelity to both magnitude scaling [24] and slip scaling relations [25]. The presence of a considerable sediment layer over the MSZ demands incorporation of its influence on the seabed deformation, since an appreciable amplification of up

to 60 % can be generated [26]. Section 2 describes the models and methods, Section 3 details the emulation framework, Section 4 discusses the results, and conclusions are drawn in Section 5.

2. Models, Data and Methods

In this section, we describe (a) the MSZ, (b) the finite fault apparatus and slip profile, (c) integration of the sediment amplification over the slips for generating seabed deformation (or uplift), and (d) tsunami propagation.

(a) Makran Subduction Zone

The MSZ is formed by the subduction of the Arabian plate under the overriding Eurasian plate. It extends ~ 900 km from the Ornach Nal fault ($\sim 67^\circ$ E) in the east to the Minab-Zendan-Palami fault ($\sim 52^\circ$ E) in the west [20,27,28]. The mega-thrust potential of the entire MSZ is estimated at M_w 9.07 – 9.22 [23]. Constraints imposed by GPS data resulted in three major segments and an estimated $\sim 58\%$ mean coupling ratio between the plates [27]. The subduction interface is divided into the eastern and western MSZ, with the eastern half being more seismically active. Given the scope of this work, we limit ourselves to the eastern MSZ, since tsunamis from western MSZ would have less appreciable effects on Karachi port than those arising from the western MSZ. Further, paleoseismic accounts hypothesize that the western MSZ is seismically inactive compared to the eastern MSZ [29,30].

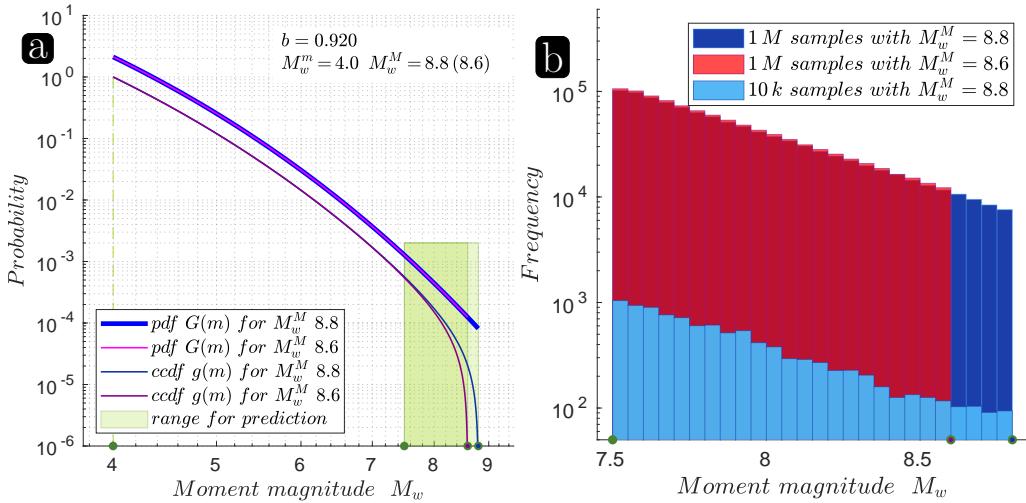


Figure 2. Magnitude-frequency distribution for the Makran Subduction Zone (MSZ). a) The Gutenberg-Richter (G-R) relation, showing probability and complementary cumulative distribution functions for two maximum moment magnitude M_w^M assumptions, viz. 8.6 and 8.8. b) Histograms of 1 million (and 10,000) samples of M_w .

Here, the probability distribution function (pdf) for the Gutenberg-Richter (G-R) relation is modeled as the doubly truncated exponential distribution [31]:

$$G(m) = \begin{cases} \frac{\beta e^{-\beta(m-M_w^m)}}{1 - e^{-\beta(M_w^M - M_w^m)}} & M_w^m \leq m \leq M_w^M \\ 0 & m > M_w^M \end{cases} \quad (2.1)$$

where $\beta = b \log_e 10$, and the lower M_w^m and upper M_w^M limits of truncation are 4 and 8.8 respectively. The upper limit of M_w 8.8 derives from the mega-thrust potential of eastern MSZ [23]. The rate parameter b of 0.92 is taken from the recent Earthquake Model of Middle East (EMME) database (see Table S2 in [32]), and refers to the whole MSZ. For the scope of this work, we assume it as representative of the eastern MSZ. The complementary cumulative distribution

function (ccdf), also called probability of exceedance or survival function is then:

$$g(m) = \begin{cases} 1 - \frac{1 - e^{-\beta(m - M_w^m)}}{1 - e^{-\beta(M_w^M - M_w^m)}} & M_w^m \leq m \leq M_w^M \\ 0 & m > M_w^M \end{cases} \quad (2.2)$$

Two cases of the truncated G-R distributions are plotted in Figure 2a, *i.e.* for maximum magnitudes M_w^M of 8.8 and 8.6. Figure 2b shows histograms of actual samples of the distribution (used later in this work).

(b) Finite Fault and Slip Profile

A finite fault (FF) on the eastern section of MSZ (blue rectangle, Figure 1b) is constructed using a total number (n_F) of 2295 rectangular segments. The overall dimension of the FF model is $420 \text{ km} \times 129 \text{ km}$ ($L^{max} \times W^{max}$). The slip on a segment is denoted by S_i , where i varies from 1 to 2295. Okada's closed-form equations transform the slips and other FF parameters into a static vertical displacement denoted by U [33]. The final vertical displacement field results from the combined superposition of vertical displacements due to all the activated fault segments. Among the FF parameters, the dip angle and fault depth (d_f) are sourced from the recent plate boundary model, Slab2 [34,35]. The strike and rake angles are kept constant at 270° and 90° .

A segment size (h_s^2) is approximately $5 \text{ km} \times 5 \text{ km}$ ($l_i \times w_i$), and the segments are arrayed in an 85×27 grid. This segment size is not chosen arbitrarily. It is selected based on a numerical study of the fidelity of the segmentation *viz.* $5 \text{ km} \times 5 \text{ km}$, $10 \text{ km} \times 10 \text{ km}$ and $20 \text{ km} \times 20 \text{ km}$ (Figure A.1a) to the earthquake dimension-magnitude scaling relation [24] (Figure 1c). The discrepancy to the scaling relation appears as discontinuities in the realizable fault lengths (L) and widths (W) (Figure A.1a inset). The size of the discontinuities are $\sim h_s$.

We use the definitions of the seismic moment $M_0 = \sum_{i=1}^{n_F} \mu l_i w_i S_i$ and moment magnitude $M_w = (2/3) (\log_{10} M_0 - 9.1)$, with $\mu = 3 \times 10^{10} \text{ N/m}^2$ being the modulus of rigidity. Our implementation of the Okada suite is adapted from the dMODELS¹ code [36,37]. Slips are usually modeled to be uniform on the FF segments, even though inversions of seismic sources evidence localised concentrations of high slips (asperities) over a backdrop of lower slips [12]. Appendix A details the construction of the non-uniform slip profile used in this work.

(c) Influence of Sediment Amplification on Seabed Deformation

Incorporation of the effect of sediments influences tsunami modelling mainly in two ways. First, the interplay of sediment transport and tsunami flow gives rise to enhanced coupled morph- and hydro-dynamics [38,39]. Second, the Okada deformation model [33], with the assumptions of an elastic, homogeneous, isotropic medium in a semi-infinite domain, can be improved by sediment models that exhibit non-linear, non-homogeneous, and an-isotropic behaviour. Considerable amplification (up to 60 % locally) of crustal deformation due to the presence of layers of sediments on the seafloor can occur [26]. In this section, we limit the incorporation of the effect of sediments to the deformation model by making use of a sediment amplification curve (Figure 3c), extracted from elastodynamic simulations of layered sediment-rock seabed [26]. The curve uses the relative depth (d_r^i) of the i th segment (Figure 3b) calculated as:

$$d_r^i = \frac{d_s^i}{d_f^i} \quad (2.3)$$

where d_s^i is the sediment thickness over the segment interpolated from GlobSed² [40], and d_f^i is the down-dip fault depth of the segment taken from Slab2 [34] (Figure 3a). Given d_r^i , the sediment amplification curve supplies the sediment amplification factor (S_a^i) on the segment

¹v1.0 available at pubs.usgs.gov/tm/13/b1/

²available from ngdc.noaa.gov/mgg/sedthick/

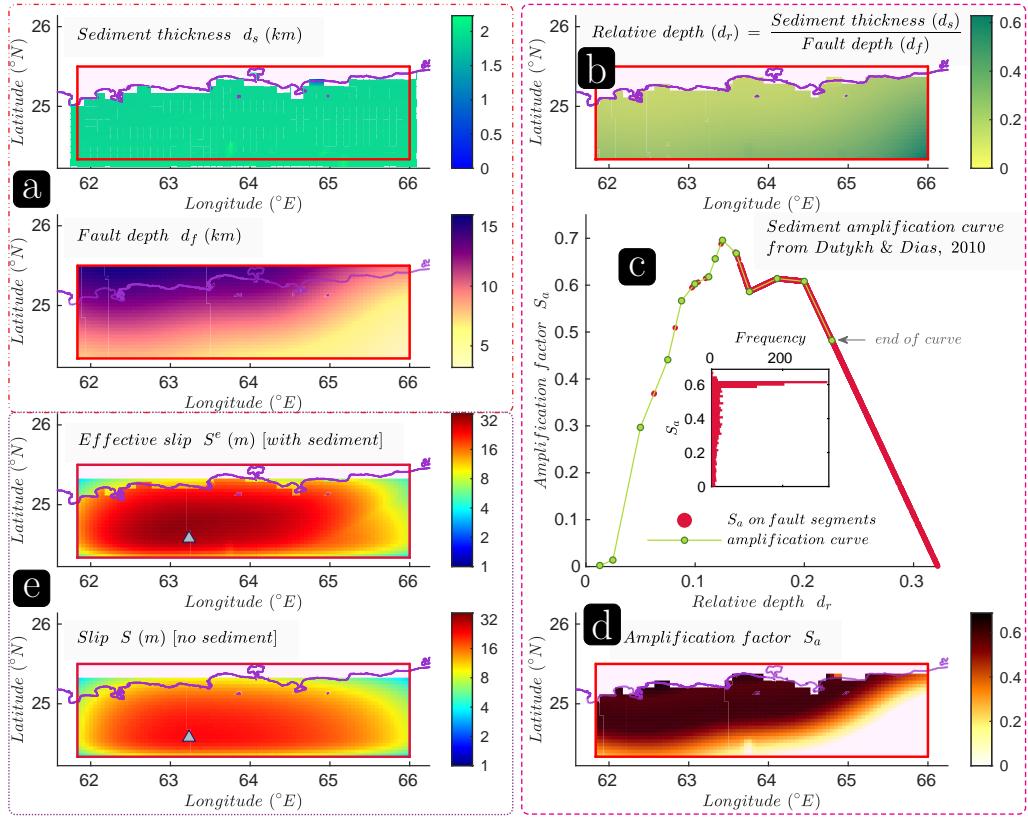


Figure 3. Sediment amplification. a) Sediment thickness d_s and fault depth d_f . b) Relative depth d_r . c) Sediment amplification curve. Inset histogram shows distribution of S_a over the FF segments. d) Sediment amplification factor S_a . e) Slip profile S (no sediments), and effective slip profile S^e incorporating influence of sediments through S_a .

(Figure 3d). The amplification due to the sediments is incorporated by multiplying the slip S^i with the sediment amplification factor S_a^i resulting in an effective slip S_a^e (Figure 3e):

$$S_a^e = S_a^i (1 + S_a^i) \quad (2.4)$$

Okada's closed-form equations transform the effective slips S_a^e into the effective vertical displacement U^e (Figure 4b) [33]. The influence of sediments not only increases the slips effectively but also modifies the profile, as evident in the emergence of a double-lobed profile (Figure 3e). The effect is more conspicuous in the associated deformations (compare Figures 4a and b). The amplification factor (S_a) peaks at a relative depth of approximately 0.13 after which it decreases. Given the geometry of the fault and overlying sediment profile, a significant number of segments have an amplification factor between 0.4 – 0.6 (or, equivalently 40 – 60 % amplification) (Figures 3c inset and d). Furthermore, the sediment amplification factor is strongly dominated by the fault depth rather than the sediment thickness which is near-uniform. The sediment amplification curve is defined only till a relative depth of 0.23 [26]. We linearly extrapolate the curve in order to be as conservative as possible in the region where it is not defined as well as to smoothly transition from regions of higher to lower fault depths. The counterparts of average slip S_{avg} and maximum slip S_{max} of S (without sediments) are defined as average effective slip S_{avg}^e and maximum effective slip S_{max}^e of S^e (with sediments). Similarly, effective moment magnitude M_w^e is defined, by replacing S_i with S_i^e in the expression of M_w . The effect of sediments on slips is compared in Figure 5a. Here, the increased scatter of S_{max}^e compared to S_{avg}^e is due to the spatial distribution of S_a , which significantly amplifies S_{max}^e depending on the epicenter (X_o, Y_o). Also, the increase in scatter of S_{max}^e as M_w decreases is due to the decrease in fault dimensions that

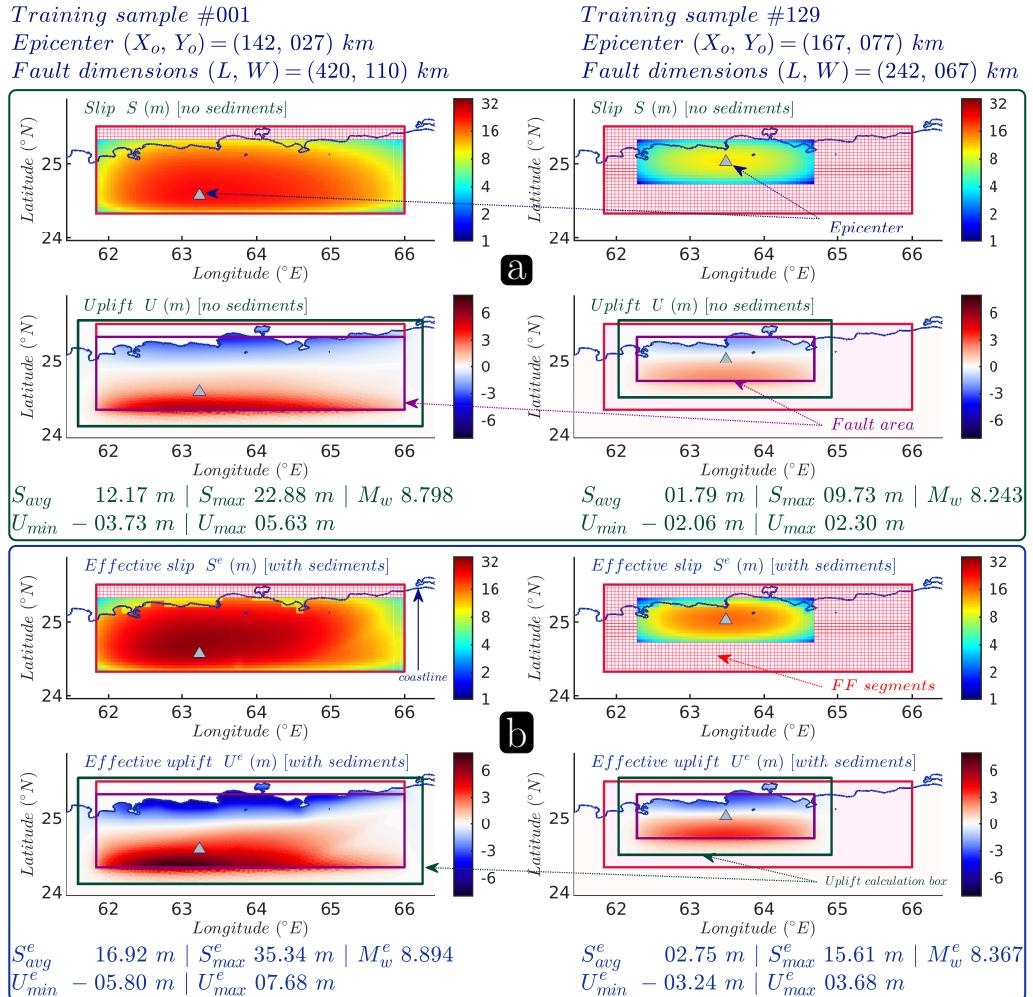


Figure 4. Comparison of slip and deformation profiles for sample nos. 1 (left column) and 129 (right column). a) Slip S and uplift U before incorporation of sediment influence. b) Effective slip S^e and uplift U^e with sediment influence. The colorbar for slip is in \log_2 scale. (See animations in the ESM for a detailed graphical overview of the 300 samples.)

allow many earthquake scenarios to be situated in areas of lower S_a . This aspect is pronounced in a similar comparison of M_w^e to M_w in Figure 5b.

(d) Tsunami Propagation

Analysing wave heights requires few hours of simulation, while investigating the velocities need longer simulation times. Thus, each scenario is run for 12 h of simulation time T_s to obtain the maximum tsunami velocity and wave height, and is therefore computationally expensive. It is not only imperative that the numerical algorithms in the computer code for tsunami simulations run efficiently at fine mesh resolutions (10 m) needed to capture the currents, but also that the code is amenable to adequate parallelisation, *e.g.* [41,42]. Thus, to run 300 such scenarios, we employ VOLNA-OP2³ that runs efficiently for unstructured meshes on parallel GPUs [18]. The number of full-fledged scenarios (*i.e.* 300) is considerably higher than in existing studies related to MSZ [43–45]. Usual simulations employ the Green's functions approach to superpose the tsunami wave heights from a multi-segment finite fault source. Here, the non-linear shallow water equations (NSWEs) model not only the propagation of the tsunami but also the run-up/down process at the

³v1.5 available at github.com/reguly/volna, with improvements to second order FV scheme and boundary conditions

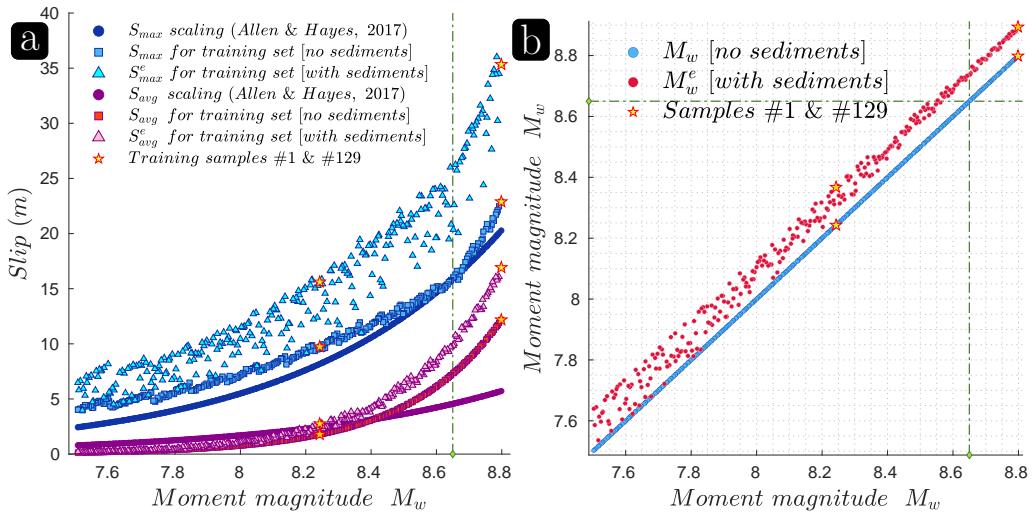


Figure 5. Effective slip and moment magnitude. a) Comparison of average and maximum slips with (S_{avg}^e , S_{max}^e) and without (S_{avg} , S_{max}) the influence of sediments for the 300 scenarios. b) Same as (a) but for moment magnitude.

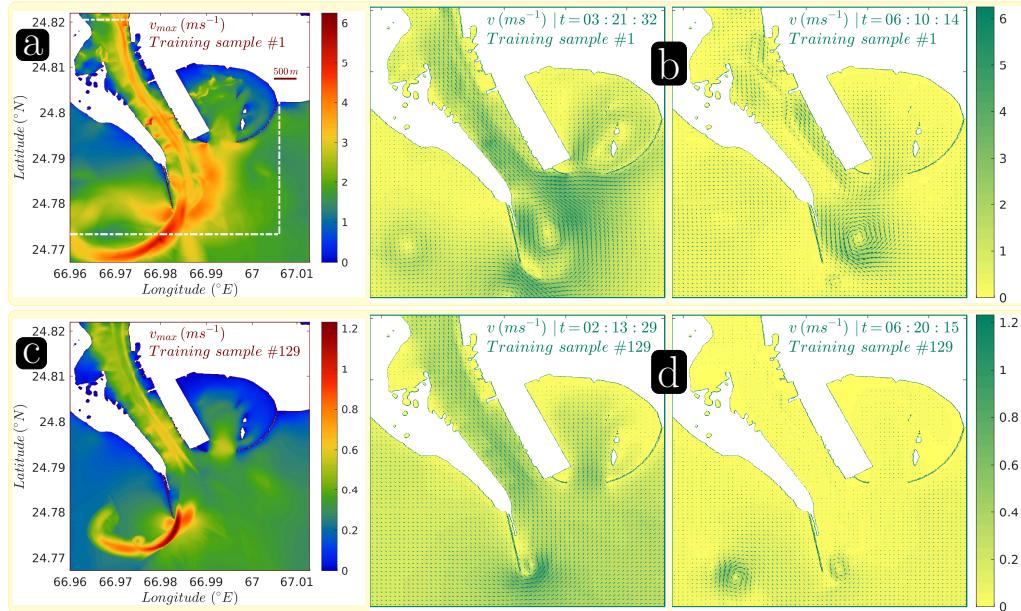


Figure 6. Tsunami velocity. a) Maximum velocity at Karachi port over 12 h for sample no. 1. b) Two snapshots of velocities for sample no. 1 restricted to the box (dashed line) in (a). c-d) Same as (a-b) but for sample no. 129.

coast [46]. The finite volume (FV) cell-centered method for tessellation of control volume is used in VOLNA, and the barycentres of the cells are associated with the degrees of freedom. Details of numerical implementation, validation against standard benchmarks and comprehensive error analysis are available [18,47]. An important factor affecting the fidelity of long-lasting simulation of currents is numerical dissipation. Giles *et al* [48] studied the numerical errors in VOLNA-OP2, wherein they are analysed by decomposing them into dispersion and dissipation components. Furthermore, an inter-model benchmarking of different numerical models highlighted the pitfalls in high-resolution current simulations [6]. In line with the scope of this work, we limit our numerical studies using VOLNA-OP2. It may be noted that although the emulation framework is independent of the specific numerical model employed, the accuracy of the emulator is limited

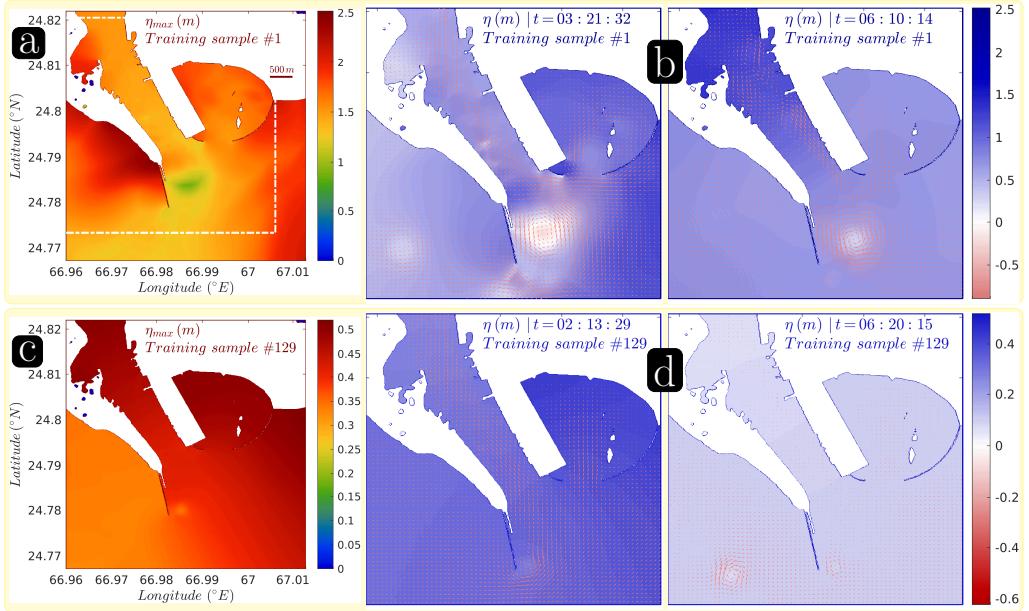


Figure 7. Tsunami height. a) Maximum height at Karachi port over 12 h for sample no. 1. b) Two snapshots of heights for sample no. 1 restricted to the box (dashed line) in (a). c-d) Same as (a-b) but for sample no. 129.

by the accuracy of the underlying numerical model. VOLNA models the tsunami life-cycle with:

$$\frac{\delta H}{\delta t} + \nabla \cdot (H \mathbf{v}) = 0 \quad (2.5)$$

$$\frac{\delta H \mathbf{v}}{\delta t} + \nabla \cdot \left(H \mathbf{v} \otimes \mathbf{v} + \frac{g}{2} H^2 \mathbf{I}_2 \right) = g H \nabla b \quad (2.6)$$

where $H(\mathbf{x}, t) = b + \eta$ is the total water depth defined as the sum of free surface elevation $\eta(\mathbf{x}, t)$, and time-dependent bathymetry $b(\mathbf{x}, t)$. The two horizontal components of the depth-averaged fluid velocity are in $\mathbf{v}(\mathbf{x}, t)$, g is the standard gravity and \mathbf{I}_2 is the 2×2 identity matrix. The maximum tsunami velocity v_{max} and wave height η_{max} at location \mathbf{x} at time t are computed as:

$$v_{max}(\mathbf{x}) = \max_{0 < t \leq T_s} \|\mathbf{v}(\mathbf{x}, t)\|_2 \quad (2.7)$$

$$\eta_{max}(\mathbf{x}) = \max_{0 < t \leq T_s} \eta(\mathbf{x}, t) \quad (2.8)$$

The dynamic bathymetry $b(\mathbf{x}, t)$ is the sum of static bathymetry $b_s(\mathbf{x})$ and U^e , the effective deformation due to the influence of sediments. Here, an instantaneous fault is assumed, *i.e.* U^e is supplied once at the beginning of the simulation. Further, to reduce the computational burden of calculating deformations from 300 events, U^e is computed only within a uplift calculation box covering the fault (green box in Figure 4).

Accurate bathymetry, precise coastline, and good quality computational mesh are vital for a proper modelling of velocities and currents in shallow water and near the coast. Thus, b_s uses GEBCO 2019 (15" resolution) [49] complemented with hydrographic charts for Karachi port (~ 30 m resolution), and SRTM v3 topography (1" resolution) [50]. For delineating port structures and breakwaters along the coastline, Google Earth's satellite imagery (~ 10 m resolution) is used. The merging is described in Appendix B. The non-uniform unstructured mesh is designed in three stages corresponding to three regions, *viz.* offshore, onshore and near the port. This three-pronged strategy strikes a balance between, having a fine mesh resolution (10 m) near Karachi port, and reducing the overall computational cost with $\sim 2.64 \times 10^6$ triangles in total. The mesh is generated using Gmsh⁴ [51]. The construction of the mesh is described in Appendix C.

⁴v4.4.1 available at gmsh.info

The outputs v_{max} and η_{max} for two training samples (nos. 1 and 129) are plotted in Figures 6 and 7 respectively, alongside snapshots taken at various time instants during the simulation.

3. Statistical Emulation

In this section, emulator (a) training, (b) diagnostics, and (c) predictions for 1 million events are described.

(a) Emulator Construction

The numerical simulation of the tsunami life cycle, *i.e.* its generation, propagation and inundation at fine mesh resolutions is computationally expensive due to model non-linearity, and typically consumes hours on supercomputers. This is all the more prohibitive for a probabilistic quantification where thousands of runs of the tsunami code are required to exhaust the range of plausible scenarios. Statistical surrogates (or emulators) provide a computationally cheap approximation of the complex tsunami solvers, together with estimates of uncertainties in the predictions. In this study, the three input model parameters are moment magnitude (M_w) and epicenter co-ordinates (X_o, Y_o) (Figure 1c inset). The co-ordinates have their origin as the southwest corner of the MSZ. The inputs are transformed into effective seafloor deformation. The consequent tsunamis are propagated till Karachi port. The outputs of interest in our case are the maximum wave height (η_{max}) and maximum wave velocity (v_{max}) at n_G (193) virtual gauge locations around the port.

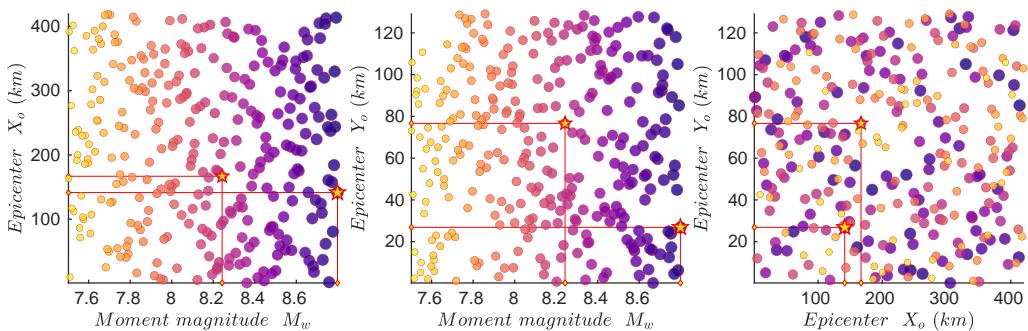


Figure 8. 300 training scenarios of input parameters (M_w, X_o, Y_o) generated by Latin Hypercube Design – projections on $M_w - X_o$ (left), $M_w - Y_o$ (centre), and $X_o - Y_o$ (right) planes. Sample nos. 1 and 129 are marked with stars.

Thus, the computer code (denoted by \mathbb{M}) simulates a multi-physics two-stage physical model, *i.e.* from the input parameters (M_w, X_o, Y_o) to deformation U^e , then from U^e to tsunami outputs v_{max} and η_{max} . An essential stage is the creation of an informative data set for constructing the emulator. This is also called the design of computer experiments and the data set is termed as the training set. The specific purpose of the design stage is to capture the functional relationship between the input parameters (M_w, X_o, Y_o) and output quantities (η_{max}, v_{max}) at a location. The Latin Hypercube Design (LHD) generates a set of points that are nearly uniformly spread to cover the input parameter space. Specifically, it maximises the minimum distance between points in the set, a feature that explores the functional relationship better than a random scatter. In a physical sense, this spread of points endeavours to capture the information inherent in the input-output relationship as much as possible. The model is evaluated by computer runs of \mathbb{M} at the training points. Here, we employ an LHD of size 300 for 3 parameters (Figure 8). This is large enough to capture complex nonlinear combined sensitivities to the input parameters (*e.g.*, the influence of size and location in relatively small and mid-size events closer to Karachi, or large regional variations in spatial distributions of slips), but still fits within our computational budget. The Gaussian Process (GP) emulator (denoted by \mathcal{M}) interpolates across the input-output points in the training set. In other words, the constructed emulator works as an approximation

of M , and can be used to generate predictions (or, evaluated) at any point in the space of input parameters. The predictions will be exact at the training points, but uncertain elsewhere. This uncertainty is modeled by a normal distribution whose mean and standard deviation are calculated using the Kriging formula (mean quantities denoted by \bar{v}_{max} and $\bar{\eta}_{max}$) explicitly accounting for the design. This structure allows for any nonlinear relationship to be modelled with uncertainties dependent on the location of the design points, unlike in more standard linear or even non-linear regressions where the structure is fixed *a priori*. Derivations and exact equations can be found in Beck and Guillas [52]. GP emulation has been instrumental in successfully quantifying uncertainties in tsunami heights generated by landslides over the North Atlantic and the Western Indian Ocean as well as earthquakes over Cascadia [13,53–55]. We use the the efficiently implemented Multiple-Output Gaussian Process emulator (MOGP)⁵ for emulation.

The covariance kernel is a key component in the construction of the emulator. Here, we use the Matern 5/2 kernel that is smooth enough to avoid a rough GP, but not extremely smooth thus being suitable for modelling the physics. The piecewise polynomial, rational quadratic, exponential, and squared exponential functions are other candidates [56]. The parameters (or length scales) in the kernels and other hyperparameters are found via non-linear optimization (L-BFGS-B) using maximum likelihood estimation (MLE). MOGP also entertains Bayesian approaches as well as a selection of optimization algorithms.

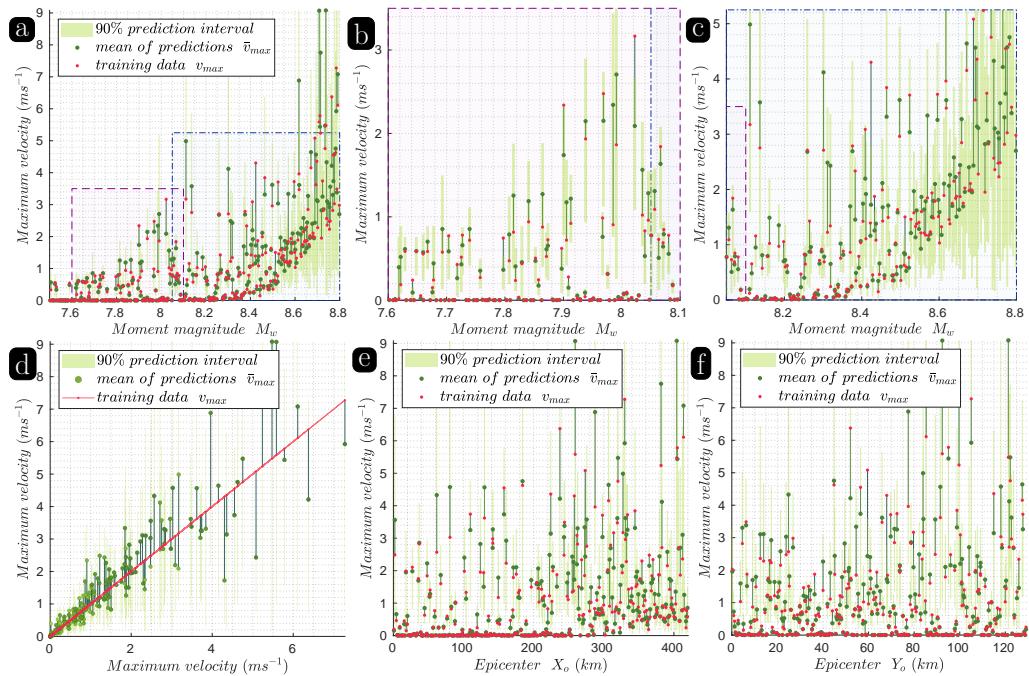


Figure 9. Emulator diagnostics (maximum velocity). a) L-O-O data for emulation of maximum velocity v_{max} at a gauge in Karachi port (gauge no. 91). The vertical line segments connect the training data to its predicted counterpart. b) Enlargement of lower moment magnitude region in (a). c) Enlargement of higher moment magnitude region in (a). d) Data in (a) on predicted \bar{v}_{max} – training v_{max} axes. e) Data in (a) on X_o -axis. f) Data in (a) on Y_o -axis.

Maximum velocity magnitudes (and heights) are positive. In order to respect this physical constraint and not predict negative velocities (and heights), we feed the logarithm of v_{max} (and η_{max}) into the construction of the emulator. Since the constructed emulator is now in the logarithmic scale, we transform the predicted quantities back to the original scale by accounting for the lognormal nature of the predicted distributions. Hence, the confidence intervals for the

⁵v0.2.0 available at github.com/alan-turing-institute/mogp_emulator

predictions, representing uncertainties, are all rendered positive, and naturally skewed in that direction. Once the emulator is constructed, it needs to be validated before employing it for predictions.

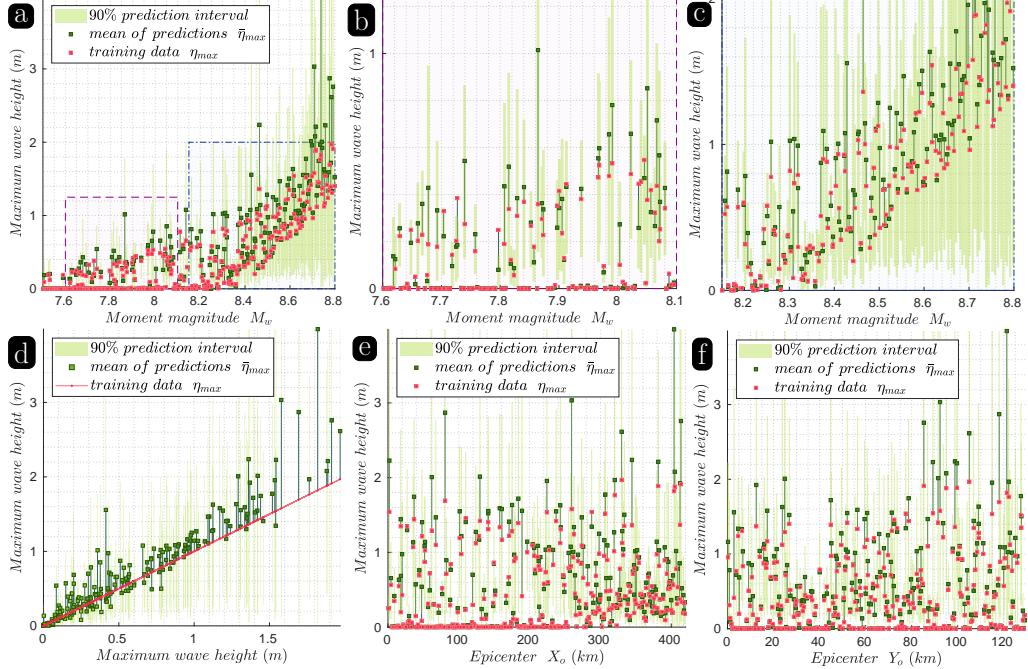


Figure 10. Emulator diagnostics (maximum height). a) L-O-O data for emulation of maximum height η_{max} at a gauge in Karachi port (gauge no. 91). The vertical line segments connect the training data to its predicted counterpart. b) Enlargement of lower moment magnitude region in (a). c) Enlargement of higher moment magnitude region in (a). d) Data in (a) on predicted $\bar{\eta}_{max}$ – training η_{max} axes. e) Data in (a) on X_o -axis. f) Data in (a) on Y_o -axis.

(b) Emulator Diagnostics

In order to validate the quality of the emulation, we provide Leave-one-out (L-O-O) diagnostics here. Our training set consists of 300 pairs of input-output quantities. In L-O-O, a reduced training set of 299 pairs is employed to build an emulator, which is then used to predict the output at inputs of the 1 pair that was left out. The predicted output (and its uncertainty) is compared to the actual output of the left out pair. This procedure is repeated 300 times to cover all the pairs in the training set. These tests are passed by the emulator, as seen for predicted \bar{v}_{max} in Figure 9 and $\bar{\eta}_{max}$ in Figure 10. The comparison between the mean of predictions from the emulator \mathcal{M} and the training data from the tsunami simulator \mathbb{M} shows that the emulator approximates well the simulator. The vertical line segments connect the predicted mean with its counterpart in the training data. More importantly, the uncertainties in the predicted mean, quantified in the form of 90% prediction intervals (green bars in Figures 9 and 10), represent well the uncertainties about these predictions (or are even slightly conservative), since around 90% or more of the outputs from the training set fall within these intervals. GP approximation works well inside the convex hull of the training points, but deteriorates near the hull's boundary or exterior giving rise to larger uncertainties in the predictions. For our design, these locations include design limits of M_w , and corners or boundaries of the FF, which are limits of (X_o, Y_o) . The L-O-O diagnostic indeed shows inadequate fit and larger uncertainties in these regions of the input space. Still, L-O-O provides validation of the emulator inside the convex hull. Further, the L-O-O diagnostics show that some of the lower M_w events do not generate appreciable velocities. In these cases, the location with respect to the port is such that negligible wave energy

is radiated to the port. Conversely, the low M_w events that do show appreciable velocities are located such that considerable wave energy reaches the port. The L-O-O also shows a decrease in this positional dependence as M_w increases, due to an accompanying increase in fault area and energy. Additionally, numerical dissipation in the model does play a role here, and numerical schemes tailored for reducing numerical dissipation would increase the accuracy [48].

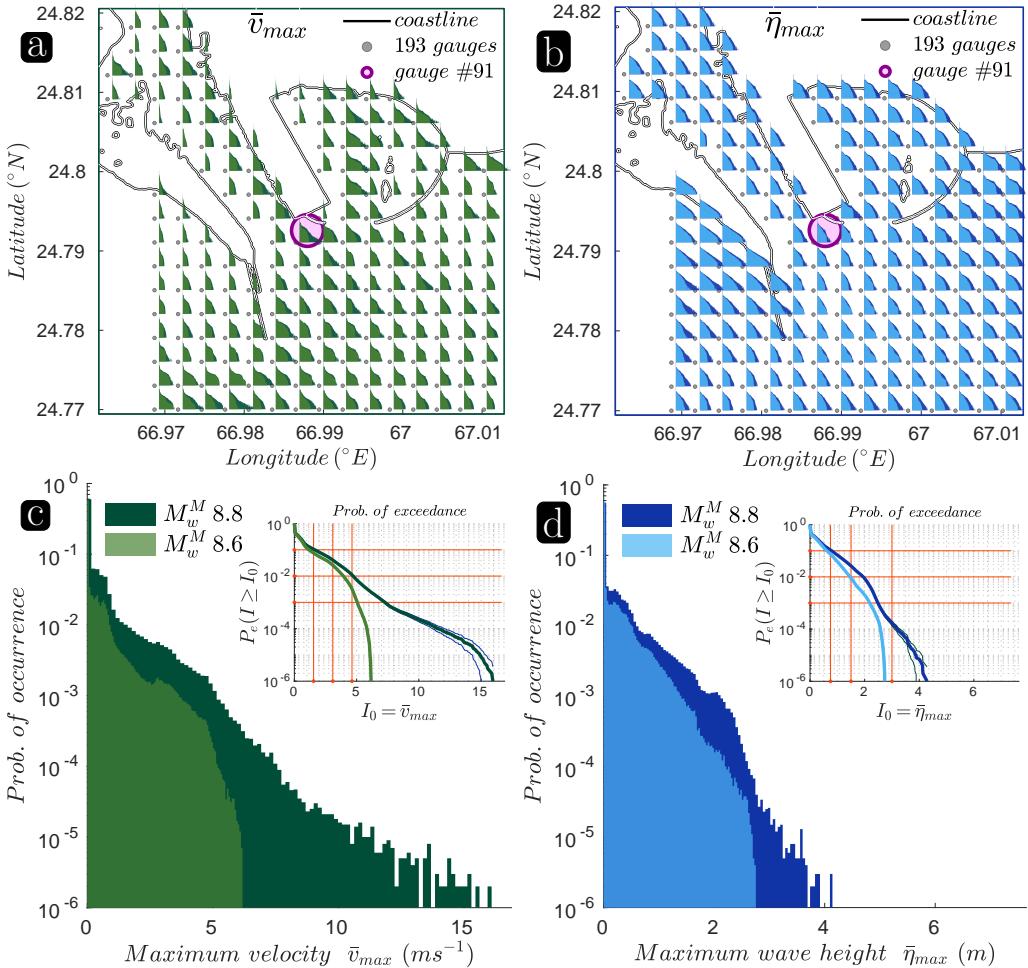


Figure 11. 1 million emulator predictions at 193 gauges. a) Histograms of predicted maximum velocities \bar{v}_{max} . Histograms from maximum moment magnitude M_w^M of 8.8 and 8.6 are superimposed. b) Same as (a) but for predicted maximum heights $\bar{\eta}_{max}$. c) Normalized histograms of \bar{v}_{max} at gauge no. 91. Inset shows probability of exceedance curves, with 99 % confidence interval. d) Same as (c) but for $\bar{\eta}_{max}$.

(c) Emulator Predictions

Although the 300 simulations by itself generate a good description of the hazard, a large number of scenarios are essential for a comprehensive probabilistic hazard assessment. Thus, we evaluate the model at n_P (1 million) values of (M_w, X_o, Y_o) at 193 virtual offshore gauges. The constructed emulator is used to evaluate the model at inputs that are different from those in the training set. These evaluations are termed predictions. A prediction returns the mean value of the emulated quantity and a measure of inherent statistical error/uncertainty in the approximation, *e.g.* the standard deviation. Cumulatively, these 193 million predictions not only comprehensively cover the geography around Karachi port, but also exhaustively sweep through the range of

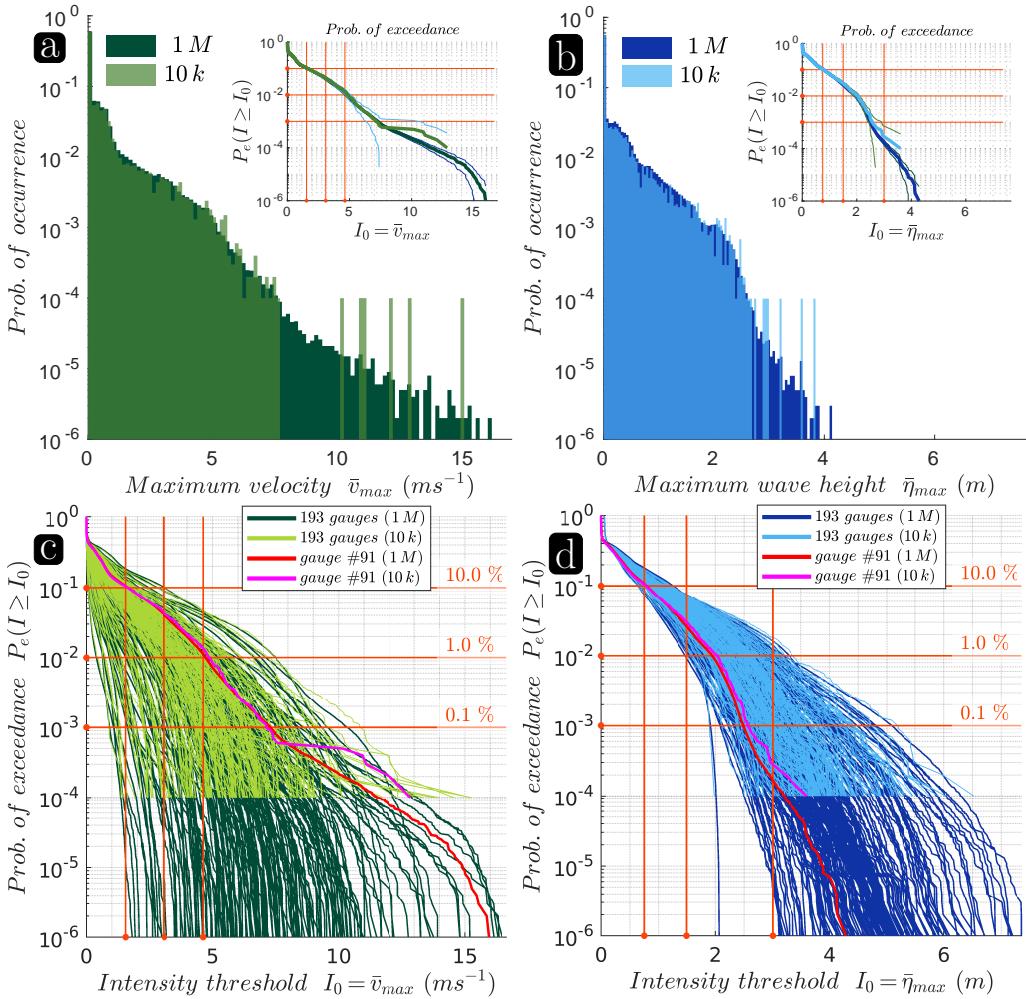


Figure 12. Hazard curves. a) Comparison of normalized histograms of 1 million (1M) and 10,000 (10k) predicted maximum velocities \bar{v}_{max} at gauge no. 91. Inset shows probability of exceedance P_e curves, with 99 % confidence interval. b) Same as (a) but for predicted maximum heights $\bar{\eta}_{max}$. c) P_e curves for \bar{v}_{max} at 193 gauges. Curves from 10k predictions are superimposed on those from 1M predictions. Probability and intensity threshold values used to generate hazard maps are shown as horizontal and vertical lines respectively. d) Same as (c) but for $\bar{\eta}_{max}$.

events in the magnitude-frequency distribution. Additionally, such a high number of samples is also needed to thoroughly explore the interplay among the three parameters in the input space of (M_w, X_o, Y_o) .

The M_w for the 1 million events are obtained by sampling the truncated Gutenberg-Richter (G-R) distribution for the MSZ within our region of interest, *i.e.* M_w 7.5 to M_w 8.8 (Figure 2a). The lower limit of M_w 7.5 is chosen for illustrative purposes. The 1 million values of (X_o, Y_o) are sampled from a uniform distribution defined over the rectangle $[0 L^{max}] \times [0 W^{max}]$ of area $420 \text{ km} \times 129 \text{ km}$. Any changes in the parameters of the G-R relation (*i.e.* β , M_w^m , M_w^M etc.) only affect the earthquake samples generated for the prediction stage. These changes can be handled in a very efficient manner as the prediction stage is the cheapest component in the entire workflow. In fact, cheap prediction permits fast propagation of uncertainties in the G-R parameters to the hazard intensities. Here, we demonstrate this for two values of one such parameter, the maximum magnitude M_w^M . Assuming a reduction of maximum magnitude M_w^M from 8.8 to 8.6 gives a perturbed G-R relation (Figure 2a). In this case, the 1 million samples come from the range M_w 7.5

to M_w 8.6. The histograms of 1 million samples for M_w are shown in Figure 2b. It also shows 10,000 samples from the range M_w 7.5 to M_w 8.8 for performing comparisons.

To be able to generate 1 million predictions, we employ MOGP. Once the predictions are finished, we are left with two histograms (one each for \bar{v}_{max} and $\bar{\eta}_{max}$) at every virtual gauge, each made up of 1 million samples of predicted quantity. The histograms are processed to extract $P_e(I(\mathbf{x}) \geq I_0)$, the probability of exceedance. P_e is the probability of the tsunami having $I(\mathbf{x}) \geq I_0$ at a gauge \mathbf{x} . The intensity I is the measure of hazard, *i.e.* either \bar{v}_{max} or $\bar{\eta}_{max}$, and I_0 is the intensity threshold for the hazard quantity under consideration.

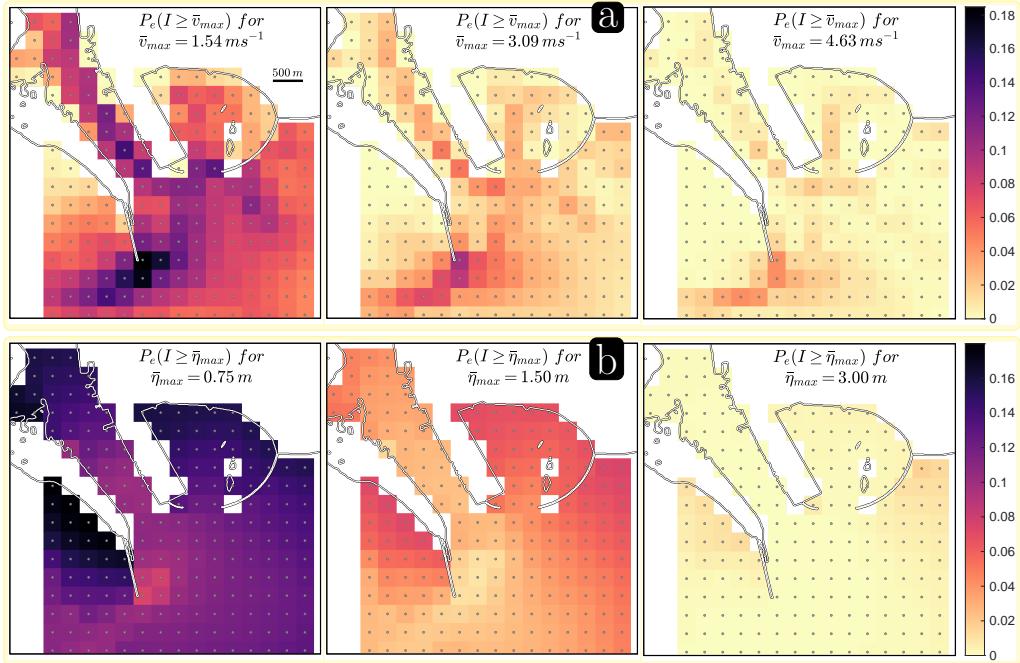


Figure 13. Hazard maps. a) Probability of exceedance at 193 gauges for predicted maximum velocities \bar{v}_{max} of 1.5 (left), 3 (centre) and 4.6 ms^{-1} (right). b) Same as (a) but for predicted maximum heights $\bar{\eta}_{max}$ of 0.75, 1.5 and 3 m.

4. Results and Discussion

We first plot the raw output from the 1 million predictions, *i.e.* the histograms at 193 gauges in Figures 11a-b. At each gauge, two histograms are superimposed on each other. These correspond to the two G-R relations with varying maximum moment magnitude assumptions, *i.e.* M_w^M 8.6 and M_w^M 8.8 (Figure 2). The histograms also act as visual indicators for the measure of the hazard at the gauge, and will be cast as hazard maps in Figures 13 and 14. Near the tip of breakwaters and the mouth of the harbor, we observe relatively higher velocities than in other regions. We also observe a complementary relation between the histograms of velocities and wave heights: the gauges having thicker histograms for velocity have thinner histograms for wave heights and *vice versa*. These phenomena can also be observed in the snapshots (compare Figure 6b with 7b).

As expected, there is a clear reduction of hazard when the maximum moment magnitude is reduced. For closer inspection, we enlarge the normalized histograms at gauge no. 91 in Figures 11c and d. Gauge no. 91 is located in the center of the map near the mouth of the port and is chosen since there is substantial spread of both maximum velocities and wave heights in its histograms. In Figure 11c, the normalized histograms for maximum velocity are plotted. The range of velocities for M_w 8.8 extends till $\sim 16 \text{ ms}^{-1}$, while it extends to only $\sim 6.2 \text{ ms}^{-1}$ for M_w 8.6. Thus, we observe a $\sim 61\%$ reduction in maximum velocity hazard for a M_w 0.2 reduction in maximum moment magnitude. In comparison, for the same reduction in maximum moment

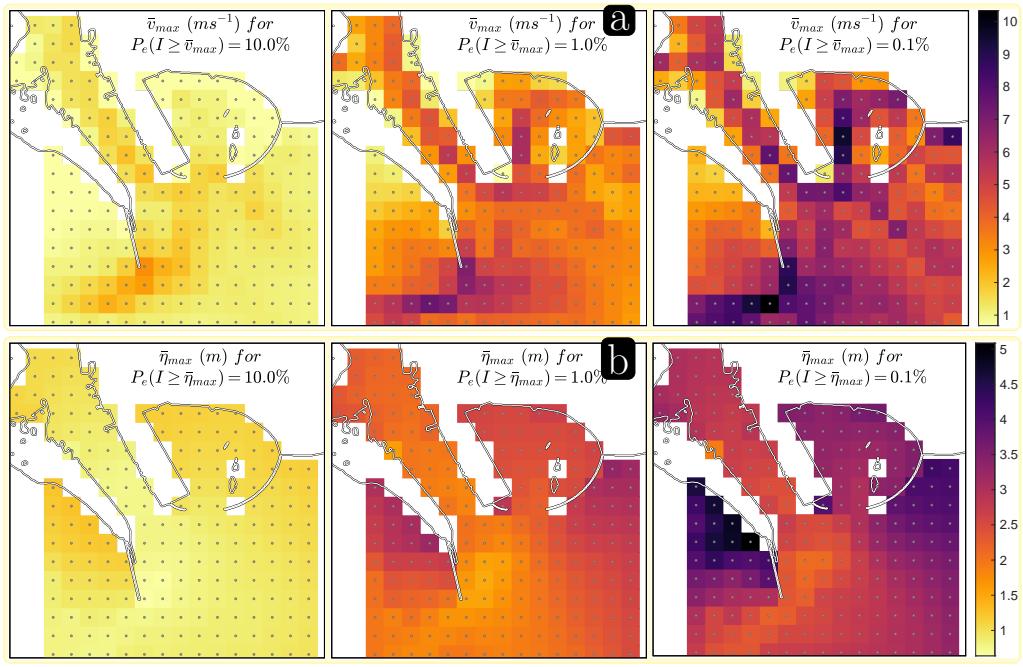


Figure 14. Hazard maps. a) Predicted maximum velocities \bar{v}_{max} for exceedance probabilities of 10^{-1} (left), 10^{-2} (centre) and 10^{-3} (right). b) Same as (a) but for predicted maximum heights $\bar{\eta}_{max}$.

magnitude, the reduction in hazard from maximum wave height is only $\sim 38\%$ (from $\sim 4.5\text{ m}$ to $\sim 2.8\text{ m}$ in Figure 11d). The probability of exceedance P_e that is extracted from the histograms is plotted in the inset of the respective figure.

Figures 12a and b compare normalized histograms for 1 million (1M) and 10,000 (10k) samples of input parameters (Figure 2b). The corresponding probability of exceedance P_e plots with their 99% confidence intervals can be seen in the inset. In Figure 12a, we observe that the histogram corresponding to 10,000 predictions is curtailed around 7.5 ms^{-1} and becomes very sparse for higher velocities. This is due to a deficit of samples that results in the isolated bars for higher velocities. This behaviour also translates into larger uncertainties (or wider confidence intervals) for estimates of low probabilities of P_e . In contrast, 1 million predictions adequately sweep through the entire range of velocities resulting in lower uncertainties (or narrower confidence intervals) for the tail probabilities. It may be noted that tail probabilities in the P_e curve correspond to extreme events with higher velocities. Similar behaviour is seen in Figure 12b, where the deficit of samples is observed for maximum wave heights higher than 2.7 m for the case of 10,000 predictions.

In Figures 12c and d, we plot the probability of exceedance curves extracted from the histograms of 1 million predictions for the 193 gauges. Superimposed on top are the P_e curves for 10,000 predictions. The horizontal lines in the plots are the chosen values of probability of exceedance, 10^{-1} , 10^{-2} and 10^{-3} , progressively decreasing by an order of magnitude. The vertical lines in Figure 12c denote maximum velocities of 1.5 , 3.1 and 4.6 ms^{-1} (or 3, 6 and 9 knots respectively), values that demarcate categories of damage [5]. The vertical lines in Figure 12c denote maximum wave heights of 0.75 , 1.5 and 3 m . These values are used to construct hazard maps in Figures 13 and 14. In both Figures 12c and d, the reach of the P_e curve is extended beyond the low probability of 10^{-4} to include even extreme events only in the case of 1 million predictions. Additionally, although the lower probabilities (around 10^{-4}) have been made accessible by 10 thousand events, they require 1 million events for accurate resolution: with only 10,000 samples, both probabilities and quantities are overestimated between 10^{-3} and 10^{-4} . Hence, being able to produce a very large number of predictions is crucial to hazard assessment.

Only with the utilization of the emulator – needing only 300 simulations – are we able to afford realistic predictions of velocities and wave heights at high resolution.

Port hazard is represented on maps by velocity zonations, a time-threshold metric and safe depths for vessel evacuation [3,5]. In this work, the probability of exceedance curves in Figure 12 are cast as hazard maps [7,8]. We plot the probability of exceedance at the 193 gauges on the map for the chosen values of maximum velocities in Figure 13a. Similar plots for chosen values of maximum wave heights are shown in Figure 13b. For both velocities and wave heights, the overall probability decreases as the intensity threshold increases. Specifically, the bulk of P_e for maximum velocities is concentrated at the tip of breakwaters and along the dredged channel leading into the port (seen in port bathymetry, Figure B.1j), as also observed in Lynett *et al* [4]. This is also supported by the patterns of localised higher maximum velocities in Figures 6a and c. In contrast, the spatial distribution of P_e for maximum wave height shows a complementary behaviour and is more spread out.

Conversely, for chosen probabilities of exceedance, the corresponding hazard thresholds at the gauges are plotted in Figures 14. As expected, the overall intensity thresholds increase with decrease in probability of exceedance. Again, the bulk of the maximum velocity threshold is concentrated at the tip of breakwaters and along the dredged channel (Figure 14a). Here too, we see a complementary behaviour for maximum wave height in Figure 14b.

Velocities have more spatial variation than heights [57], and show increased sensitivity to port configurations, compared to wave heights [58]. The larger spatial variation of velocities in Figure 12c compared to wave heights in Figure 12d is evident in the probability of exceedance plotted for all the gauges. This can be attested in Figures 11a and b, where the bulkiness of velocity histograms varies spatially much more than that of the heights. Additionally, at a given gauge, we observe that the spread of velocities is much more than those of the heights for the same set of earthquake scenarios, *e.g.* compare Figures 11a and b for gauge no. 91. These behaviours can also be observed for individual runs from the spatial variations of maximum velocity and wave height, compare panels (a) and (c) in Figure 6 to those of Figure 7.

The probability of exceedance extracted in this work acts as the basic input for common hazard outputs of probability of occurrence (and return periods), especially the ~ 2475 year mean return period for the Maximum Considered Tsunami (MCT) as laid out in Chapter 6 of ASCE 7-16 [59]. It also feeds into loss estimation functions [60]. Although, a full/complete probabilistic description of hazard may remain elusive, a realistic goal of ‘fullness’ will be to carefully define and perform each step in the PTHA. In these terms, a ‘full’ probabilistic assessment would ideally need to include further sources of uncertainties, including a thorough analysis of the source uncertainties in its seismic and tectonic setting. These include layers of uncertainties that are either epistemic or aleatoric in nature. Epistemic uncertainties include the scaling relation, and the Gutenberg-Richter approximation of the occurrence-magnitude relationship [61], *i.e.* both the maximum moment magnitude and the b -value. For MSZ, the major influence of the maximum magnitude was illustrated in an initial work [62], with a simplified tsunami modelling strategy. Here, we only assess two cases, for M_w^M 8.6 and M_w^M 8.8. Uncertainties in the near-shore bathymetry also have a large influence on near-shore hazard [63]. Further, the entire MSZ need be modelled for an area-wide assessment of hazard at the major ports in Pakistan, Iran, Oman, and India, whilst accounting for crustal, outer-rise and imbricate faults. Secondary tsunamigenic effects from earthquakes in the continental crust (submarine slumps, and slides) need additional parameters, *e.g.* 27 November 1945 M_w 8.1 [64], and 24 September 2013 M_w 7.7 [22] events. Similarly, with appropriate additional parameters, outer-rise and splay faults can be incorporated into the source, *e.g.* barrier models. Although a large increase in the number of parameters (especially for spatial fields of parameters) presents a challenge to emulation, a solution presents itself in the combination of dimension reduction and emulation [63].

Aleatoric uncertainties in the variations of the geometry in the seafloor uplift and subsidence can be readily incorporated. An alternative to our slip profile generation is to directly parameterize the co-seismic deformation profile using 3 parameters (or more) [55]. The Okada

model that transforms the slips to the vertical deformation is then bypassed. This route is quite attractive since it allows the creation of very realistic deformation patterns with a fixed number of parameters, and does away with the dependency of the deformation/slip on the resolution of the segmentation (shown in Figure A.1a inset). Our work uniformly samples the 1 million samples for epicenter co-ordinates (another aleatoric uncertainty). However, a recent spatial distribution of locking has been made available for the MSZ [27]. It would be even more realistic to sample the epicenter coordinates using the locking distribution, since zones of high locking act as a major cause for earthquake recurrence as recently hypothesised [65]. The locations could be further distributed based on the depth-dependent rigidity [66].

Randomness in tide levels at the time of impact (consequent changes of up to 25% reported [67]) could be included. A better approximation of the currents would be through 3-D modelling that accounts for fluid behaviour of the vertical water column and variable vertical flow [6,68]. Better designs of computer experiments than the Latin Hypercube Design could be employed to reduce uncertainties in the emulator's approximation, such as sequential design [52]. Instead of investigating a range of scenarios, if one only wants to examine the maximum wave height in order to build defences for instance, a recent surrogate-based optimization could be pursued whereby the design of experiment is combined with a search for the maximum, saving large quantities of computational time and increasing accuracy due to the focus on the optimization [69]. To be able to emulate a sequence of multiple models of seabed deformation and tsunami propagation, and possibly a 3-D model of currents locally, a new approach, called integrated emulation allows even better designs [70]. The most influential models are run more times where it matters, and the integrated emulator propagates uncertainties with higher fidelity by taking into account the intermediate models in the system of simulators. This approach has the potential to enable fully realistic end-to-end coupling of 3-D earthquake sources models with tsunami models [71].

5. Conclusions

In this paper, we provide a novel end-to-end quantification of uncertainties of future earthquake-generated tsunamis heights and currents in the MSZ:

(i) We replace the complex, expensive high-resolution tsunami simulator by a functionally simple, cheap statistical emulator trained using 300 tsunami simulations at 10 m mesh resolution in the vicinity of the port. We propagate uncertainties from the Gutenberg-Richter relation to tsunami impacts of maximum velocities and wave heights in the port area of Karachi, Pakistan. We observe maximum (extreme event) velocities and wave heights of up to 16 ms^{-1} and 8 m respectively for the range $M_w 7.5 - 8.8$ (Figure 11).

(ii) We perform the largest emulation using 1 million predictions/source scenarios. To our knowledge, this is the first large-scale uncertainty quantification of earthquake-generated tsunami current hazard. We are able to display the necessity of this very large number of predictions for resolving very low probabilities of exceedance ($< 10^{-3}$) - very high impact extreme events ($v_{max} > 7.5\text{ ms}^{-1}$ and $\eta_{max} > 3\text{ m}$) with tighter uncertainties (Figure 12).

(iii) We observe that reduction in hazard due to a reduction in maximum moment magnitude is more for velocities than wave heights. Near the mouth of the harbor, the reduction in hazard is $\sim 61\%$ for maximum velocity, but only $\sim 38\%$ for maximum wave height (corresponding to a reduction in maximum moment magnitude from 8.8 to 8.6) (Figure 12c).

(iv) We generate the first area-wide probabilistic hazard maps of tsunami currents from 1 million predicted scenarios at the Karachi port (Figures 13a and 14a). It shows patterns that are geophysically meaningful and important for the next steps of disaster risk reduction. We identify concentrations of high probability of exceedance around the port for given intensity threshold (a maximum of $\sim 18\%$, 10% and 4% for 3, 6 and 9 knots respectively) (Figure 13a). Conversely, the same regions also have high intensity thresholds given probability of exceedance (a maximum of

$\sim 3.1, 7.5$ and 10.3 ms^{-1} for $10\%, 1\%$ and 0.1% respectively) (Figure 14a). Overall, without the large-scale emulation, such outputs would be impractical to produce due to computational costs.

(v) We display more spatial variations for maximum velocity compared to wave heights around the port and their complementary behaviour for the aggregate of 1 million scenarios (Figures 6, 7, 11, 12, 13, and 14).

Algorithm 1 Slip profile generation

- 1: For a given earthquake moment magnitude M_w , find the fault length L and width W from the scaling relation.
- 2: Fit the fault rectangle of size $L \times W$ into the FF. There are two possibilities, the epicenter (X_o, Y_o) being located: (i) at the centre of the fault and equidistant from the boundaries of the fault rectangle, *i.e.* with distances $L/2$ and $W/2$, and (ii) away from the centre of the fault. In this case, (X_o, Y_o) is not equidistant from the boundaries of the fault rectangle.
- 3: Use Eqn. A.2 to construct the lobes $\phi(x; r_E, \alpha)$ and $\phi(x; r_W, \alpha)$ and form the bi-lobed kernel for fault length $\Phi(x; r_W, r_E, \alpha)$. Similarly, form the bi-lobed kernel for fault width $\Phi(x; r_N, r_S, \alpha)$ by constructing the lobes $\phi(x; r_N, \alpha)$ and $\phi(x; r_S, \alpha)$.
- 4: Use Eqn. A.3 to construct the tensor product Φ^\otimes of $\Phi(x; r_W, r_E, \alpha)$ and $\Phi(x; r_N, r_S, \alpha)$.
- 5: Multiply the values of Φ^\otimes at the centres of each segment (*i.e.* Φ_i^\otimes) with a factor $M_w \left(\sum_{i=1}^{n_F} \mu_i w_i \Phi_i^\otimes \right)^{-1}$ to get the slip S_i on the segment.

A. Slip Profile Generation

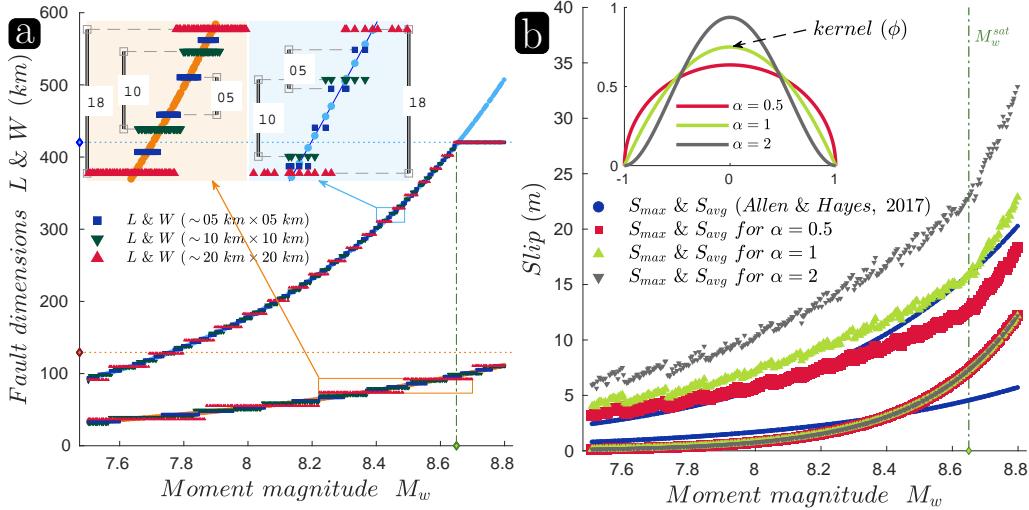


Figure A.1. a) Realizable fault dimensions made up of $\sim 5 \text{ km} \times 5 \text{ km}$, $\sim 10 \text{ km} \times 10 \text{ km}$ and $\sim 20 \text{ km} \times 20 \text{ km}$ segments. The inset plots zoom on to the scaling relation to reveal discontinuities. b) Validation of slip profile by varying steepness α and comparing with maximum S_{max} and average S_{avg} slips for 300 scenarios.

We fix the dimension h_s of an FF segment based on: (i) computational effort required – scales as $O(n_F) \sim O(h_s^{-2})$ and, (ii) fidelity to the scaling relation (Figure A.1a inset) – earthquake dimensions are resolved to $O(h_s)$ (Figure A.1a). An $h_s \sim 5 \text{ km}$ gives 2295 segments for the overall FF dimensions of $L^{max} \sim 420 \text{ km}$ and $W^{max} \sim 129 \text{ km}$. To resolve the slip profile adequately, we require a fault to span a minimum of 4 segments along both the length and width directions. Using the scaling relation for $h_s \sim 5 \text{ km}$, this requirement gives a minimum M_w 6.32 that can be accommodated on the FF. This is sufficient as our region of investigation starts at $M_w^{min} = 7.5$. The scaling relation also limits the maximum M_w that can be accommodated on the FF area of

$L^{max} \times W^{max}$, giving $M_w^{sat} = 8.65$ (Figure 1c). Since our region of investigation is till $M_w 8.8$, for $M_w^{sat} < M_w < 8.8$, we proportionately increase the slip on the maximum dimensions. Now, to generate the slip profile, a positive kernel function ϕ is used (Figure A.1b inset):

$$\phi(x; r, \alpha) = \begin{cases} \frac{\Gamma(2\alpha+2)}{2^{2\alpha+1}\Gamma(\alpha+1)^2} \left(1 - \left|\frac{x}{r}\right|^2\right)^\alpha & |x| \leq r \\ 0 & |x| > r \end{cases} \quad (\text{A.1})$$

where the gamma function Γ enters the normalization constant, length scale r defines where ϕ is non-zero and α adjusts the steepness of ϕ . With ϕ as the core, the bi-lobed kernel Φ is defined as:

$$\Phi(x; r_l, r_r, \alpha) = \begin{cases} \phi(x; r_l, \alpha) & -r_l \leq x \leq 0 \\ \phi(x; r_r, \alpha) & 0 \leq x \leq r_r \end{cases} \quad (\text{A.2})$$

where r_l and r_r are the length scales of the left and right lobes, their values depending on the position of epicenter (X_o, Y_o) with respect to fault length (L) and width (W). The tensor product of two bi-lobed kernels, one along the length and another along the width of the fault yields the surface Φ^\otimes (Figure A.2):

$$\Phi^\otimes(x, y; \mathbf{r}^\otimes, \alpha) = \Phi(x; r_W, r_E, \alpha) \otimes \Phi(y; r_S, r_N, \alpha) \quad (x, y) \in [-r_W, r_E] \times [-r_S, r_N] \quad (\text{A.3})$$

where $[-r_W, r_E] \times [-r_S, r_N]$ denotes the domain of the fault and $\mathbf{r}^\otimes = \{r_W, r_E, r_S, r_N\}$, the distances of western, eastern, southern and northern sides of the fault rectangle from (X_o, Y_o) . A normalization of Φ^\otimes with the required moment magnitude yields the final slip profile S (e.g. Figure 4). We select $\alpha = 1$ by varying α to mirror the maximum slip S_{max} and average slip S_{avg} curves from empirical scaling relations (see Table 2 in [25]) (Figure A.1b).

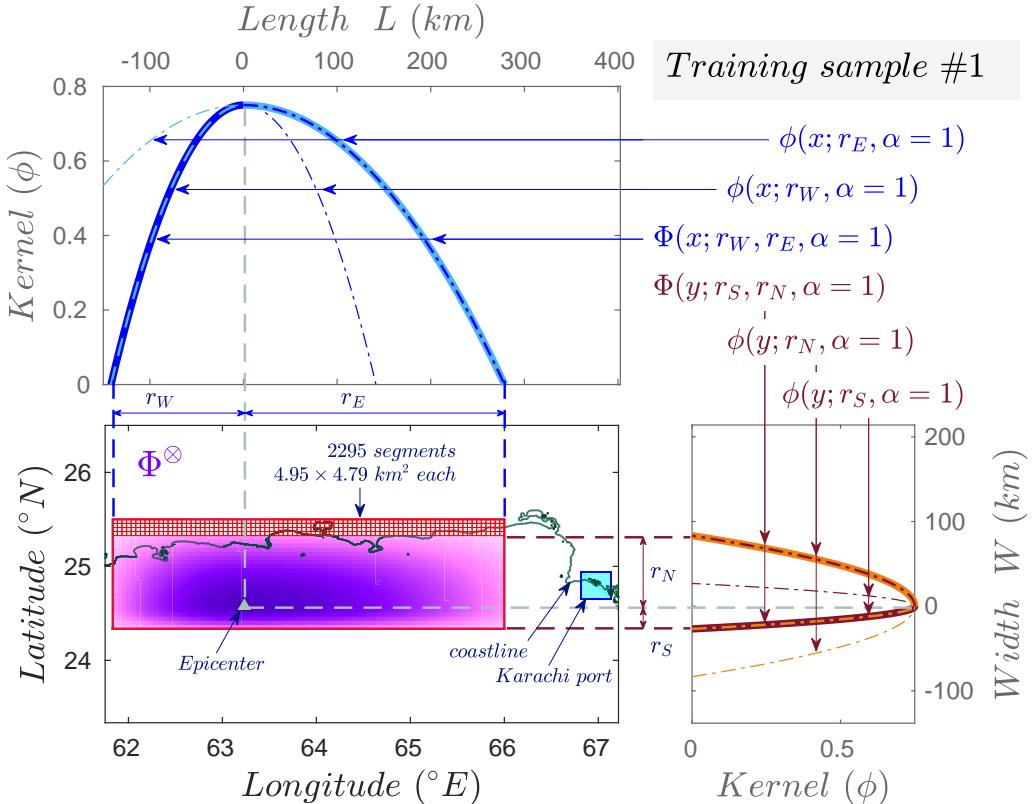


Figure A.2. Generation of slip profile (for sample no. 1) by tensor product of bi-lobed kernels along fault dimensions.

B. Merging Bathymetry Data near Port

Substitution of GEBCO bathymetry with data from other sources gives rise to unrealistic discontinuities in the merged data (Figure B.1e). We ameliorate this by a procedure using cosine-tapered Tukey windows (Figures B.1f-g), smoothly merging GEBCO and hydrographic chart data. The chart data is available in the domain of interest (DOI) $[66.9332, 67.0168]^\circ E \times [24.7666, 24.8334]^\circ N$. The cosine-tapered Tukey window is given by:

$$\theta(x, r_c) = \begin{cases} \frac{1}{2} \left\{ 1 + \cos \left(\frac{2\pi}{r_c} [x - r_c/2] \right) \right\} & 0 \leq x \leq \frac{r_c}{2} \\ 1 & \frac{r_c}{2} \leq x \leq 1 - \frac{r_c}{2} \\ \frac{1}{2} \left\{ 1 + \cos \left(\frac{2\pi}{r_c} [x - 1 + r_c/2] \right) \right\} & 1 - \frac{r_c}{2} \leq x \leq 1 \end{cases} \quad (\text{B.1})$$

where r_c is the cosine-fraction, *i.e.* ratio of length of cosine-taper to the total window length of 1. Shifted and dilated versions of θ are used to create the tensor product:

$$\Theta(x - x_p, y - y_p, d_x, d_y, r_c^x, r_c^y) = \theta\left(\frac{x - x_p}{d_x}, r_c^x\right) \otimes \theta\left(\frac{y - y_p}{d_y}, r_c^y\right) \quad (\text{B.2})$$

where (x_p, y_p) are co-ordinates of the centre of the DOI, $d_x \times d_y$ is its area, and (r_c^x, r_c^y) are the cosine-fractions along the length and width of the DOI. Algorithm 2 and Figure B.1 detail the procedure used for merging the different bathymetries. It is noted that inadequacies in the available bathymetries, and difficulty in ascertaining the datum in each data set have prompted us to assume a common datum. A future step would incorporate different datum within the framework of a more rigorous merging procedure, *e.g.* remove-restore [72].

Algorithm 2 Merging bathymetry data near port

- 1: Up-sample the hydrographic chart data and GEBCO bathymetry in the DOI on a rectangular grid having a resolution of the computational mesh ($\sim 10\text{ m}$) (Figures B.1a and d).
- 2: Integrate the polygonal domains of resolved coastline features into up-sampled bathymetry by filling land areas with a positive constant (2 m) (Figure B.1b).
- 3: Interpolate the SRTM data for land onto the grid. If SRTM data exists on the water area after integration of port features, discard the SRTM data there (Figure B.1c).
- 4: Construct tensor product of cosine-tapered Tukey windows (Θ) and its complement ($1 - \Theta$) with cosine fractions $r_c^x = r_c^y = 10\%$ (Figures B.1f and g).
- 5: Multiply hydrographic chart data integrated with port coastline features and SRTM data (Figure B.1c) with Θ (Figure B.1f) to get windowed merged bathymetry (Figure B.1h). The data at the start of the taper is used for the tapered region.
- 6: Multiply GEBCO data (Figure B.1d) with $1 - \Theta$ (Figure B.1g) to get windowed GEBCO bathymetry (Figure B.1i).
- 7: Add windowed merged bathymetry (Figure B.1h) with windowed GEBCO bathymetry (Figure B.1i) to get the merged bathymetry (Figure B.1j).

C. Non-uniform Unstructured Mesh with Local Refinement

Offshore region – The mesh sizing function $h(b_s)$ is based on the merged bathymetry b_s (Figure C.1a inset). With the dimensions of the finite fault earthquake source ($L \times W$), we assume an approximate source wavelength λ_o ($< \sqrt{L^2 + W^2}$) of the tsunami, and a representative ocean depth of the Makran trench b_o ($\sim 3\text{ km}$), to calculate the time period T_λ of the wave as:

$$T_\lambda = \lambda_o / \sqrt{gb_o} \quad (\text{C.1})$$

Here, λ_o is 60 km , which is $\sim 60\%$ of the diagonal in the smallest fault, *i.e.* of size $\sim 94\text{ km} \times 34\text{ km}$ for a $M_w 7.5$ event (sample no. 300). Assuming the time period to be same everywhere, the

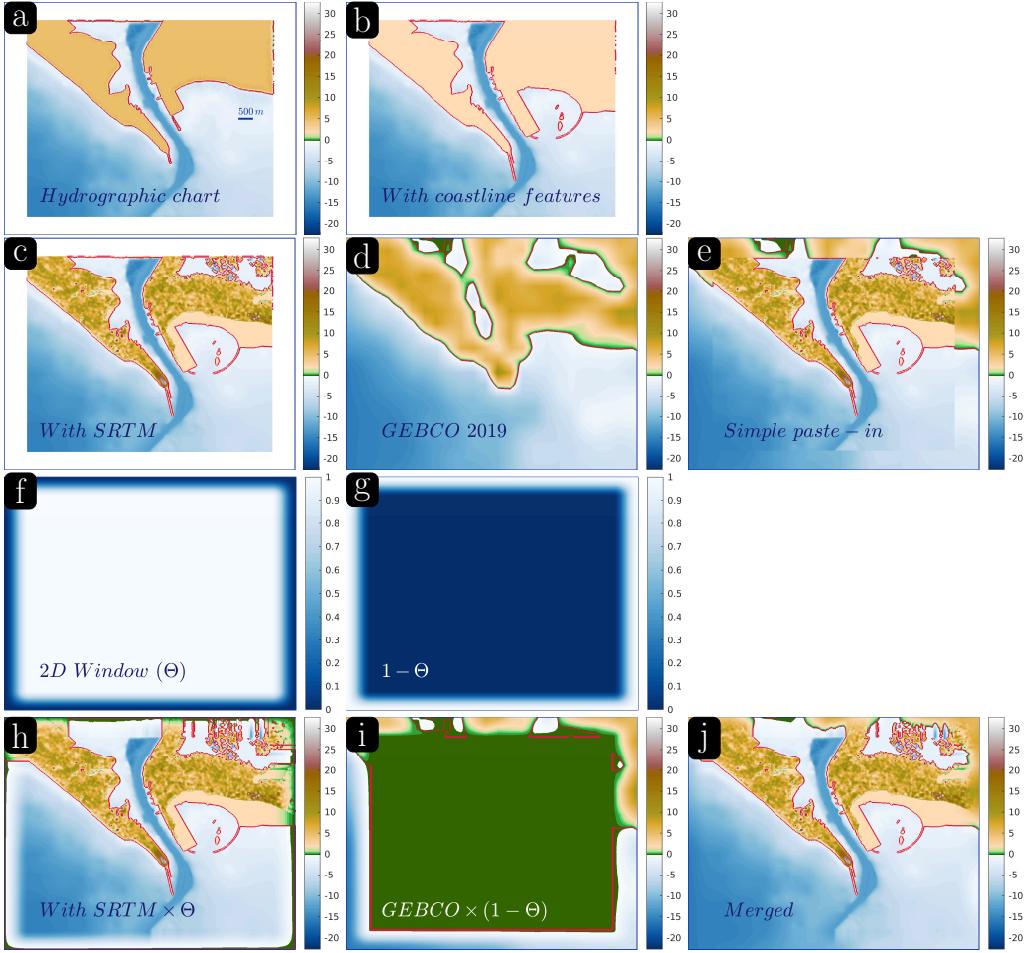


Figure B.1. Merging of bathymetries. a) Digitized hydrographic chart data. b) Coastline altered using Google Earth. c) Supplemented with SRTM data. d) Bathymetry from GEBCO. e) Merged bathymetry, with chart data in (c) pasted into GEBCO data in (d). f) 2D window Θ . g) Complement of Θ , i.e. $1 - \Theta$. h) Chart data in (c) multiplied by Θ . i) GEBCO data in (d) multiplied by $1 - \Theta$. j) Merged bathymetry resulting from addition of windowed data in (h-i).

wavelength λ_n at depth $b_s(\mathbf{x})$ is found as [73]:

$$\lambda_n / \sqrt{b_s(\mathbf{x})} = \lambda_o / \sqrt{b_o} \quad (\text{C.2})$$

This relates the characteristic length of mesh triangle (or mesh size) $h_\lambda(b_s)$ at depth $b_s(\mathbf{x})$ as:

$$h_\lambda(b_s) = (\lambda_o / n_h) \sqrt{b_s(\mathbf{x}) / b_o} \quad (\text{C.3})$$

where $n_h = \lambda_n / h_\lambda(b_s) = 10$ is the number of triangles in one wavelength λ_n . At the coast (i.e. $b_s = 0$), a minimum mesh size h_m (500 m) is specified. The mesh sizing h_λ may be steep, or having a high gradient with respect to the bathymetry b_s (green curve, Figure C.1a). A reduction in gradient is achieved by linearly interpolating the mesh size λ_o / n_h at b_o and the minimum mesh size h_m at the coast, i.e. $b_s = 0$ (red curve, Figure C.1a):

$$h_{\mathcal{I}}(b_s) = b_s(\mathbf{x}) * (\lambda_o / n_h - h_m) / (b_o - 0) + h_m \quad (\text{C.4})$$

The mesh sizing function $h(b_s)$ is then given by the minimum:

$$h(b_s) = \min(h_\lambda(b_s), h_{\mathcal{I}}(b)) \quad (\text{C.5})$$

Onshore region – The mesh sizing function $h(\pi)$ is based on the coast proximity $\pi(\mathbf{x})$ (Figure C.1b inset), which is defined as the minimum distance of point \mathbf{x} from the coastline \mathcal{C} of the merged bathymetry b_s :

$$\pi(\mathbf{x}) = \min_{\mathbf{x}_c \in \mathcal{C}} \|\mathbf{x} - \mathbf{x}_c\|_2 \quad (\text{C.6})$$

The construction of $h(\pi)$ is split into three regions, *viz.* inundation, stretch and blow-up (Figure C.1b). In the inundation region which is defined to extend inland for a distance π_I (2.5 km) from the coast, the mesh size is prescribed as the minimum mesh size h_m (500 m). Thus, the inundation region facilitates smooth transition between the onshore and offshore meshes. Further inland, we require the triangle sizes to explode quickly to the maximum mesh size h_M (25 km). This region is called the blow-up region (from π_S to π_B in Figure C.1b). We introduce the stretch region between the end of the inundation region and the beginning of the blow-up region (*i.e.* from π_I to π_S in Figure C.1b), for a gradual transition of corresponding mesh sizes, *i.e.* from h_m to h_S (10 km). This gradual change is achieved by setting the size ratio ρ , which is the ratio of characteristic lengths of adjacent triangles (or grading gauge [74]) to 1.3. The stretch distance $\pi_S - \pi_I$ is calculated as:

$$\pi_S - \pi_I = h_m + \rho h_m + \rho^2 h_m + \dots + \rho^{n_S} h_m \quad (\text{C.7})$$

Eqn. C.7 is a geometric series that approximates the distance by summing up the sizes of $n_S + 1$ triangles, lined up end-to-end in a straight line, monotonically increasing in size by a factor of ρ [74], starting from h_m to $\rho^{n_S} h_m$. Equating the last term to h_S , solve for integer n_S as:

$$n_S = \lceil \log_{\rho} \left(\frac{h_S}{h_m} \right) \rceil \quad (\text{C.8})$$

where $\lceil \cdot \rceil$ denotes the ceiling function. Similarly, the blow-up distance $\pi_B - \pi_S$ is calculated as:

$$\pi_B - \pi_S = h_S + \rho h_S + \rho^2 h_S + \dots + \rho^{n_B} h_S \quad (\text{C.9})$$

The description of Eqn. C.9 is similar to Eqn. C.7. Equating the last term to h_M , get integer n_B as:

$$n_B = \lceil \log_{\rho} \left(\frac{h_M}{h_S} \right) \rceil \quad (\text{C.10})$$

Note – The mesh sizing functions $h(b_s)$ and $h(\pi)$ are specified to Gmsh on a background rectangular grid that has twice the resolution (~ 210 m) of GEBCO 2019 grid, sufficient for specifying the minimum mesh size h_m (500 m). The number of levels in Figure C.1b are the number of grids needed in the background mesh to specify mesh sizes in the respective region.

Port region – The strategy is similar to that in the stretch region, but the radial distance from the centre (x_p, y_p) of the DOI (or port) is used instead of the coast proximity. The mesh size is fixed at h_m^p (10 m) in the DOI where resolved bathymetry is available. The resolution of background rectangular grid near the port is 10 m $\sim h_m^p$. A smaller size ratio ρ^p of 1.05 ensures a gradual transition of mesh sizes. In increasing radii extending outwards from the DOI, the mesh size increases similar to Eqn. C.7, but iteratively with increasing number of terms in the geometric progression. The iterative procedure is employed to effect a smooth transition of the mesh at the port with existing offshore and onshore meshes (Figure C.2b). For contrast, Figures C.2a show Pasabandar port where local refinement of mesh is absent.

Data Accessibility. The data and codes used have been cited and/or linked in footnotes an first mention.

Authors' Contributions. MH and SG conceptualized the problem. SG and DG conceptualized the employment of large-scale statistical emulation and the inclusion of the effect of sediments. MH digitized the bathymetry for Karachi port. DG designed the problem with inputs and supervision from SG and MH, developed codes, curated data, carried out the simulations with associated validation, analysis and data processing, and created visualizations for main article and ESM. All authors drafted and critically reviewed the manuscript. All authors give final approval for publication and agree to be held accountable for the work performed herein.

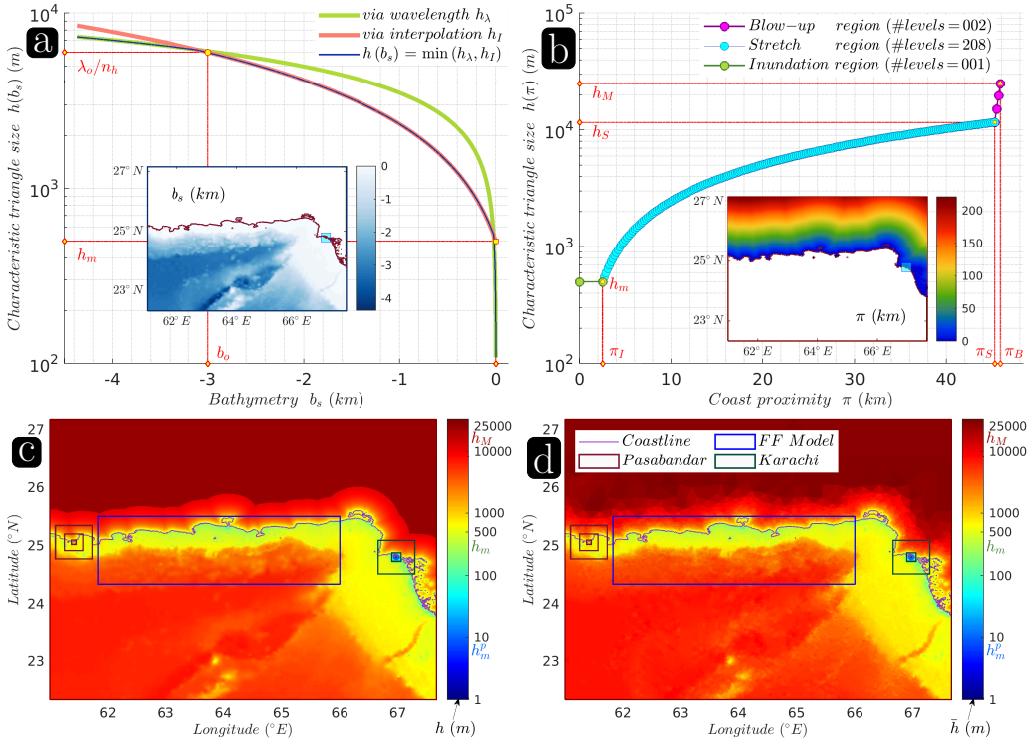


Figure C.1. Localised non-uniform unstructured mesh. a) Mesh sizing for offshore region based on bathymetry b_s (inset). b) Mesh sizing for onshore region based on coast proximity π (inset). c) Mesh sizing function h supplied to Gmsh. d) Mesh sizes \bar{h} in mesh generated by Gmsh using h in (c).

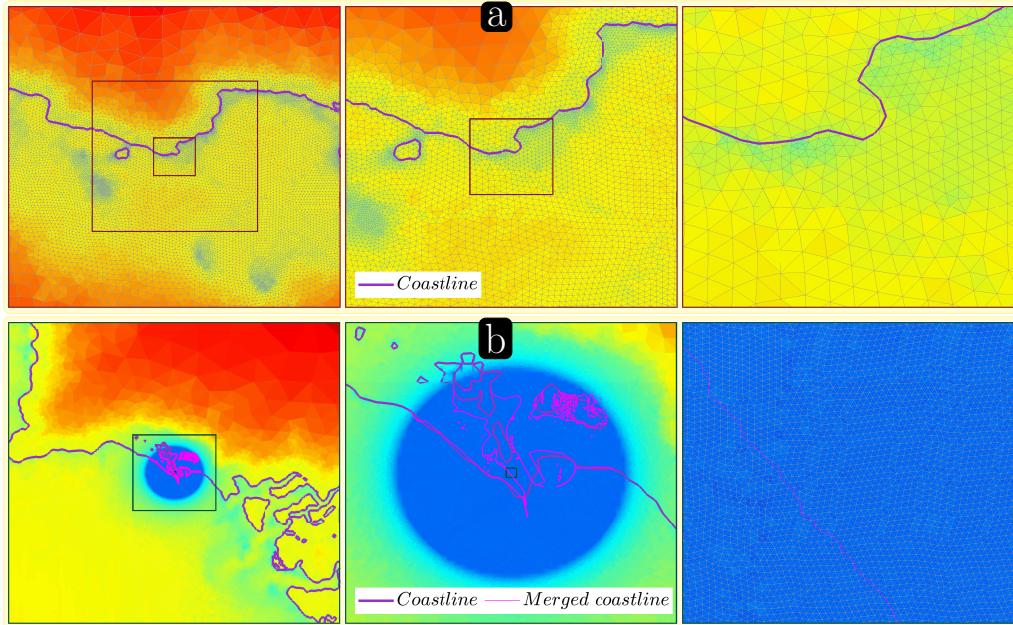


Figure C.2. Localised non-uniform unstructured mesh. a) Mesh without local refinement at Pasabandar shown at scales of 64 km x 64 km (left), 32 km x 32 km (centre) and 8 km x 8 km (right) respectively. b) Locally refined mesh at Karachi port shown at scales of 64 km x 64 km (left), 16 km x 16 km (centre) and 0.5 km x 0.5 km (right) respectively.

Competing Interests. We declare we have no competing interests.

Funding. DG and SG were supported by the Alan Turing Institute under the EPSRC grant [EP/N510129/1]. MH was supported by the Royal Society grant [CHL/R1/180173]. DG was partially funded by the Royal Society-SERB Newton International Fellowship [NF151483]. DG, MH and SG acknowledge support from the NERC grant [NE/P016367/1].

Acknowledgements. This work has been performed using resources provided by the Cambridge Tier-2 system (CSD3 Wilkes2) operated by the University of Cambridge Research Computing Service funded by EPSRC Tier-2 capital grant EP/P020259/1. The authors would like to acknowledge the use of the University of Oxford Advanced Research Computing (ARC) facility (JADE) in carrying out this work (doi.org/10.5281/zenodo.22558). Preparative simulations were performed on the EMERALD High Performance Computing facility provided via the EPSRC funded Centre for Innovation (EP/K000144/1 and EP/K000136/1), owned and operated by the e-Infrastructure South Consortium formed by the universities of Bristol, Oxford, Southampton and UCL in partnership with STFC Rutherford Appleton Laboratory. We thank Eric Daub and Oliver Strickson for active development of MOGP UQ suite, Daniel Giles for improvements to second order FV scheme and boundary conditions in VOLNA-OP2, István Reguly for its installation and running on CSD3, Deyu Ming and Mariya Mamajiwala on truncated G-R distribution and prediction intervals for L-O-O diagnostics, Frédéric Dias on meshing strategies and sediment amplification curve, Theodoros Mathikolonus on emulation, and Simon Day, Kusala Rajendran and C.P. Rajendran on seismicity of MSZ. We thank the referees whose detailed comments were instrumental in enhancing the content and presentation of the article.

References

1. Borrero JC, Lynett PJ, Kalligeris N. 2015 Tsunami currents in ports. *Philosophical Transactions of the Royal Society A: Mathematical, Physical and Engineering Sciences* **373**, 20140372.
2. Okal EA, Fritz HM, Raad PE, Synolakis C, Al-Shibbi Y, Al-Saifi M. 2006 Oman field survey after the December 2004 Indian Ocean tsunami. *Earthquake Spectra* **22**, 203–218.
3. Borrero JC, Goring DG, Greer SD, Power WL. 2015 Far-field tsunami hazard in New Zealand ports. *Pure and Applied Geophysics* **172**, 731–756.
4. Lynett PJ, Borrero JC, Weiss R, Son S, Greer D, Renteria W. 2012 Observations and modeling of tsunami-induced currents in ports and harbors. *Earth and Planetary Science Letters* **327-328**, 68 – 74.
5. Lynett PJ, Borrero J, Son S, Wilson R, Miller K. 2014 Assessment of the tsunami-induced current hazard. *Geophysical Research Letters* **41**, 2048–2055.
6. Lynett PJ et al. 2017 Inter-model analysis of tsunami-induced coastal currents. *Ocean Modelling* **114**, 14 – 32.
7. Park H, Cox DT. 2016 Probabilistic assessment of near-field tsunami hazards: Inundation depth, velocity, momentum flux, arrival time, and duration applied to Seaside, Oregon. *Coastal Engineering* **117**, 79 – 96.
8. Gonzalez FI, LeVeque RJ, Adams LM. 2013 Probabilistic Tsunami Hazard Assessment (PTHA) for Crescent City, CA. Final Report for Phase I. Technical report University of Washington Department of Applied Mathematics.
9. Geist EL, Parsons T. 2006 Probabilistic analysis of tsunami hazards. *Natural Hazards* **37**, 277–314.
10. Annaka T, Satake K, Sakakiyama T, Yanagisawa K, Shuto N. 2007 Logic-tree Approach for Probabilistic Tsunami Hazard Analysis and its Applications to the Japanese Coasts. In Satake K, Okal EA, Borrero JC, editors, *Tsunami and Its Hazards in the Indian and Pacific Oceans* pp. 577–592 Basel. Birkhäuser Basel.
11. Grezio A, Marzocchi W, Sandri L, Gasparini P. 2010 A Bayesian procedure for Probabilistic Tsunami Hazard Assessment. *Natural Hazards* **53**, 159–174.
12. Grezio A, Babeyko A, Baptista MA, Behrens J, Costa A, Davies G, Geist EL, Glimsdal S, González FI, Griffin J, Harbitz CB, LeVeque RJ, Lorito S, Løvholt F, Omira R, Mueller C, Paris R, Parsons T, Polet J, Power W, Selva J, Sørensen MB, Thio HK. 2017 Probabilistic tsunami hazard analysis: Multiple sources and global applications. *Reviews of Geophysics* **55**, 1158–1198.

13. Salmanidou DM, Heidarzadeh M, Guillas S. 2019 Probabilistic landslide-generated tsunamis in the Indus Canyon, NW Indian Ocean, using statistical emulation. *Pure and Applied Geophysics* **176**, 3099–3114.
14. Kotani T, Tozato K, Takase S, Moriguchi S, Terada K, Fukutani Y, Otake Y, Nojima K, Sakuraba M, Choe Y. 2020 Probabilistic tsunami hazard assessment with simulation-based response surfaces. *Coastal Engineering* **160**, 103719.
15. Zhang X, Niu X. 2020 Probabilistic tsunami hazard assessment and its application to southeast coast of Hainan Island from Manila Trench. *Coastal Engineering* **155**, 103596.
16. Lorito S, Selva J, Basili R, Romano F, Tiberti M, Piatanesi A. 2014 Probabilistic hazard for seismically induced tsunamis: accuracy and feasibility of inundation maps. *Geophysical Journal International* **200**, 574–588.
17. Volpe M, Lorito S, Selva J, Tonini R, Romano F, Brizuela B. 2019 From regional to local SPTA: efficient computation of probabilistic tsunami inundation maps addressing near-field sources. *Natural Hazards and Earth System Sciences* **19**, 455–469.
18. Reguly IZ et al.. 2018 The VOLNA-OP2 tsunami code (version 1.5). *Geoscientific Model Development* **11**, 4621–4635.
19. Rajendran CP, Heidarzadeh M, Sanwal J, Karthikeyan A, Rajendran K. 2020 The Orphan Tsunami of 1524 on the Konkan Coast, Western India, and Its Implications. *Pure and Applied Geophysics*.
20. Byrne DE, Sykes LR, Davis DM. 1992 Great thrust earthquakes and aseismic slip along the plate boundary of the Makran Subduction Zone. *Journal of Geophysical Research: Solid Earth* **97**, 449–478.
21. Heidarzadeh M, Pirooz MD, Zaker NH, Yalciner AC, Mokhtari M, Esmaeily A. 2008 Historical tsunami in the Makran Subduction Zone off the southern coasts of Iran and Pakistan and results of numerical modeling. *Ocean Engineering* **35**, 774–786.
22. Heidarzadeh M, Satake K. 2014 Possible sources of the tsunami observed in the northwestern Indian Ocean following the 2013 September 24 Mw 7.7 Pakistan inland earthquake. *Geophysical Journal International* **199**, 752–766.
23. Smith GL, McNeill LC, Wang K, He J, Henstock TJ. 2013 Thermal structure and megathrust seismogenic potential of the Makran subduction zone. *Geophysical Research Letters* **40**, 1528–1533.
24. Blaser L, Krüger F, Ohrnberger M, Scherbaum F. 2010 Scaling relations of earthquake source parameter estimates with special focus on subduction environment. *Bulletin of the Seismological Society of America* **100**, 2914–2926.
25. Allen TI, Hayes GP. 2017 Alternative rupture-scaling relationships for subduction interface and other offshore environments. *Bulletin of the Seismological Society of America* **107**, 1240–1253.
26. Dutykh D, Dias F. 2010 Influence of sedimentary layering on tsunami generation. *Computer Methods in Applied Mechanics and Engineering* **199**, 1268–1275. Multiscale Models and Mathematical Aspects in Solid and Fluid Mechanics.
27. Frohling E, Szeliga W. 2016 GPS constraints on interplate locking within the Makran subduction zone. *Geophysical Journal International* **205**, 67–76.
28. Penney C, Tavakoli F, Saadat A, Nankali HR, Sedighi M, Khorrami F, Sobouti F, Rafi Z, Copley A, Jackson J, Priestley K. 2017 Megathrust and accretionary wedge properties and behaviour in the Makran subduction zone. *Geophysical Journal International* **209**, 1800–1830.
29. Page WD, Alt JN, Cluff LS, Plafker G. 1979 Evidence for the recurrence of large-magnitude earthquakes along the Makran coast of Iran and Pakistan. *Tectonophysics* **52**, 533–547. Recent Crustal Movements.
30. Rajendran CP, Rajendran K, Shah-hosseini M, Beni AN, Nautiyal CM, Andrews R. 2013 The hazard potential of the western segment of the Makran subduction zone, northern Arabian Sea. *Natural Hazards* **65**, 219–239.
31. Cosentino P, Ficarra V, Luzio D. 1977 Truncated exponential frequency-magnitude relationship in earthquake statistics. *Bulletin of the Seismological Society of America* **67**, 1615–1623.
32. Danciu L, Sesetyan K, Demircioglu M et al.. 2018 The 2014 Earthquake Model of the Middle East: seismogenic sources. *Bulletin of Earthquake Engineering* **16**, 3465–3496.
33. Okada Y. 1985 Surface deformation due to shear and tensile faults in a half-space. *Bulletin of the Seismological Society of America* **75**, 1135–1154.
34. Hayes GP, Moore GL, Portner DE, Hearne M, Flamme H, Furtney M, Smoczyk GM. 2018 Slab2, a comprehensive subduction zone geometry model. *Science* **362**, 58–61.

35. Hayes GP. 2018 Slab2 - A Comprehensive Subduction Zone Geometry Model: U.S. Geological Survey data release. .
36. Battaglia M, Cervelli P, Murra-Muraleda J. 2012 Modeling crustal deformation_A catalog of deformation models and modeling approaches. *US Geological Survey Techniques and Methods* pp. 37–85.
37. Battaglia M, Cervelli PF, Murray JR. 2013 dMODELS: A MATLAB software package for modeling crustal deformation near active faults and volcanic centers. *Journal of Volcanology and Geothermal Research* **254**, 1 – 4.
38. Li L, Qiu Q, Huang Z. 2012 Numerical modeling of the morphological change in Lhok Nga, west Banda Aceh, during the 2004 Indian Ocean tsunami: understanding tsunami deposits using a forward modeling method. *Natural Hazards* **64**, 1549–1574.
39. Jaffe B, Goto K, Sugawara D, Gelfenbaum G, Selle SL. 2016 Uncertainty in Tsunami Sediment Transport Modeling. *Journal of Disaster Research* **11**, 647–661.
40. Straume EO et al.. 2019 GlobSed: Updated total sediment thickness in the world's oceans. *Geochemistry, Geophysics, Geosystems* **20**, 1756–1772.
41. Macías J, Castro MJ, Ortega S, Escalante C, González-Vida JM. 2017 Performance Benchmarking of Tsunami-HySEA Model for NTHMP's Inundation Mapping Activities. *Pure and Applied Geophysics* **174**, 3147–3183.
42. Yuan Y, Shi F, Kirby JT, Yu F. 2020 FUNWAVE-GPU: Multiple-GPU Acceleration of a Boussinesq-Type Wave Model. *Journal of Advances in Modeling Earth Systems* **12**, e2019MS001957.
43. Rashidi A, Shomali ZH, Dutykh D, Keshavarz Farajkhah N. 2020 Tsunami hazard assessment in the Makran subduction zone. *Natural Hazards* **100**, 861–875.
44. Hasan H, Lodhi H, LeVeque R, Lodhi S, Ahmed S. 2017 Assessing tsunami risk to Karachi Port through simulation of currents that were reportedly produced there by the 1945 Makran tsunami. In *Proceedings of the 16th world conference on earthquake engineering, Santiago, Chile*.
45. Heidarzadeh M, Kijko A. 2011 A probabilistic tsunami hazard assessment for the Makran subduction zone at the northwestern Indian Ocean. *Natural Hazards* **56**, 577–593.
46. Dias F, Dutykh D, ÓBrien L, Renzi E, Stefanakis T. 2014 On the modelling of tsunami generation and tsunami inundation. *Procedia IUTAM* **10**, 338 – 355. Mechanics for the World: Proceedings of the 23rd International Congress of Theoretical and Applied Mechanics, ICTAM2012.
47. Dutykh D, Poncet R, Dias F. 2011 The VOLNA code for the numerical modeling of tsunami waves: Generation, propagation and inundation. *European Journal of Mechanics - B/Fluids* **30**, 598 – 615. Special Issue: Nearshore Hydrodynamics.
48. Giles D, Kashdan E, Salmanidou DM, Guillas S, Dias F. 2020 Performance analysis of Volna-OP2 – massively parallel code for tsunami modelling. *Computers & Fluids* **209**, 104649.
49. GEBCO Bathymetric Compilation Group 2019. 2019 The GEBCO_2019 Grid - a continuous terrain model of the global oceans and land. .
50. NASA JPL. 2013 NASA Shuttle Radar Topography Mission Global 1 arc second [Data set]. .
51. Geuzaine C, Remacle JF. 2009 Gmsh: A 3-D finite element mesh generator with built-in pre- and post-processing facilities. *International Journal for Numerical Methods in Engineering* **79**, 1309–1331.
52. Beck J, Guillas S. 2016 Sequential design with Mutual Information for Computer Experiments (MICE): Emulation of a tsunami model. *SIAM/ASA Journal on Uncertainty Quantification* **4**, 739–766.
53. Sarri A, Guillas S, Dias F. 2012 Statistical emulation of a tsunami model for sensitivity analysis and uncertainty quantification. *Natural Hazards and Earth System Sciences* **12**, 2003–2018.
54. Salmanidou DM, Guillas S, Georgioupolou A, Dias F. 2017 Statistical emulation of landslide-induced tsunamis at the Rockall Bank, NE Atlantic. *Proceedings of the Royal Society A: Mathematical, Physical and Engineering Sciences* **473**, 20170026.
55. Guillas S, Sarri A, Day SJ, Liu X, Dias F et al.. 2018 Functional emulation of high resolution tsunami modelling over Cascadia. *The Annals of Applied Statistics* **12**, 2023–2053.
56. Rasmussen CE, Williams CK. 2005 Covariance functions. In *Gaussian Processes for Machine Learning*. The MIT Press.
57. Arcos MEM, LeVeque RJ. 2015 Validating velocities in the GeoClaw tsunami model using observations near Hawaii from the 2011 Tohoku tsunami. *Pure and Applied Geophysics* **172**, 849–867.

58. Dengler L, Uslu B. 2011 Effects of harbor modification on Crescent City, California's tsunami vulnerability. *Pure and Applied Geophysics* **168**, 1175–1185.
59. Chock GYK. 2016 Design for tsunami loads and effects in the ASCE 7-16 standard. *Journal of Structural Engineering* **142**, 04016093.
60. Muhari A, Charvet I, Tsuyoshi F, Suppasri A, Imamura F. 2015 Assessment of tsunami hazards in ports and their impact on marine vessels derived from tsunami models and the observed damage data. *Natural Hazards* **78**, 1309–1328.
61. Davies G et al.. 2018 A global probabilistic tsunami hazard assessment from earthquake sources. *Geological Society, London, Special Publications* **456**, 219–244.
62. Hoechner A, Babeyko AY, Zamora N. 2016 Probabilistic tsunami hazard assessment for the Makran region with focus on maximum magnitude assumption. *Natural Hazards and Earth System Sciences* **16**, 1339–1350.
63. Liu X, Guillas S. 2017 Dimension reduction for Gaussian process emulation: An application to the influence of bathymetry on tsunami heights. *SIAM/ASA Journal on Uncertainty Quantification* **5**, 787–812.
64. Heidarzadeh M, Satake K. 2017 A Combined Earthquake–Landslide Source Model for the Tsunami from the 27 November 1945 M_w 8.1 Makran Earthquake. *Bulletin of the Seismological Society of America* **107**, 1033–1040.
65. Moernaut J, Daele MV, Fontijn K, Heirman K, Kempf P, Pino M, Valdebenito G, Urrutia R, Strasser M, Batist MD. 2018 Larger earthquakes recur more periodically: New insights in the megathrust earthquake cycle from lacustrine turbidite records in south-central Chile. *Earth and Planetary Science Letters* **481**, 9–19.
66. Scala A, Lorito S, Romano F, Murphy S, Selva J, Basili R, Babeyko A, Herrero A, Hoechner A, Løvholt F, Maesano FE, Perfetti P, Tiberti MM, Tonini R, Volpe M, Davies G, Festa G, Power W, Piatanesi A, Cirella A. 2019 Effect of shallow slip amplification uncertainty on probabilistic tsunami hazard analysis in subduction zones: Use of long-term balanced stochastic slip models. *Pure and Applied Geophysics*.
67. Ayca A, Lynett PJ. 2016 Effect of tides and source location on nearshore tsunami-induced currents. *Journal of Geophysical Research: Oceans* **121**, 8807–8820.
68. Lynett PJ. 2016 Precise prediction of coastal and overland flow dynamics: A grand challenge or a fool's errand. *Journal of Disaster Research* **11**, 615–623.
69. Mathikolonis T, Roeber V, Guillas S. 2019 Computationally efficient surrogate-based optimization of coastal storm waves heights and run-ups. *arXiv preprint arXiv:1910.01932*.
70. Ming D, Guillas S. 2019 Integrated emulators for systems of computer models. *arXiv preprint arXiv:1912.09468*.
71. Ulrich T, Vater, S. Madden EH, Behrens J, van Dinther Y, van Zelst I, Fielding EJ, Liang C, Gabriel AA. 2019 Coupled, physics-based modeling reveals earthquake displacements are critical to the 2018 Palu, Sulawesi Tsunami. *Pure and Applied Geophysics* **176**, 4069–4109.
72. Hell B, Jakobsson M. 2011 Gridding heterogeneous bathymetric data sets with stacked continuous curvature splines in tension. *Marine Geophysical Research* **32**, 493–501.
73. van Scheltinga ADT, Myers PG, Pietrzak JD. 2012 Mesh generation in archipelagos. *Ocean Dynamics* **62**, 1217–1228.
74. Legrand S, Deleersnijder E, Hanert E, Legat V, Wolanski E. 2006 High-resolution, unstructured meshes for hydrodynamic models of the Great Barrier Reef, Australia. *Estuarine, Coastal and Shelf Science* **68**, 36 – 46.