

A hybrid method for linearized wave radiation and diffraction problem by a three dimensional floating structure in a polynya

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Abstract

A hybrid method is developed to solve the interaction problem of wave with a three dimensional floating structure in a polynya. The linearized velocity potential theory is used for fluid flow, and the thin elastic plate model is adopted for the infinitely extended ice sheet. Because of sudden change of the upper boundary of the computational domain, namely from the ice sheet to the free surface, the domain is divided into two sub-domains, one below free surface and the other below the ice sheet. The solution method is divided into three components. The first component is the integral equation over the structure surface and the interface of the two sub-domains. In the second component, the velocity potential is expanded into a series of eigenfunctions in the vertical directions, which avoids the numerical difficulty in calculation of the fifth derivatives. This is coupled with a series of integral equations along the edge of the ice sheet. In the third component of the method, two orthogonal inner products are used to impose the continuity conditions of the velocity and pressure on the interface, as well as the boundary conditions on the ice edge. The developed method is verified through comparison with the analytical solution for a circular cylinder. Case study is then made for a FPSO in a polynya with different shapes and floating positions. The hydrodynamic coefficients, wave exciting force and wave elevation in polynya are provided and analyzed.

Keywords: hybrid method; polynya; flexural-gravity wave; free surface wave; hydrodynamic load

1. Introduction

Accurate prediction of wave-induced loads is very important in both the design and operation of offshore structures. Motivated by the new potentials in natural resources, there is major interest in building structures for the far north Arctic regions, such as drilling rigs, oil-gas production facilities [1]. This has led to some new challenges to the hydrodynamics, as the platform may

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1 operate in different icy water conditions, e.g. near large icebergs or ice blocks, floating in a
2 polynya surrounded by large ice sheet. The last case will be considered in this work and solved by
3 an efficient hybrid method based on the velocity potential theory.

4 There has been extensive research over the many decades on the free surface wave interactions
5 with a structure in open sea, and various numerical methods have been developed (e.g. [2-4]).
6 These methods are nowadays well established and their applications have significantly advanced
7 our understanding of water wave interactions with structures. Among them is the boundary
8 element method which transforms the governing Laplace equation for the velocity potential into
9 the boundary integral equation over the boundary, with the help of a Green function. In general,
10 the integral equation is over the entire boundary. For the linearized problem, however, the Green
11 function satisfying all the boundary conditions, apart from that on the structure surface can be
12 derived. In such a case, the integral equation involves only the body surface and the need to
13 discretize the free surface is removed. The method has then been widely used in the free surface
14 problem (e.g. [5-7]). In [7], the integral was over an vertical circular cylindrical surface and
15 performed analytically, which was then used as a radiation condition for the inner domain with
16 Rankine source as the Green function.

17 For the problem considered in the present paper, a body floating on the free surface of a polynya
18 surrounded by ice sheet, it is not straight forward to derive the Green function satisfying both the
19 free surface boundary conditions in the polynya and the ice sheet boundary conditions. An
20 alternative may be to use the so called Rankine source method. In such a case, both the ice sheet
21 and free surface have to be discretized, together with the body surface. It will increase the number
22 of panels drastically. More importantly, there are higher derivatives, up to fifth one, on the ice
23 sheet (see [8-10] for example). To deal with higher derivatives in the integral equation is always
24 difficult because of the singular behaviour of the Green function. In fact, even for the second
25 derivative, some special method is needed (e.g. [11, 12]). Thus for the two dimensional problem
26 (2D), a hybrid method was introduced [13]. In the polynya, the boundary element method was
27 used. In the region below the ice sheet, the potential is expanded in terms of eigenfuctions in the
28 vertical direction. Since it is a 2D problem, the Laplace equation can be replaced by a series of
29 second order linear ordinary differential equations, each of which can be solved analytically.
30 Because of the analytical expression, the higher derivatives can be obtained directly through
31 differentiating the known functions. The method was verified with analytical solution and found to
32 be very accurate and efficient, and was then used for a 2D floating body of arbitrary shape. Here
33 the hybrid method is extended to the 3D problem. A major difference is that in the horizontal
34 direction the governing 3D Laplace equation does not reduce to ordinary differential equations.

1 Instead it becomes a series of 2D Helmholtz equations, each of which has to be solved through
2 integral equations (e.g. [14]).

3 It is also important to ensure the continuity of velocity and pressure at the interface of the
4 polynya and the region below the ice sheet. As expansion is already used in the vertical direction,
5 an inner product of orthogonality can be used to enforce the required continuity [15]. Thus the
6 computational procedure for the problem has three components: (1) a 3D integral equation in the
7 polynya, (2) an infinite number of 2D integral equations in the region below the ice sheet, and (3)
8 inner product on the interface to ensure continuity. This forms the major novelty of the present
9 paper.

10 Apart from the development in terms of the computational technique, the present work also
11 attempts to use the developed method to investigate the mechanism of the wave/ice sheet/body
12 interaction. There has been extensive work on wave/ice interaction in the context of geophysics.
13 Much of them are focused on the 2D simplified problems, for example wave propagating from
14 open sea to the semi-infinite shore fast ice (e.g. [16-19]), across a polynya or an ice floe (e.g.
15 [20-22]), in an infinite ice sheet but with step changes of thickness (e.g. [23]), and through one or
16 multiple cracks (e.g. [24-26]). The methods used are generally the matched eigenfunction
17 expansion, boundary integral equation, Wiener-Hopf technique, or the Residue Calculus approach.
18 A recent work conducted by Mattsson, Dunham and Werpers [27] also solved the similar 2D
19 problem through a high order finite difference method, but with the focus mainly on the acoustic
20 waves. For an ice sheet with a complicated edge shape, the 3D effects must be considered directly
21 through the 3D model, for example wave scatterings by a circular ice floe (e.g. [28, 29]) or an ice
22 floe with arbitrary geometry (e.g. [30]), and by an arbitrary smooth ice polynya (e.g. [31]). As
23 mentioned above, there has also been extensive work on wave/body interaction, such as the linear
24 and higher order wave loads on ships and offshore structures in open sea (e.g. [32-34]), in a water
25 channel (e.g. [6, 35]), or in a harbor (e.g. [36, 37]). In contrast, there have been much fewer works
26 on the wave/ice sheet/body interaction. Typical work includes those 2D problems of a body
27 submerged below an ice floe or floating in a polynya (e.g. [13, 38-40]), and below an ice sheet
28 with one or multiple cracks (e.g. [41-43]). However, for 3D problems, the work has been mainly
29 focusing on structures with simple geometries, for example bottom-mounted vertical circular
30 cylinders in an infinitely extended ice sheet (e.g. [44, 45]) or in a circular polynya (e.g. [46]). Thus
31 the main goal of the present work is to develop an accurate numerical solution approach for a 3D
32 structure with a complicated shape, and undertake in depth study for the behaviour of a practical
33 structure in the polynya surrounded by ice sheet.

34 The paper is organized as follows. The governing equation of the problem and corresponding

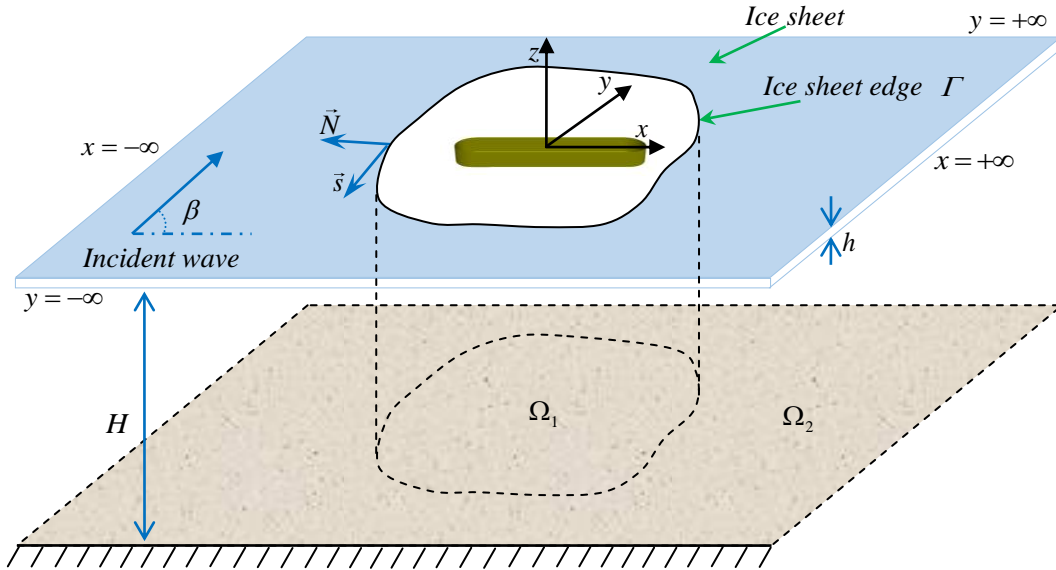
1 boundary conditions are described in section 2. Formulations for the velocity potential in interior
 2 sub-domain or polynya are established in section 3.1, while those in exterior sub-domain or
 3 infinite ice covered region are given in section 3.2. In section 3.3., the solution from matching
 4 procedure on the control surface are shown. Equations for hydrodynamic loads are given in
 5 section 3.4, and numerical discretization scheme is provided in section 3.5. In section 4,
 6 verifications of the proposed method are first carried out, and then the code WISPICE is applied to
 7 a FPSO in a polynya with different shapes and floating positions. Finally, conclusions are drawn in
 8 section 5.

9 2. Mathematical Model

10 The problem of flexural-gravity wave interaction with a body floating on a polynya of arbitrary
 11 shape is sketched in Fig. 1. The polynya is confined by an infinitely extended ice sheet, and the
 12 water depth of finite value H is assumed to be constant. To describe the problem, a Cartesian
 13 coordinate system $O-xyz$ is defined, with $O-xy$ plane being the undisturbed mean free water
 14 surface, and z axis pointing vertically upwards. The oscillation of the body is excited by a
 15 flexural-gravity incident wave, which propagates underneath the ice sheet from infinity from an
 16 angle β with the positive x axis. When the floating body is at its equilibrium position, the z
 17 axis passes through the centre of mass of the body, as shown in the figure. The edge of polynya
 18 can be described parametrically by

$$19 \Gamma = (x(s), y(s)) \quad (-\gamma < s < +\gamma), \quad (1)$$

20 where 2γ is the total arc length of the edge, and s is the curvilinear coordinate along the edge.



21
 22 Figure 1. Coordinate system and sketch of the problem.

23 Based on the assumption that the fluid is inviscid, incompressible and homogeneous, and its

1 motion irrotational, the velocity potential Φ can be introduced to describe the fluid flow. When
 2 the amplitude of wave motion is small compared to its length and the dimension of body, the
 3 linearization of boundary conditions can be further adopted. For sinusoidal motion in time with
 4 radian frequency ω , we may write the total velocity potential as

$$5 \quad \Phi(x, y, z, t) = \text{Re}[\eta_0 \phi_0(x, y, z)e^{i\omega t} + \sum_{i=1}^6 i\omega \eta_i \phi_i(x, y, z)e^{i\omega t}], \quad (2)$$

6 where $\phi_0 = \phi_i + \phi_D$ is the scattering potential with ϕ_i as the incident potential and ϕ_D as the
 7 diffracted potential, η_0 is the amplitude of the incident wave; ϕ_i is the radiation potential due to
 8 the i -th mode of body oscillation in six degrees of freedom with complex amplitude η_i , i.e. η_i
 9 ($i=1,2,3$) are the translational modes along x , y and z directions respectively, while η_i
 10 ($i=4,5,6$) are the corresponding rotational modes. The conservation of mass requires that the
 11 velocity potential ϕ_i ($i=0, \dots, 6$) should satisfy the following Laplace equation

$$12 \quad \nabla^2 \phi_i + \frac{\partial^2 \phi_i}{\partial z^2} = 0, \quad (3)$$

13 throughout the fluid, where

$$14 \quad \nabla^2 = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2}, \quad (4)$$

15 is the Laplacian in horizontal plane. In polynya, the combination of linearized dynamic and
 16 kinematic free surface boundary conditions provides

$$17 \quad -\omega^2 \phi_i + g \frac{\partial \phi_i}{\partial z} = 0 \quad (\text{in } \Omega_1 \text{ with } z=0), \quad (5)$$

18 where g is the acceleration due to gravity. Following Squire [10] and others, the ice sheet can be
 19 modelled as a thin elastic plate with its properties, Young's modulus E , Poisson's ratio ν ,
 20 density ρ_i and thickness h , being assumed to be constant and its draught effect being ignored.
 21 By assuming that there is no gap between ice sheet and water upper surface, the boundary
 22 condition on their interface can be written as

$$23 \quad (L\nabla^4 + \rho_w g - m\omega^2) \frac{\partial \phi_i}{\partial z} - \rho_w \omega^2 \phi_i = 0 \quad (\text{in } \Omega_2 \text{ with } z=0), \quad (6)$$

24 where $L = Eh^3 / [12(1-\nu^2)]$ and $m = \rho_i h$ are respectively the effective flexural rigidity and
 25 mass per unit area of the ice sheet, ρ_w is the density of water. On the boundary of polynya, the
 26 edge of ice sheet is assumed to be free to move, i.e. zero bending moment and shear force
 27 conditions should be satisfied, or [47]

$$28 \quad \mathcal{B}\left(\frac{\partial \phi_i}{\partial z}\right) = 0 \quad \text{and} \quad \mathcal{S}\left(\frac{\partial \phi_i}{\partial z}\right) = 0 \quad ((x, y) \in \Gamma, z=0), \quad (7)$$

29 for $i=0, \dots, 6$, where the operator \mathcal{B} and \mathcal{S} are respectively defined as

1
$$\mathcal{B} = \nabla^2 - \nu_0(\sin^2 \Theta \frac{\partial^2}{\partial x^2} + \cos^2 \Theta \frac{\partial^2}{\partial y^2} - \sin 2\Theta \frac{\partial^2}{\partial x \partial y}), \quad (8)$$

2
$$\mathcal{S} = \frac{\partial}{\partial N} \nabla^2 + \nu_0 \frac{\partial}{\partial s} [\cos 2\Theta \frac{\partial^2}{\partial x \partial y} + \frac{\sin 2\Theta}{2} (\frac{\partial^2}{\partial y^2} - \frac{\partial^2}{\partial x^2})], \quad (9)$$

3 with $\nu_0 = 1 - \nu$, and \vec{N} and \vec{s} are the unit vectors along the normal and tangential directions
 4 respectively, as shown in Fig. 1. Here, $\Theta(s)$ is the angle between positive \vec{N} and x axis, and
 5 thus $\vec{N} = (\cos \Theta, \sin \Theta)$ and $\vec{s} = (-\sin \Theta, \cos \Theta)$. It should be noted that in (6) and (7), the
 6 following kinematic boundary condition has been used

7
$$\frac{\partial W}{\partial t} = \frac{\partial \Phi}{\partial z} \quad (\text{in } \Omega_2 \text{ with } z = 0), \quad (10)$$

8 where W is the deflection of the ice sheet and can be written as

9
$$W = \text{Re}(\eta_0 w_0 e^{i\omega t} + \sum_{i=1}^6 i \omega \eta_i w_i e^{i\omega t}). \quad (11)$$

10 Here, w_0 is the component due to the scattering potential, and w_i is that due to the radiation
 11 potential in i -th mode. The impermeable condition on the mean wetted body surface S_B can be
 12 written as

13
$$\frac{\partial \phi_i}{\partial n} = n_i \quad \text{and} \quad \frac{\partial \phi_D}{\partial n} = -\frac{\partial \phi_I}{\partial n} \quad (i = 1, \dots, 6), \quad (12)$$

14 where $(n_1, n_2, n_3) = \vec{n}$ are the components related to the translational modes, with \vec{n} as the unit
 15 normal vector pointing into body, $(n_4, n_5, n_6) = (\vec{r} - \vec{r}_0) \times \vec{n}$ are those related to the rotational
 16 modes, with \vec{r}_0 as the position of the rotational centre. On the flat seabed, we have

17
$$\frac{\partial \phi_i}{\partial z} = 0 \quad (z = -H), \quad (13)$$

18 for $i = 0, \dots, 6$. At infinity the radiation condition should be imposed, which requires that the
 19 radiated and diffracted waves should propagate outwards, or

20
$$\lim_{r_h \rightarrow \infty} \sqrt{r_h} \left(\frac{\partial \phi_i}{\partial r_h} + i \kappa_0 \phi_i \right) = 0 \quad \text{and} \quad \lim_{r_h \rightarrow \infty} \sqrt{r_h} \left(\frac{\partial \phi_D}{\partial r_h} + i \kappa_0 \phi_D \right) = 0, \quad (14)$$

21 for $i = 1, \dots, 6$, where $r_h^2 = x^2 + y^2$, κ_0 is the flexural-gravity wave number and $i = \sqrt{-1}$. It may
 22 be noticed that κ_0 is the purely positive real root of the dispersion equation $K_2(\omega, \kappa_0) = 0$ for
 23 flexural-gravity wave in the ice sheet, with

24
$$K_2(\omega, k) \equiv (Lk^4 + \rho_w g - m\omega^2)k \tanh(kH) - \rho_w \omega^2. \quad (15)$$

25 Here, we may notice that the adopted linear velocity potential theory for fluid flow and the thin
 26 elastic plate model for ice sheet flexural motion have been verified extensively, e.g. as reviewed
 27 by Squire [10].

28 **3. Solution Procedures**

1 From the above section, it can be seen that the boundary condition for the velocity potential ϕ_i
2 on the upper surface is non uniform, i.e. in polynya the free surface boundary condition (5) should
3 be enforced, while out of polynya the ice sheet boundary condition (6) should be satisfied. To
4 solve such a problem, we shall use the hybrid method, i.e. divide the total fluid domain into two
5 sub-domains Ω_1 and Ω_2 . In the former, the free surface Green function $G^{(1)}(p, q)$ will be used
6 to construct the boundary integral equation, while in the latter, expansion in the vertical direction
7 will be adopted. On the interface between Ω_1 and Ω_2 , the continuity of pressure and normal
8 velocity conditions will be enforced, and the free bending moment and shear force conditions will
9 be imposed at the ice sheet edge.

10 3.1. Boundary integral equation in polynya sub-domain Ω_1

11 In the polynya region with a free surface, we may use the following Green function [48]

$$12 \quad G^{(1)}(p, q) = \frac{1}{r_1} + \frac{1}{r_2} + 2 \int_0^{+\infty} e^{-kH} \frac{gk + \omega^2}{K_1(\omega, k)} \frac{\cosh[k(\zeta + H)]}{\cosh(kH)} \cosh[k(z + H)] J_0(kR) dk, \quad (16)$$

13 where the integral route from 0 to $+\infty$ should pass over the pole at $k = k_0$ with $K_1(\omega, k_0) = 0$.

14 Here,

$$15 \quad K_1(\omega, k) \equiv gk \tanh(kH) - \omega^2, \quad (17)$$

16 is the dispersion equation for free surface gravity wave, r_1 is the distance between p and q ,
17 r_2 is the distance between p and the mirror image of q about the flat seabed, $J_0(kR)$ is the
18 zero order Bessel function of first kind [49], with R as the horizontal distance between p and
19 q . We may note that (16) can be also written into the series form as [48]

$$20 \quad G^{(1)}(p, q) = -4\pi i \sum_{m=0}^{\infty} D_m \cosh^2(k_m H) Z_m(z) Z_m(\zeta) H_0^{(2)}(k_m R), \quad (18)$$

21 where $H_0^{(2)}(k_m R)$ is the Hankel function of second kind [49],

$$22 \quad D_m = \frac{k_m}{2k_m H + \sinh(2k_m H)}, \quad (19)$$

$$23 \quad Z_m(z) = \frac{\cosh[k_m(z + H)]}{\cosh(k_m H)}, \quad (20)$$

24 with k_m as the root of the dispersion equation in (17). It should be noticed that k_0 is the purely
25 positive real root, and k_m ($m = 1, \dots, \infty$) are an infinite number of purely negative imaginary
26 roots. It may be noticed that the series of $Z_m(z)$ is orthogonal [2] and a velocity potential can be
27 expanded into a series of these functions in the vertical direction.

28 Applying Green's identity to $G^{(1)}$ and ϕ_i , we have

$$29 \quad \alpha^{(1)}(p) \phi_i(p) = \int_S [G^{(1)}(p, q) \frac{\partial \phi_i(q)}{\partial n} - \frac{\partial G^{(1)}(p, q)}{\partial n} \phi_i(q)] ds \quad (i = 0, \dots, 6), \quad (21)$$

1 where both the normal derivative and integration are carried out with respect to the source point
2 q , $\alpha^{(1)}(p)$ is the solid angle at point p , and S is comprised of all the boundaries of the
3 sub-domain Ω_1 . It should be noticed that inclusion or removal of the imaginary part of $G^{(1)}$ in
4 (16) would not affect (21). As the imaginary part is nonsingular, its integration with ϕ_i through
5 Green's identity over the closed boundary S_1 of Ω_1 is zero. Invoking the boundary conditions
6 satisfied by $G^{(1)}$ and ϕ_i , we have that only the integrals over the mean wetted body surface S_B
7 and the interface S_C are nonzero. Then the integral equation (21) can be further written as

$$8 \quad \alpha^{(1)}(p)\phi_i(p) = \int_{S_B+S_C} [G^{(1)}(p,q) \frac{\partial \phi_i(q)}{\partial n} - \frac{\partial G^{(1)}(p,q)}{\partial n} \phi_i(q)] ds. \quad (22)$$

9 It might be noticed that when S_C is removed from (22) there will be irregular frequencies for the
10 Neumann problem exterior the body, which correspond to the eigensolutions of the interior
11 Dirichlet problem. However, the irregular frequencies without S_C will not be irregular
12 frequencies here. On the interface S_C , which is vertically extended from the ice edge to the
13 seabed, we may expand ϕ_i and its normal derivative $\partial \phi_i / \partial n$ into a series of orthogonal
14 functions $Z_m(z)$ in (20) or

$$15 \quad \phi_i(p) = \sum_{m=0}^{\infty} \phi_{i,m}(x,y) Z_m(z), \quad \frac{\partial \phi_i(p)}{\partial n} = \sum_{m=0}^{\infty} \frac{\partial \phi_{i,m}(x,y)}{\partial n} Z_m(z). \quad (23)$$

16 Here, it may be noticed that since S_C is a vertical surface, the normal vector \bar{n} is independent
17 of z , or it is the same as that of Γ defined below (9). Substituting (23) into (22), we have

$$18 \quad \alpha^{(1)}(p)\phi_i(p) = \int_{S_B} [G^{(1)}(p,q) \frac{\partial \phi_i(q)}{\partial n} - \frac{\partial G^{(1)}(p,q)}{\partial n} \phi_i(q)] ds, \quad (24)$$

$$+ \sum_{m=0}^{\infty} \int_{\Gamma} [U_m(p,\xi,\eta) \frac{\partial \phi_{i,m}(\xi,\eta)}{\partial N} - V_m(p,\xi,\eta) \phi_{i,m}(\xi,\eta)] dl$$

19 where

$$20 \quad U_m(p,\xi,\eta) = \int_{-H}^0 [G^{(1)}(p,q) Z_m(\zeta)] d\zeta, \quad (25)$$

$$21 \quad V_m(p,\xi,\eta) = \int_{-H}^0 [\frac{\partial G^{(1)}(p,q)}{\partial N} Z_m(\zeta)] d\zeta. \quad (26)$$

22 Substituting the series form of $G^{(1)}$ in (18) into the above two equations, and noticing the
23 orthogonality of the vertical modes $Z_m(z)$, we have

$$24 \quad U_m(p,\xi,\eta) = -\pi i Z_m(z) H_0^{(2)}(k_m R), \quad (27)$$

$$25 \quad V_m(p,\xi,\eta) = -\pi i Z_m(z) \frac{\partial H_0^{(2)}(k_m R)}{\partial N}. \quad (28)$$

26 When the field point p is located on the interface S_C , we may also replace $\phi_i(p)$ on the left
27 hand side of (24) with (23). Then multiplying both sides of the obtained results with $Z_m(z)$, and
28 integrating with respect to z from $-H$ to 0 , we have

$$\begin{aligned} \frac{C_{\bar{m}}}{\pi} i\alpha^{(1)}\phi_{i,\bar{m}}(x, y) = & \int_{S_b} [Z_{\bar{m}}(\zeta)H_0^{(2)}(k_{\bar{m}}R) \frac{\partial\phi_i(q)}{\partial n} - \frac{\partial Z_{\bar{m}}(\zeta)H_0^{(2)}(k_{\bar{m}}R)}{\partial n} \phi_i(q)] ds, \\ & + C_{\bar{m}} \int_{\Gamma} [H_0^{(2)}(k_{\bar{m}}R) \frac{\partial\phi_{i,\bar{m}}(\xi, \eta)}{\partial N} - \frac{\partial H_0^{(2)}(k_{\bar{m}}R)}{\partial N} \phi_{i,\bar{m}}(\xi, \eta)] dl \end{aligned} \quad (29)$$

2 where

$$C_{\bar{m}} = \int_{-H}^0 Z_{\bar{m}}(z)Z_{\bar{m}}(z)dz = \left[\frac{H}{2} + \frac{\sinh(2k_{\bar{m}}H)}{4k_{\bar{m}}} \right] / \cosh^2(k_{\bar{m}}H). \quad (30)$$

4 3.2. Series expansion in the ice covered sub-domain Ω_2

5 In the ice covered sub-domain Ω_2 , through using variable separation method, we have

$$\phi_i(p) = \sum_m \varphi_{i,m}(x, y)\psi_m(z), \quad (31)$$

7 where the summation contains all possible eigen values and nonzero eigenfunctions, with

$$\nabla^2 \varphi_{i,m} + \kappa_m^2 \varphi_{i,m} = 0, \quad (32)$$

9 and

$$\frac{d^2\psi_m}{dz^2} - \kappa_m^2 \psi_m = 0. \quad (33)$$

11 From (33), (6) and (13), we have

$$\psi_m(z) = \frac{\cosh[\kappa_m(z+H)]}{\cosh(\kappa_m H)}, \quad (34)$$

13 where κ_m are the roots of the dispersion equation in (15). It may be noticed that κ_{-2} and κ_{-1}
14 are two complex roots with negative imaginary parts and symmetric about the imaginary axis, κ_0
15 is the purely positive real root, κ_m ($m=1, \dots, \infty$) is an infinite number of purely negative
16 imaginary roots. Thus (31) becomes

$$\phi_i(p) = \delta_{0,i}\phi_i(p) + \sum_{m=-2}^{\infty} \varphi_{i,m}(x, y)\psi_m(z), \quad (35)$$

18 where $\delta_{0,i}=1$ if $i=0$ and $\delta_{0,i}=0$ for others. The incident velocity potential in (35) can be
19 given as

$$\phi_i(p) = \varphi_i(x, y)\psi_0(z), \quad (36)$$

21 where

$$\varphi_i(x, y) = Ie^{-i\kappa_0(x\cos\beta+y\sin\beta)}, \quad (37)$$

23 with $I = ig/\omega$.

24 Here, it may be noted that Bennetts and Williams [31] used the method of vertical modes, or the
25 velocity potential is expanded into a series of vertical eigenfunctions which are the same as those
26 in (34). Then the functions of κ_{-2} and κ_{-1} can be written in terms of others, i.e. the summation
27 in (31) starts from $m=0$ and the governing equations for $\varphi_{i,m}$ are coupled. Here, we follow the

1 normal variable separation procedure and take into account all the eigen values. The governing
 2 equation for each $\varphi_{i,m}$ in (32) is independent. To solve the Helmholtz equation in (32), together
 3 with the radiation condition in (14), we may use the Green function [14]

$$4 \quad G^{(2)}(p, q) = \frac{\pi}{2i} H_0^{(2)}(\kappa_m R). \quad (38)$$

5 Applying Green's identity to $G^{(2)}(p, q)$ and $\varphi_{i,m}$, we have along the ice sheet edge

$$6 \quad \alpha^{(2)}(p)\varphi_{i,m}(p) = \int_{\Gamma} [G^{(2)}(p, q) \frac{\partial \varphi_{i,m}(q)}{\partial N} - \frac{\partial G^{(2)}(p, q)}{\partial N} \varphi_{i,m}(q)] dl \quad (i=0, \dots, 6), \quad (39)$$

7 where $\alpha^{(2)}$ is the two dimensional solid angle at point p . Here, for $i=0$, the incident velocity
 8 potential should be excluded. It may be noticed that since both $G^{(2)}$ and $\varphi_{i,m}$ satisfy the
 9 radiation condition in (14), the integral along the circle of infinite radius has been removed.

10 It should be noticed when expressing $\varphi_{i,m}$ by $\partial \varphi_{i,m} / \partial N$ through solving the boundary
 11 integral equation (39), irregular frequencies may appear. To avoid this or obtain a unique
 12 relationship between $\varphi_{i,m}$ and $\partial \varphi_{i,m} / \partial N$, (39) is added by a hypersingular integral equation [50]

$$13 \quad \beta \alpha^{(2)}(p) \frac{\partial \varphi_{i,m}(p)}{\partial N_p} = \beta \int_{\Gamma} \left[\frac{\partial G^{(2)}(p, q)}{\partial N_p} \frac{\partial \varphi_{i,m}(q)}{\partial N} - \frac{\partial^2 G^{(2)}(p, q)}{\partial N_p \partial N} \varphi_{i,m}(q) \right] dl. \quad (40)$$

14 Here, β is a coupling constant that can be chosen as $i/|\kappa_m|$ [51].

15 3.3. Matching on the interface between the two sub-domains

16 On the interface S_C between the two sub-domains, both the pressure and normal velocity
 17 should be continuous, i.e.

$$18 \quad \phi_i^{(1)}(x, y, z) = \phi_i^{(2)}(x, y, z), \quad (41)$$

$$19 \quad \frac{\partial \phi_i^{(1)}(x, y, z)}{\partial n} = \frac{\partial \phi_i^{(2)}(x, y, z)}{\partial n}, \quad (42)$$

20 where the superscripts (1) and (2) indicate that the velocity potential and its normal derivative
 21 are for the sub-domain Ω_1 and Ω_2 respectively. To satisfy the conditions in (41) and (42), we
 22 may use the following inner product [15]

$$23 \quad \langle \psi_m, \psi_{\tilde{m}} \rangle = \int_{-H}^0 \psi_m \psi_{\tilde{m}} dz + \frac{L}{\rho_w \omega^2} \left(\frac{\partial \psi_m}{\partial z} \frac{\partial^3 \psi_{\tilde{m}}}{\partial z^3} + \frac{\partial^3 \psi_m}{\partial z^3} \frac{\partial \psi_{\tilde{m}}}{\partial z} \right)_{z=0}. \quad (43)$$

24 Then $\langle \psi_m, \psi_{\tilde{m}} \rangle = 0$ if $m \neq \tilde{m}$, and $\langle \psi_m, \psi_{\tilde{m}} \rangle = Q_m$ if $m = \tilde{m}$, with

$$25 \quad Q_m = \frac{2\kappa_m H + \sinh(2\kappa_m H)}{4\kappa_m \cosh^2(\kappa_m H)} + \frac{2L\kappa_m^4}{\rho_w \omega^2} \tanh^2(\kappa_m H). \quad (44)$$

26 To satisfy the continuity condition of pressure in (41), applying the inner product to $\phi_i^{(2)}$ and $\psi_{\tilde{m}}$,
 27 we have

$$\begin{aligned}
1 \quad & \langle \phi_i^{(2)}, \psi_{\bar{m}} \rangle = \int_{-H}^0 \phi_i^{(2)} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left(\frac{\partial \phi_i^{(2)}}{\partial z} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} + \frac{\partial^3 \phi_i^{(2)}}{\partial z^3} \frac{\partial \psi_{\bar{m}}}{\partial z} \right)_{z=0} \\
& = \int_{-H}^0 \phi_i^{(1)} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left(\frac{\partial \phi_i^{(2)}}{\partial z} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} + \frac{\partial^3 \phi_i^{(2)}}{\partial z^3} \frac{\partial \psi_{\bar{m}}}{\partial z} \right)_{z=0}. \quad (45)
\end{aligned}$$

2 Similarly, we have for the continuity condition of the normal velocity

$$\begin{aligned}
3 \quad & \left\langle \frac{\partial \phi_i^{(2)}}{\partial n}, \psi_{\bar{m}} \right\rangle = \int_{-H}^0 \frac{\partial \phi_i^{(2)}}{\partial n} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left(\frac{\partial^2 \phi_i^{(2)}}{\partial z \partial N} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} + \frac{\partial^4 \phi_i^{(2)}}{\partial z^3 \partial N} \frac{\partial \psi_{\bar{m}}}{\partial z} \right)_{z=0} \\
& = \int_{-H}^0 \frac{\partial \phi_i^{(1)}}{\partial n} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left(\frac{\partial^2 \phi_i^{(2)}}{\partial z \partial N} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} + \frac{\partial^4 \phi_i^{(2)}}{\partial z^3 \partial N} \frac{\partial \psi_{\bar{m}}}{\partial z} \right)_{z=0}. \quad (46)
\end{aligned}$$

4 Substituting equation (35) for $\phi_i^{(2)}$ respectively into the left hand sides of (45) and (46), we have

$$5 \quad (\delta_{0,i} \delta_{0,\bar{m}} \varphi_I + \varphi_{i,\bar{m}}) Q_{\bar{m}} = \int_{-H}^0 \phi_i^{(1)} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left(\frac{\partial \phi_i^{(2)}}{\partial z} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} + \frac{\partial^3 \phi_i^{(2)}}{\partial z^3} \frac{\partial \psi_{\bar{m}}}{\partial z} \right)_{z=0}, \quad (47)$$

6 and

$$7 \quad (\delta_{0,i} \delta_{0,\bar{m}} \frac{\partial \varphi_I}{\partial N} + \frac{\partial \varphi_{i,\bar{m}}}{\partial N}) Q_{\bar{m}} = \int_{-H}^0 \frac{\partial \phi_i^{(1)}}{\partial n} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left(\frac{\partial^2 \phi_i^{(2)}}{\partial z \partial N} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} + \frac{\partial^4 \phi_i^{(2)}}{\partial z^3 \partial N} \frac{\partial \psi_{\bar{m}}}{\partial z} \right)_{z=0}. \quad (48)$$

8 By noticing equation (3) or $\partial^2 \phi_i / \partial z^2 = -\nabla^2 \phi_i$, we may rewrite the above two equations as

$$9 \quad (\delta_{0,i} \delta_{0,\bar{m}} \varphi_I + \varphi_{i,\bar{m}}) Q_{\bar{m}} = \int_{-H}^0 \phi_i^{(1)} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left[\frac{\partial \phi_i^{(2)}}{\partial z} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} - \nabla^2 \left(\frac{\partial \phi_i^{(2)}}{\partial z} \right) \frac{\partial \psi_{\bar{m}}}{\partial z} \right]_{z=0}, \quad (49)$$

10 and

$$11 \quad (\delta_{0,i} \delta_{0,\bar{m}} \frac{\partial \varphi_I}{\partial N} + \frac{\partial \varphi_{i,\bar{m}}}{\partial N}) Q_{\bar{m}} = \int_{-H}^0 \frac{\partial \phi_i^{(1)}}{\partial n} \psi_{\bar{m}} dz + \frac{L}{\rho_w \omega^2} \left\{ \frac{\partial^2 \phi_i^{(2)}}{\partial z \partial N} \frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} - \left[\frac{\partial}{\partial N} \nabla^2 \left(\frac{\partial \phi_i^{(2)}}{\partial z} \right) \right] \frac{\partial \psi_{\bar{m}}}{\partial z} \right\}_{z=0}. \quad (50)$$

12 Using

$$13 \quad \frac{\partial x}{\partial s} = -\sin \Theta \quad \text{and} \quad \frac{\partial y}{\partial s} = \cos \Theta, \quad (51)$$

14 we can write the operators \mathcal{B} and \mathcal{S} in (8) and (9) respectively as

$$15 \quad \mathcal{B} = \nabla^2 - \nu_0 \left(\frac{\partial^2}{\partial s^2} + \frac{\partial \Theta}{\partial s} \frac{\partial}{\partial N} \right), \quad (52)$$

$$16 \quad \mathcal{S} = \frac{\partial}{\partial N} \nabla^2 + \nu_0 \frac{\partial}{\partial s} \left(\frac{\partial^2}{\partial s \partial N} - \frac{\partial \Theta}{\partial s} \frac{\partial}{\partial s} \right). \quad (53)$$

17 It should be noticed that in (52) and (53), the partial derivatives with respect to s are carried out
18 with respect to the curvilinear coordinate, instead of the tangential direction. For the first order
19 derivative, the former is the same as the latter. However, this may not be the case for higher order
20 derivatives. Invoking the ice sheet edge condition in (7), we then have

$$21 \quad \nabla^2 \left(\frac{\partial \phi_i^{(2)}}{\partial z} \right) = \nu_0 \left(\frac{\partial^2}{\partial s^2} + \frac{\partial \Theta}{\partial s} \frac{\partial}{\partial N} \right) \left(\frac{\partial \phi_i^{(2)}}{\partial z} \right), \quad (54)$$

$$22 \quad \frac{\partial}{\partial N} \nabla^2 \left(\frac{\partial \phi_i^{(2)}}{\partial z} \right) = -\nu_0 \frac{\partial}{\partial s} \left(\frac{\partial^2}{\partial s \partial N} - \frac{\partial \Theta}{\partial s} \frac{\partial}{\partial s} \right) \left(\frac{\partial \phi_i^{(2)}}{\partial z} \right). \quad (55)$$

1 Substituting equations (54) and (55) into (49) and (50) respectively, and noticing equations (35)
 2 and (23), we have

$$3 \quad (\delta_{0,i}\delta_{0,\bar{m}}\varphi_I + \varphi_{i,\bar{m}})Q_{\bar{m}} = \sum_{m=0}^{\infty} \phi_{i,m} E_{m,\bar{m}} + \sum_{m=-2}^{\infty} f_{m,\bar{m}}^P \varphi_{i,m} + \delta_{0,i} f_{0,\bar{m}}^P \varphi_I, \quad (56)$$

4 and

$$5 \quad (\delta_{0,i}\delta_{0,\bar{m}} \frac{\partial \varphi_I}{\partial N} + \frac{\partial \varphi_{i,\bar{m}}}{\partial N})Q_{\bar{m}} = \sum_{m=0}^{\infty} \frac{\partial \phi_{i,m}}{\partial N} E_{m,\bar{m}} + \sum_{m=-2}^{\infty} f_{m,\bar{m}}^V \varphi_{i,m} + \delta_{0,i} f_{0,\bar{m}}^V \varphi_I. \quad (57)$$

6 where the operators $f_{m,\bar{m}}^P$ and $f_{m,\bar{m}}^V$ are respectively defined as

$$7 \quad f_{m,\bar{m}}^P = \frac{L}{\rho_w \omega^2} \frac{\partial \psi_m}{\partial z} \left[\left(\frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} - v_0 \frac{\partial \psi_{\bar{m}}}{\partial z} \frac{\partial^2}{\partial s^2} \right) - v_0 \frac{\partial \psi_{\bar{m}}}{\partial z} \frac{\partial \Theta}{\partial s} \frac{\partial}{\partial N} \right]_{z=0}, \quad (58)$$

$$8 \quad f_{m,\bar{m}}^V = \frac{L}{\rho_w \omega^2} \frac{\partial \psi_m}{\partial z} \left[\left(\frac{\partial^3 \psi_{\bar{m}}}{\partial z^3} + v_0 \frac{\partial \psi_{\bar{m}}}{\partial z} \frac{\partial^2}{\partial s^2} \right) \frac{\partial}{\partial N} - v_0 \frac{\partial \psi_{\bar{m}}}{\partial z} \left(\frac{\partial^2 \Theta}{\partial s^2} \frac{\partial}{\partial s} + \frac{\partial \Theta}{\partial s} \frac{\partial^2}{\partial s^2} \right) \right]_{z=0}, \quad (59)$$

9 and

$$10 \quad E_{m,\bar{m}} = \int_{-H}^0 Z_m \psi_{\bar{m}} dz = \frac{1}{\cosh(k_m H) \cosh(\kappa_{\bar{m}} H)} \left\{ \frac{\sinh[H(k_m - \kappa_{\bar{m}})]}{2(k_m - \kappa_{\bar{m}})} + \frac{\sinh[H(k_m + \kappa_{\bar{m}})]}{2(k_m + \kappa_{\bar{m}})} \right\}. \quad (60)$$

11 It may be noticed that in equations (56) and (57), there exist up to second order partial derivatives
 12 with respect to the curvilinear coordinate s , which can be obtained numerically through the five
 13 point finite difference method [52], or

$$14 \quad \frac{\partial f_i}{\partial s} = \frac{1}{12\Delta s} (f_{i-2} - 8f_{i-1} + 8f_{i+1} - f_{i+2}), \quad (61)$$

$$15 \quad \frac{\partial^2 f_i}{\partial s^2} = \frac{1}{12\Delta s^2} (-f_{i-2} + 16f_{i-1} - 30f_i + 16f_{i+1} - f_{i+2}), \quad (62)$$

16 where Δs is the equally spaced arc length between the two adjacent points.

17 3.4. Hydrodynamic coefficients and wave exciting force

18 After the velocity potentials in sub-domains Ω_1 and Ω_2 have been solved, we can obtain the
 19 pressure at any point in fluid through the linear Bernoulli equation. Then the hydrodynamic force
 20 exerting on the body can be computed through integrating the pressure over the body surface.
 21 Invoking the decomposition of the velocity potential in equation (2), we may divide the total force
 22 into two parts, i.e. the radiation force due to the forced body oscillation, and the wave exciting
 23 force due to the incident and diffracted potentials [2]. For the radiation force, we may write it in
 24 form of added mass μ_{jk} and damping coefficient λ_{jk} as

$$25 \quad \mu_{jk} - i \frac{\lambda_{jk}}{\omega} = \rho_w \int_{S_B} \phi_k n_j ds, \quad (63)$$

26 while for the wave exciting force $f_{E,j}$, we have

$$27 \quad f_{E,j} = -i\omega\rho_w \int_{S_B} \phi_0 n_j ds. \quad (64)$$

1 3.5. Numerical solution through discretization and truncation

2 To solve the velocity potentials ϕ_i numerically, the complex body surface S_B is divided into
3 N_B flat panels, while the curved ice sheet edge Γ is divided into N_Γ straight line segments.
4 On each panel or segment, the potentials ϕ_i or $\phi_{i,m}$ and $\varphi_{i,m}$ are assumed to be constant, and
5 the conditions are imposed at the centre of each panel. This means that the solid angle α_1 in the
6 integral equation (21) can always be taken to be 2π , and the solid angle α_2 in the integral
7 equation (39) can always be taken to be $-\pi$, where minus sign is due to the fact that the normal
8 vector is pointing from free surface into the ice sheet. For the infinite series in (23) and (35), the
9 upper bound is truncated at $M-1$ and $M-3$ respectively. Therefore, we will have
10 $N_B + 4N_\Gamma \times M$ unknowns. From (24) and (29) we can obtain $N_B + N_\Gamma \times M$ equations, while
11 from equation (39) we have another $N_\Gamma \times M$ equations. The continuity requirement in (56) and
12 (57) will provide additional $2N_\Gamma \times M$ equations. This makes a total of $N_B + 4N_\Gamma \times M$
13 equations, which is the same as the number of unknowns.

14 The Hankel function used in (39) contains a logarithmic singularity as the source point
15 approaches to the field point, or

$$16 \quad H_0^{(2)}(kR) \rightarrow \frac{2}{\pi i} \ln(R) \text{ as } R \rightarrow 0. \quad (65)$$

17 Thus, when conducting the integration of the Hankel function over a line element, we may
18 subtract the singular term from the integral

$$19 \quad H_0^{(2)}(kR) = [H_0^{(2)}(kR) - \frac{2}{\pi i} \ln(R)] + \frac{2}{\pi i} \ln(R), \quad (66)$$

20 The integration of the first term can be carried out through the standard numerical scheme, while
21 the second term can be evaluated analytically [2].

22 4. Numerical Results

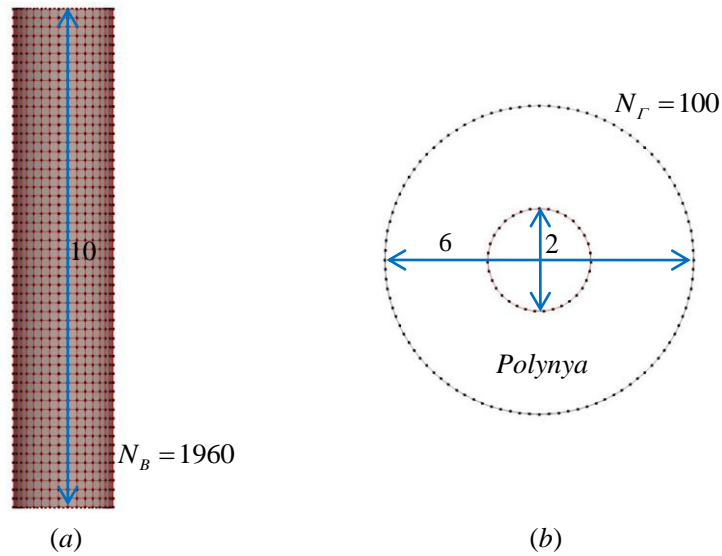
23 In this section, a bottom mounted vertical circular cylinder in a circular polynya is first studied
24 to verify the developed method. Then the code WISPICE is used for obtaining the hydrodynamic
25 loads on a FPSO, which is a realistic offshore structure rather than an idealized geometry, floating
26 in a polynya. In following numerical computations, the typical parameters of ice sheet are taken to
27 be [8]

$$28 \quad E = 5 \text{ GPa}, \quad \nu = 0.3, \quad \rho_i = 922.5 \text{ kg m}^{-3}, \quad (67)$$

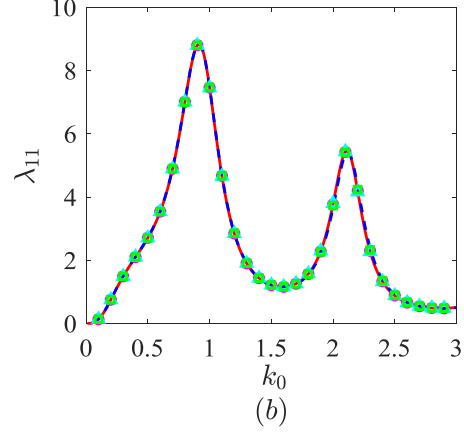
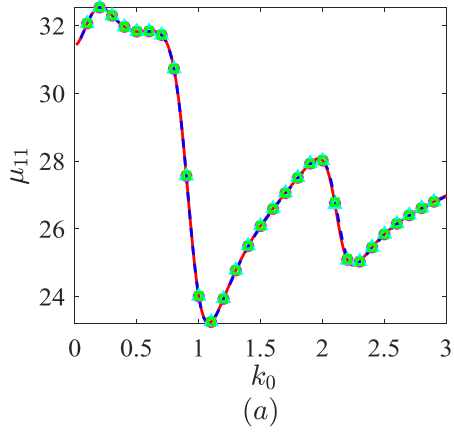
29 to provide physical meaningful results. All the results will be presented in the dimensionless form
30 based on the basic parameters, i.e. density of water $\rho_w = 1025 \text{ kg m}^{-3}$, acceleration due to gravity
31 $g = 9.80 \text{ m s}^{-2}$, and a characteristic length scale.

32 4.1. Verification: a bottom mounted vertical circular cylinder in a circular polynya

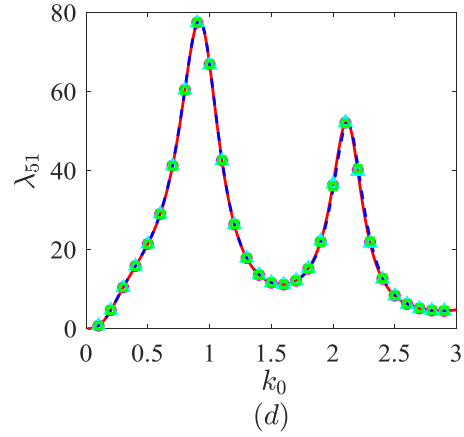
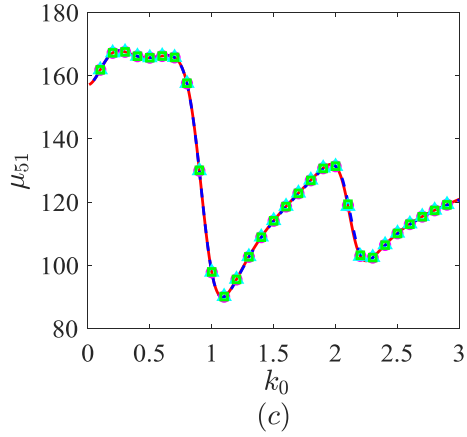
1 We first consider the wave radiation and diffraction problems of a circular cylinder with radius
 2 a standing in a circular polynya with radius c . The centre of cylinder coincides with that of
 3 polynya. For the rotational motion, it is assumed that the cylinder is articulated at the bottom or
 4 $\vec{r}_0 = (0, 0, -H)$ and allowed to rotate in the Oxz plane, as in Drake, Eatock Taylor and Matsui
 5 [53]. The case has been investigated by Ren, Wu and Ji [46] analytically, through the matched
 6 eigenfunction expansions. To demonstrate the accuracy and efficiency of the present hybrid
 7 method, we adopt the same parameters as those in Ren, Wu and Ji [46], and the radius of cylinder
 8 a is chosen as the characteristic length scale, i.e. $a = 1$, $c = 3$, $H = 10$, $h = 0.1$. The incident
 9 flexural-gravity wave angle is set to be $\beta = 0$. There are 1960 panels on the body surface, and
 10 100 line segments on the ice sheet edge, with the discretization being shown in Fig. 2. The infinite
 11 series of the eigenfunction expansions is truncated at $M = 50$. These have been found to be
 12 sufficient for convergence based on the convergence study similar to that in Shi, Li and Wu [36].
 13 Fig. 3 shows the added mass μ_{ij} and damping coefficient λ_{ij} against wave number k_0 ,
 14 together with the analytical results from Ren, Wu and Ji [46] denoted by dashed lines. The
 15 corresponding wave exciting force in surge and pitch are given in Fig. 4. It can be seen from these
 16 two figures that **further increasing N_B , N_r and M will give graphically indistinguishable**
 17 **results, and the convergent** numerical results are in excellent agreement with those analytical
 18 solutions. This shows that the present hybrid method is accurate for wave radiation and diffraction
 19 problems of a structure in a polynya. It means that results at a desired accuracy can be achieved
 20 through refining the mesh with larger N_B and N_r , and increasing the number of terms M
 21 kept in the infinite series.



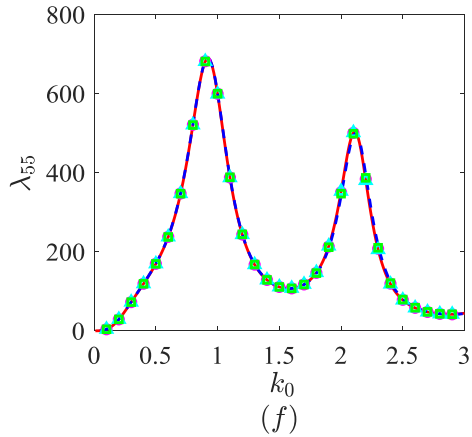
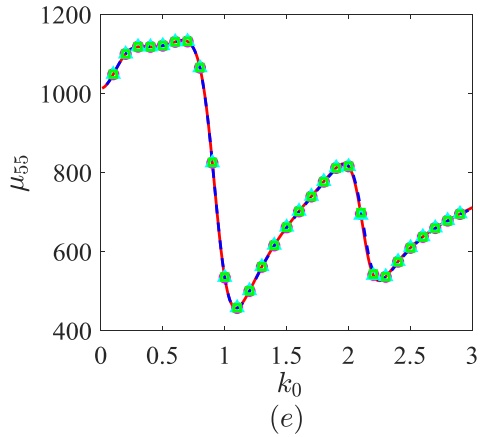
22
 23 Fig. 2. The mesh on the body surface and ice sheet edge. (a) on the body; (b) on the ice sheet edge.



1

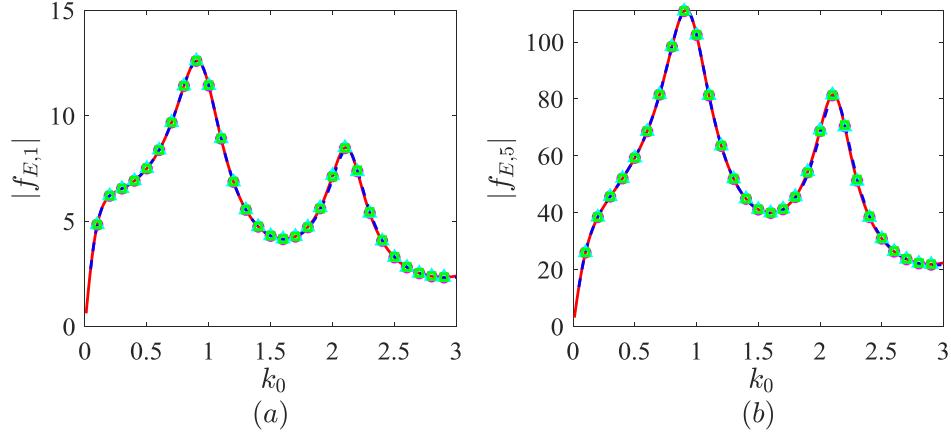


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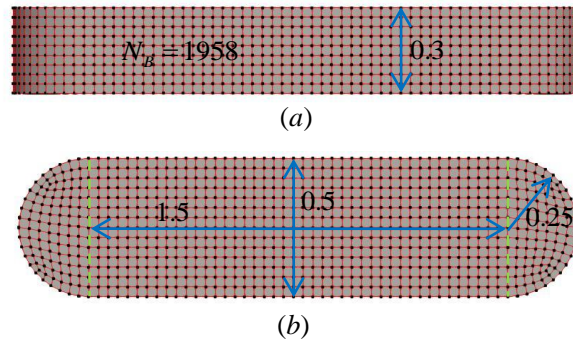
4 Fig. 3. Added mass μ_{jk} and damping coefficient λ_{jk} of a circular cylinder in a circular polynya against wave
5 number k_0 . Solid lines: numerical results by the present hybrid method ($N_B = 1960$, $N_r = 100$, $M = 50$);
6 dashed lines: analytical solutions from Fig. 4 of Ren, Wu and Ji [46]; circles: finer mesh on the body surface with
7 $N_B = 2924$; triangles: finer mesh on the ice sheet edge with $N_r = 150$; squares: more terms kept in the series
8 with $M = 75$. ($a=1$, $c=3$, $H=10$, $h=0.1$, $m=0.09$, $L=4.5582$, $\beta=0$)



1
2 Fig. 4. Wave exciting force $f_{E,j}$ of a circular cylinder in a circular polynya against wave number k_0 . (As in Fig.
3 3.)

4 4.2. Application: hydrodynamic loads on a FPSO floating in a polynya

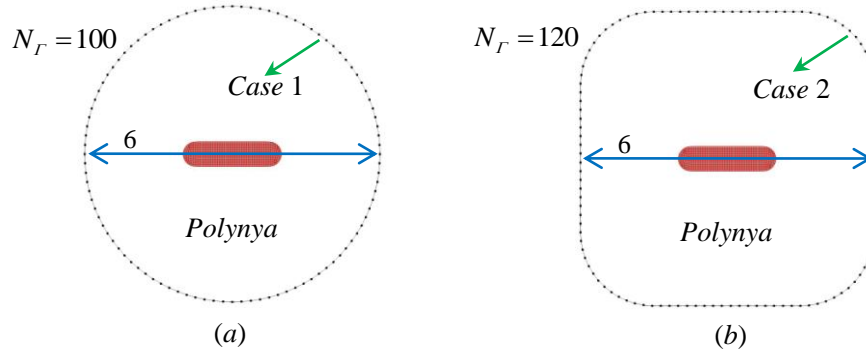
5 As shown in Fig. 5 (a) and (b), a floating structure FPSO is chosen for the case study. Its
6 half-length l is chosen as the characteristic length for non-dimensionalisation. The cross section
7 of middle ship is rectangle, with its length and width respectively to be 1.5 and 0.5. The bow
8 and stern have the same shape, a half cylinder with radius as 0.25. The draught of the FPSO is
9 0.3. On the mean wetted body surface of FPSO, $N_B = 1958$ quadrilateral panels are used, which
10 is sufficient for convergence. The rotational centre is assumed at the geometry centre of FPSO.
11 The water depth and ice sheet thickness are taken as $H = 10$ and $h = 0.1$ respectively. Five
12 cases will be considered in this section, i.e. 1) case 1 is for a circular polynya with its radius as
13 $c = 3$, and the FPSO is centrally located; 2) case 2 is similar to case 1, but with the shape of
14 polynya as a square with its half-length taken to be $d = 3$; 3) case 3 is similar to case 1, but with a
15 much larger polynya or $c = 5$; 4) case 4 is similar to case 3 but with geometry centre at
16 $(1, 0, -0.15)$; 5) case 5 is similar to case 3 but with geometry centre at $(0, 1, -0.15)$. Without
17 losing generality, in all of these five cases the incident flexural-gravity wave angle is taken as
18 $\beta = \pi/4$. The wave number k_0 varies from 0.01 to 3.0, and the increment has been chosen
19 to be sufficiently small, or 0.01, to capture the position and the value of the peak of the curve.



20
21 Fig. 5. The geometry shape and mesh on the mean wetted body surface of FPSO.

1 4.2.1. Hydrodynamic loads on a FPSO floating in polynya with different shapes

2 Here, case 1 and case 2 respectively for circular and square polynyas will be considered to show
 3 the effects of polynya shape on the hydrodynamic loads. It may be noticed that the main
 4 dimensions of both cases are the same or $c = d = 3$. In latter case, the four corners of the square
 5 polynya are smoothed by circular arc of radius 1.5. As shown in Fig. 6 (a) and (b), the circular
 6 and square ice sheet edges are respectively discretized into $N_r = 100$ and $N_r = 120$ straight
 7 line segments, and convergent results can be obtained.



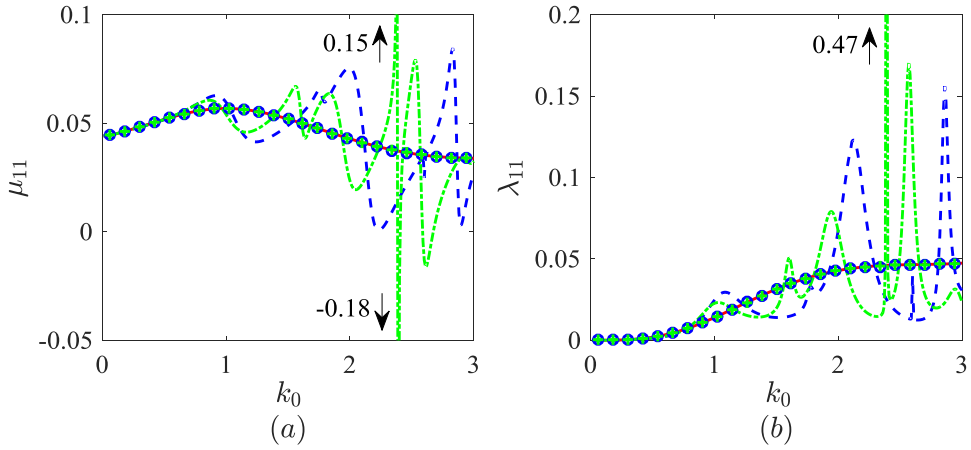
8
 9 Fig. 6. The mesh on the ice sheet edge. (a) case 1 for a circular polynya ($N_r = 100$); (b) case 2 for a square
 10 polynya with smoothed corners ($N_r = 120$).

11 The computed diagonal terms of the added mass and damping coefficient are shown in Fig. 7
 12 against wave number k_0 , while the wave exciting force are plotted in Fig. 8. To see the
 13 asymptotic behaviours as $h \rightarrow 0$, the numerical results for a very small ice thickness or
 14 $h = 0.001$ are plotted, too. As a comparison, the results corresponding to the open sea or $h = 0$
 15 are also provided. It should be mentioned that for $h = 0$ both the hybrid method and the direct
 16 method have been used, and graphically indistinguishable results are obtained. Here, direct
 17 method means that the velocity potential are computed through the boundary integral equation in
 18 (21) over the body surface only. From these two figures, it can be seen that when h is small, the
 19 numerical results are very close to those for open water. In fact, as $h \rightarrow 0$ both the dispersion
 20 equation and the eigenfunctions for an ice sheet will tend to those for open water, and the inner
 21 product in equation (43) will become the usual definition of orthogonality. Therefore, as $h \rightarrow 0$
 22 the solution for sub-domain Ω_2 will serve as a radiation condition of free surface for sub-domain
 23 Ω_1 .

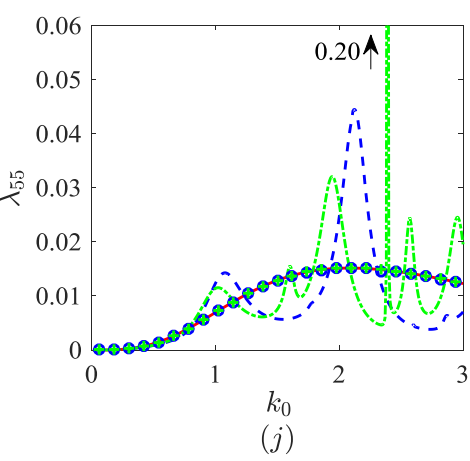
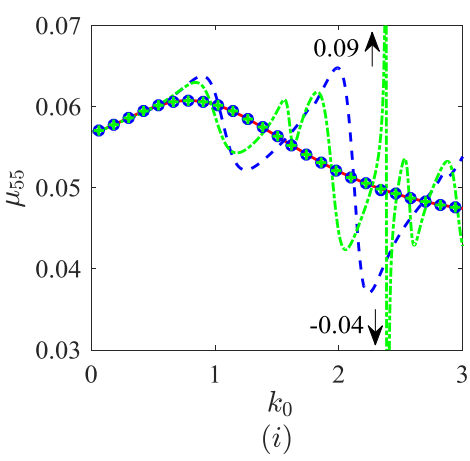
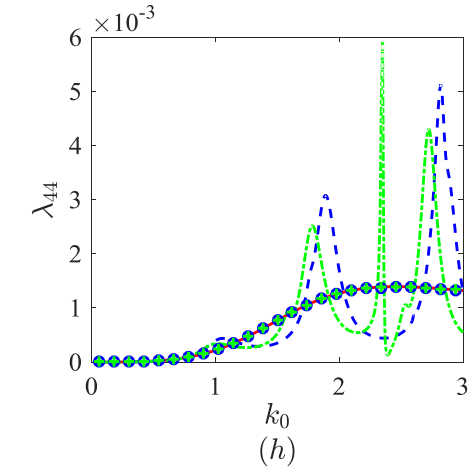
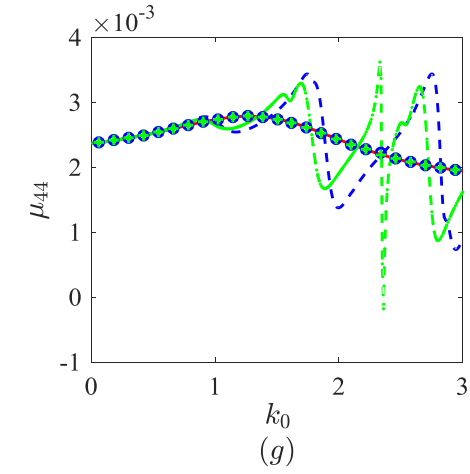
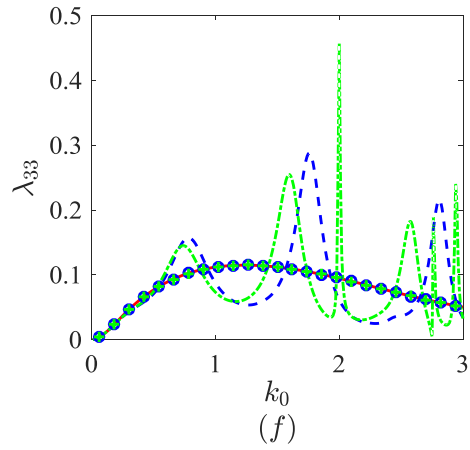
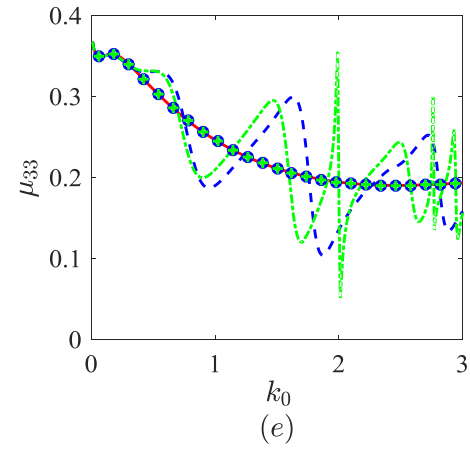
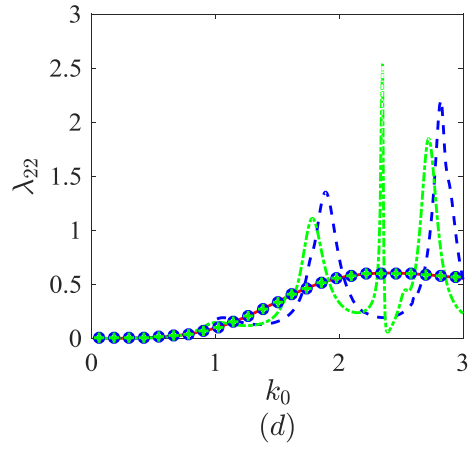
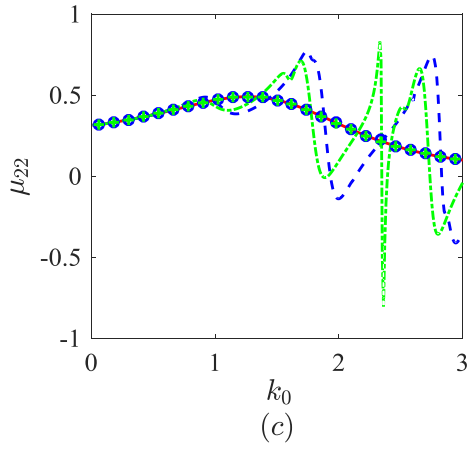
24 When the wave number is small, the results of the hydrodynamic loads for polynya are very
 25 close to those for open sea, as can be observed from Figs. 7 and 8. From equations (5) and (6), we
 26 have that when $k_0 \rightarrow 0$ or $\omega \rightarrow 0$, both the leading terms of the boundary conditions on free
 27 surface and ice sheet will tend to be the same or $\partial\phi_i / \partial z = 0$. This means that the upper surface
 28 boundary condition for polynya will be the same as that for open sea as $k_0 \rightarrow 0$. Thus the results

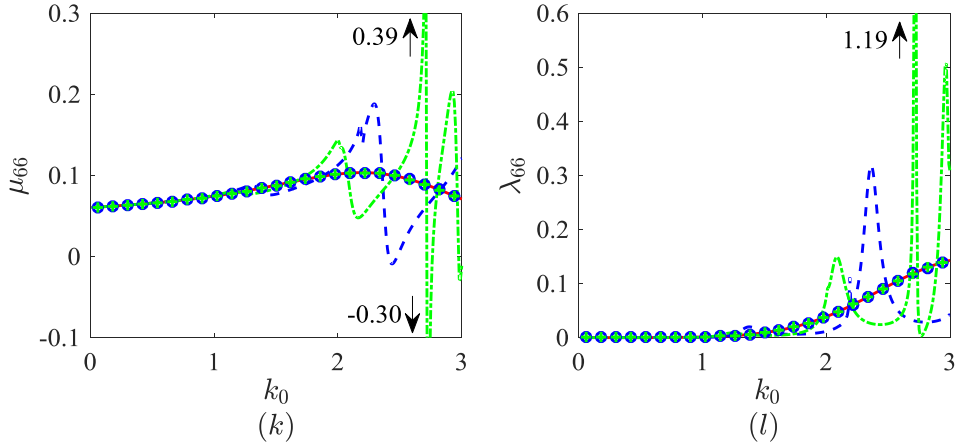
1 for FPSO floating in polynya will be very similar to those for open sea when k_0 is small.

2 However, when k_0 becomes larger, discrepancy appears. The hydrodynamic loads for polynya
 3 exhibit an oscillatory behavior around those for open sea, as shown in Figs. 7 and 8. From the
 4 dispersion equations (15) and (17), we have that when waves propagate from open sea to the ice or
 5 the other way around, there will be a change of the dispersion relation, i.e. the waves will be both
 6 reflected back to the original region and transmitted to the other region. Thus, as the wave
 7 generated or disturbed by the structure encounters the ice edge, part of it will be reflected back to
 8 the structure, and then will be further reflected back to the ice edge. This resembles the “sloshing
 9 wave in a tank”, leading to the oscillations of the hydrodynamic loads. These oscillatory features
 10 are similar to those in a two dimensional polynya [40]. An important difference is that for three
 11 dimensional polynya, the wave energies will also propagate laterally. This means that the
 12 hydrodynamic loads on the body surface will be affected not only by the dimension of polynya,
 13 but also by its geometry. This can be clearly seen in Figs. 7 and 8 that when the wave number k_0
 14 is not small, the hydrodynamic loads for a square polynya will exhibit a quite different oscillatory
 15 patterns with those for a circular polynya, although the main dimension of these two polynyas are
 16 similar. Specifically, for the former case, in addition to the overall variations, there are also some
 17 local oscillations vary sharply from peaks to the troughs. These are similar to the resonant motions
 18 of fluid in a tank, in which circular and rectangular shapes would resemble different natural
 19 frequencies [2]. Another similar problem is a ship floating in a harbor, and very large
 20 hydrodynamic loads on the structure could occur at some specific wave numbers [36]. However, a
 21 significant physical difference is that in our work only part of wave energy will be reflected back
 22 due to the change of wave dispersion relation, while in the former two cases the wave energy will
 23 be fully reflected by the solid wall due to the non-penetration condition. All these show the
 24 importance of the effects of polynya shape on the hydrodynamic loads for a realistic structure
 25 operating in icy waters.

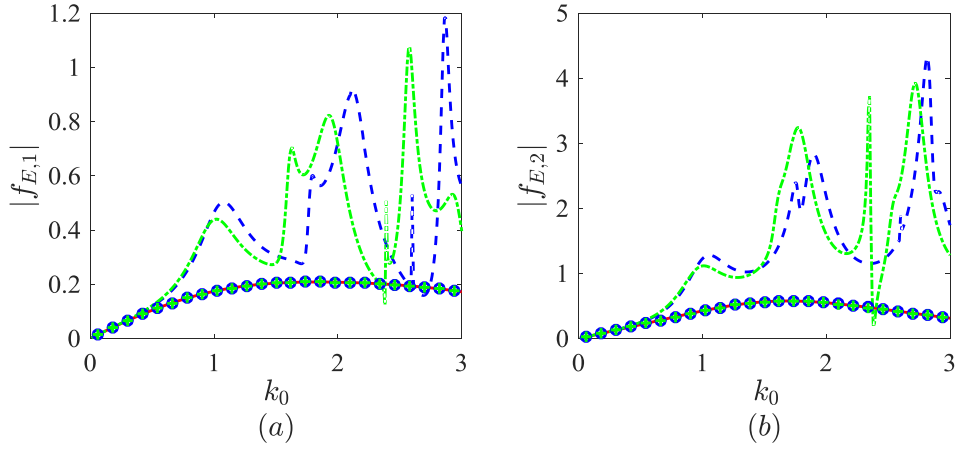


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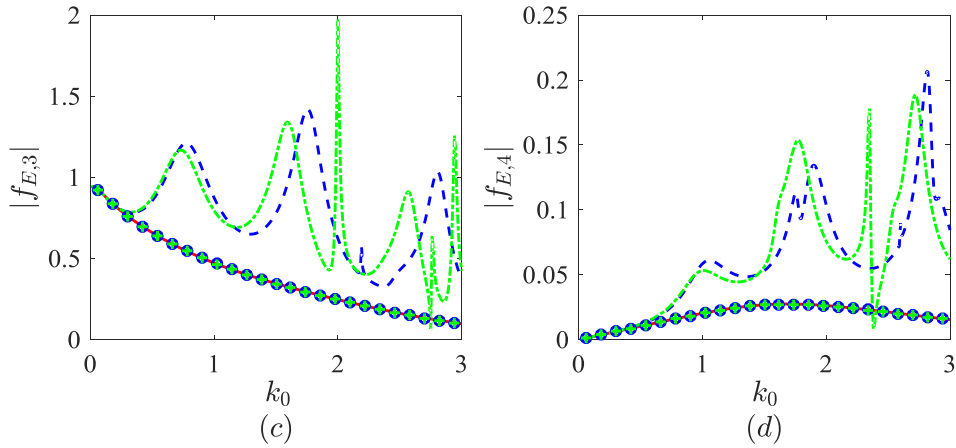




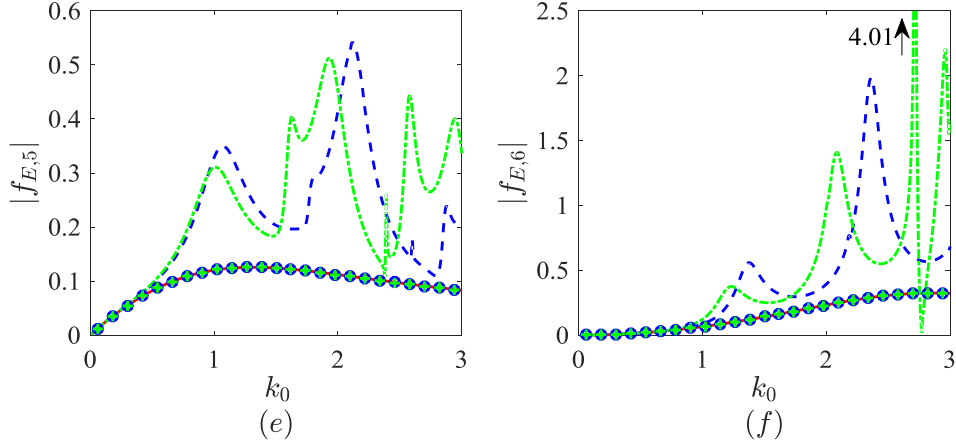
1
 2 Fig. 7. Added mass μ_{jk} and damping coefficient λ_{jk} of a FPSO floating in polynya with different shapes
 3 against wave number k_0 . Solid lines: results for open sea; dashed lines: results for circular polynya; dash-dotted
 4 lines: results for square polynya; circles: similar to dashed lines but for $h=0.001$; pluses: similar to dash-dotted
 5 lines but for $h=0.001$. ($l=1$, $c=d=3$, $H=10$, $h=0.1$, $m=0.09$, $L=4.5582$, $\beta=\pi/4$)



6



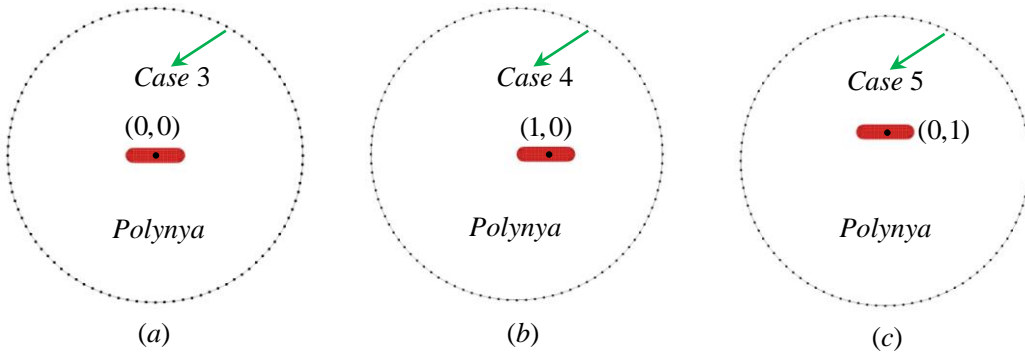
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1
2 Fig. 8. Wave exciting force $f_{E,j}$ of a FPSO floating in polynya with different shapes against wave number k_0 .
3 (As in Fig. 7.)

4 *4.2.2. Hydrodynamic loads on a FPSO floating in polynya with different positions*

5 Computations are now carried out for cases 3 to 5 to show the effects of floating position of
6 FPSO in polynya on the hydrodynamic loads. In these three cases, the polynya has the same shape
7 as that in Fig. 6 (a), but with a much larger polynya radius or $c = 5$. Here, three different floating
8 positions of FPSO are considered, i.e. centrally located, near right ice sheet edge with geometry
9 centre at $(1,0,-0.15)$, near top ice sheet edge with geometry centre at $\vec{r}_0 = (0,1,-0.15)$, as shown
10 in Fig. 9.

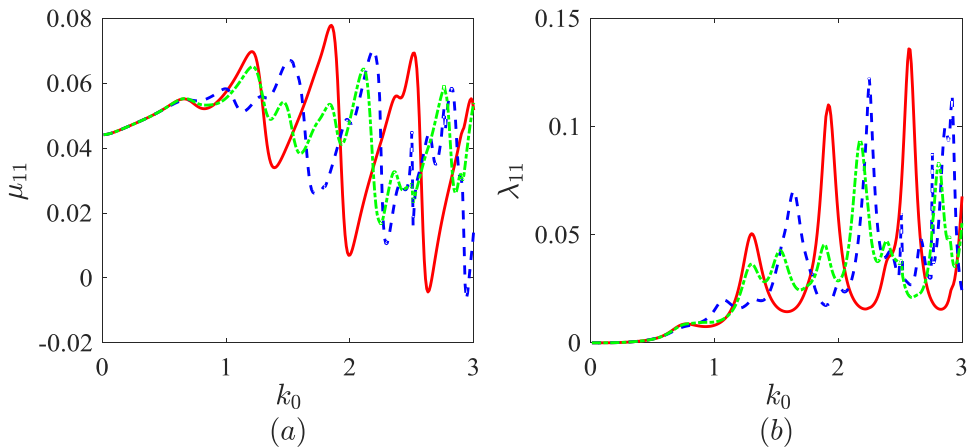


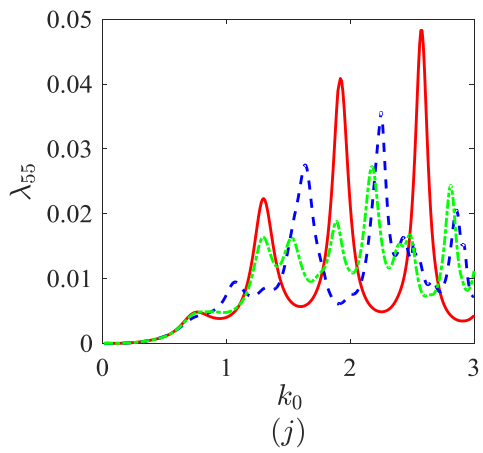
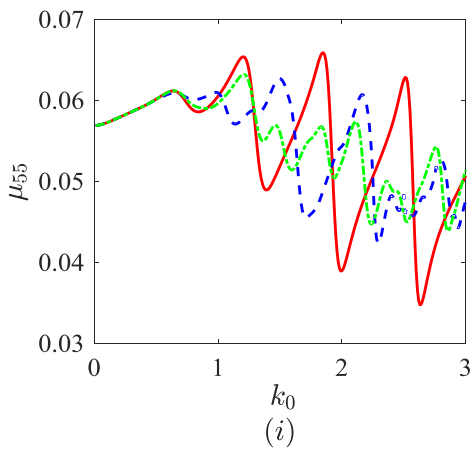
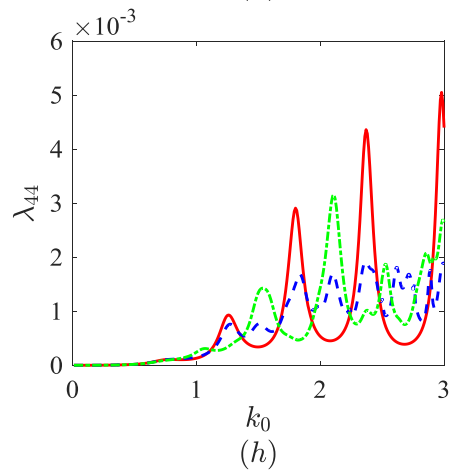
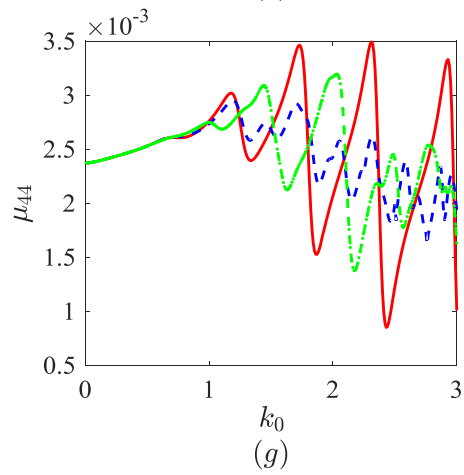
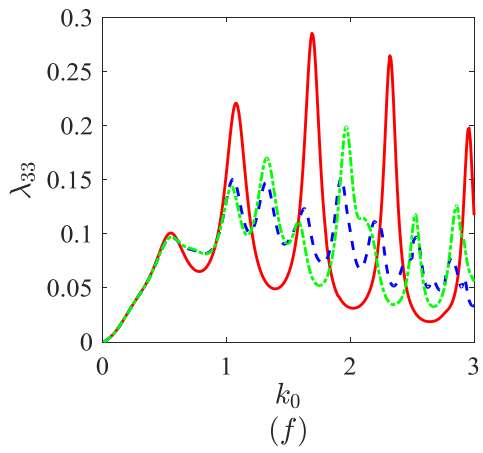
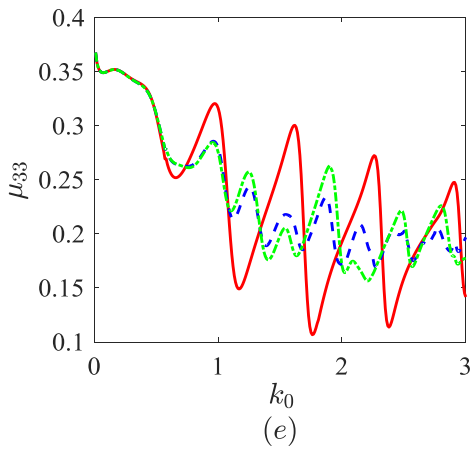
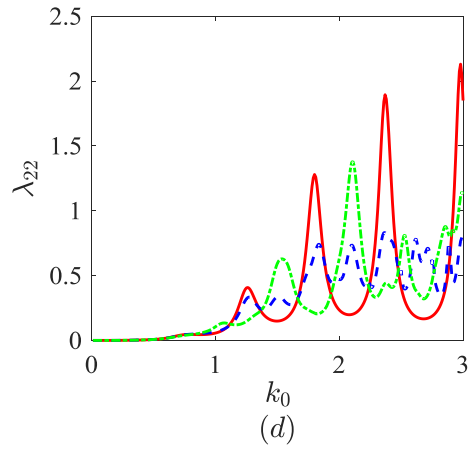
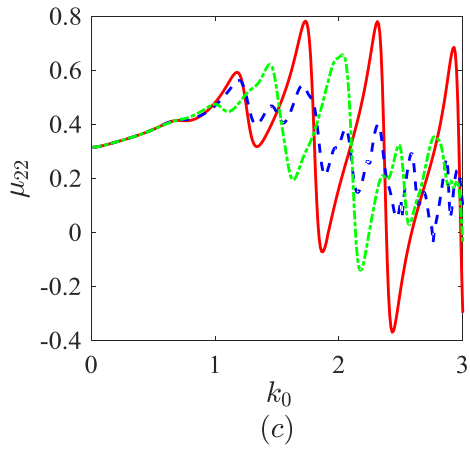
11
12 Fig. 9. The floating position of FPSO in a circular polynya with radius $c = 5$. (a) case 3 for centrally located; (b)
13 case 4 for near the right ice sheet edge; (c) case 5 for near the top ice sheet edge.

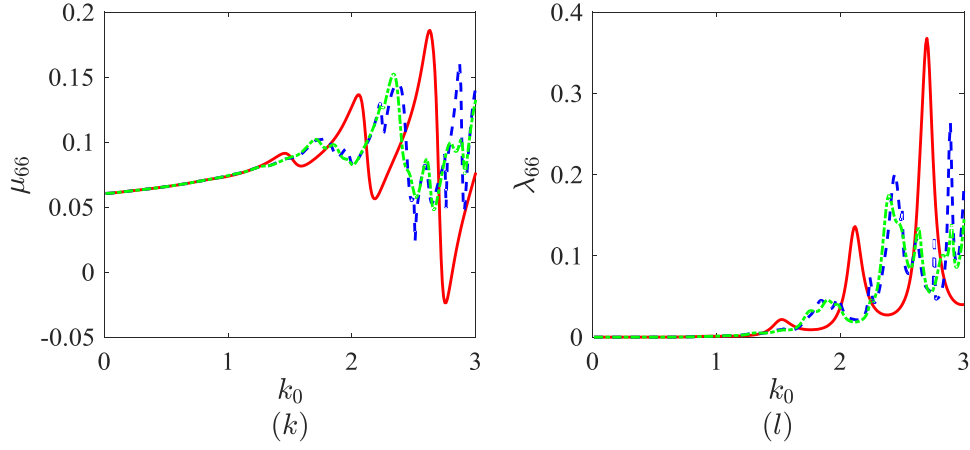
14 Fig. 10 shows the diagonal terms of the hydrodynamic coefficients of a FPSO floating in a
15 circular polynya in different positions, i.e. centrally located, near the right and top ice sheet edges
16 respectively. The corresponding wave exciting force is given in Fig. 11. For the centrally located
17 case, it differs from that in Figs. 7 and 8 only by the size of the polynya. As one can expect, the
18 oscillatory behaviour of the results will be very much affected by $k_0 c$. Thus there are more peaks
19 and troughs in the Figs. 10 and 11 with larger c within the same wave number span as in Figs. 7
20 and 8. When the polynya shape keeps the same and floating position of the body varies, the results

1 in Figs. 10 and 11 reveal that the hydrodynamic loads can be much more oscillatory than those for
2 centrally located case. These may be partly explained by the approximate formulations in Li, Shi
3 and Wu [40] for a 2D body floating in a wide polynya. In such a simplified model, through
4 ignoring the evanescent wave modes in the coupled ice body interactions, the hydrodynamic loads
5 on the body in polynya can be computed via the solutions of wave ice interactions without the
6 body and wave body interactions without the ice. The explicit equations show that at a given wave
7 number, the hydrodynamic loads on the body will oscillate periodically against two spatial
8 variables, i.e. the width of the polynya and the distance between the polynya centre and body
9 centre. When the value of these two parameters increase, the corresponding oscillatory period will
10 decrease or the results will be more oscillatory. While for a centrally located body, only the first
11 oscillatory term will appear because of symmetry. However, when the body is away from the
12 polynya centre, there will be two oscillatory terms, leading to a much more oscillatory results.

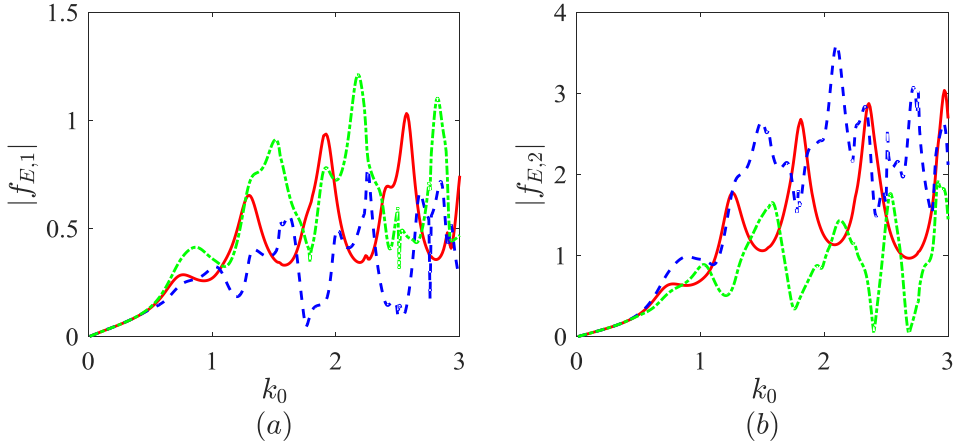
13 The wave elevation w_0 in polynya along the ice edge due to incident and diffracted waves is
14 shown in Fig. 12 against $\theta = \arctan(y/x)$, at different wave numbers k_0 . While in Fig. 13, w_0
15 is provided against k_0 at different ice edge positions θ . From these two figures we observe that
16 when $k_0 \rightarrow 0$ or the wave length tends to infinity, w_0 will tend to 1 or the diffracted wave will
17 be zero. This is consistent with the fact that as $k_0 \rightarrow 0$ both the boundary conditions on ice sheet
18 and free surface will tend to be the same, i.e. no diffracted wave will be generated by the polynya
19 and then by the body due to an incident wave of infinite wavelength. As k_0 increases, it can be
20 seen that the variation of w_0 against θ for a given k_0 or against k_0 for a given θ becomes
21 highly complex. Specifically, the wave elevation in polynya can be many times of the incident
22 wave amplitude. Strictly speaking, the linear theory is for infinitesimal wave. In practical
23 problems, this applicability depends on the accuracy desired. It is then possible that the linear
24 theory may be valid for the incident wave, but the nonlinearity may become important in the
25 polynya. In such a case, nonlinear correction should be introduced into the model, or the linear
26 theory is valid only for even smaller incoming wave amplitude.



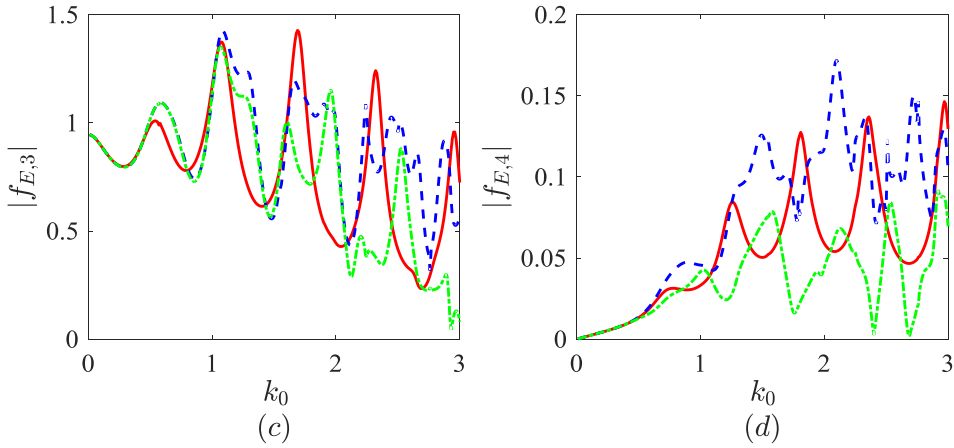




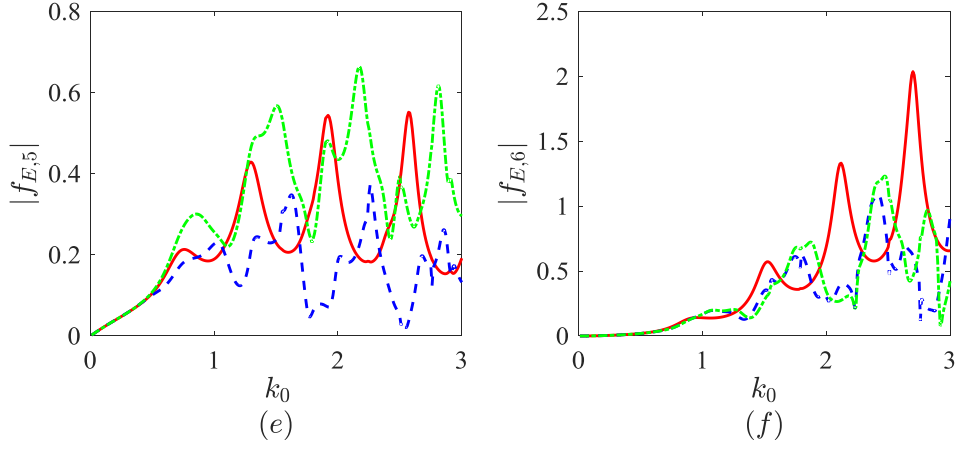
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 2 Fig. 10. Added mass μ_{jk} and damping coefficient λ_{jk} of a FPSO floating in polynya with different positions
 3 against wave number k_0 . Solid lines: centrally located with $(x_0, y_0) = (0, 0)$; dashed lines: near the right ice sheet
 4 edge with $(x_0, y_0) = (1, 0)$; dash-dotted lines: near the top ice sheet edge with $(x_0, y_0) = (0, 1)$. ($l = 1$, $c = 5$,
 5 $H = 10$, $h = 0.1$, $m = 0.09$, $L = 4.5582$, $\beta = \pi/4$)



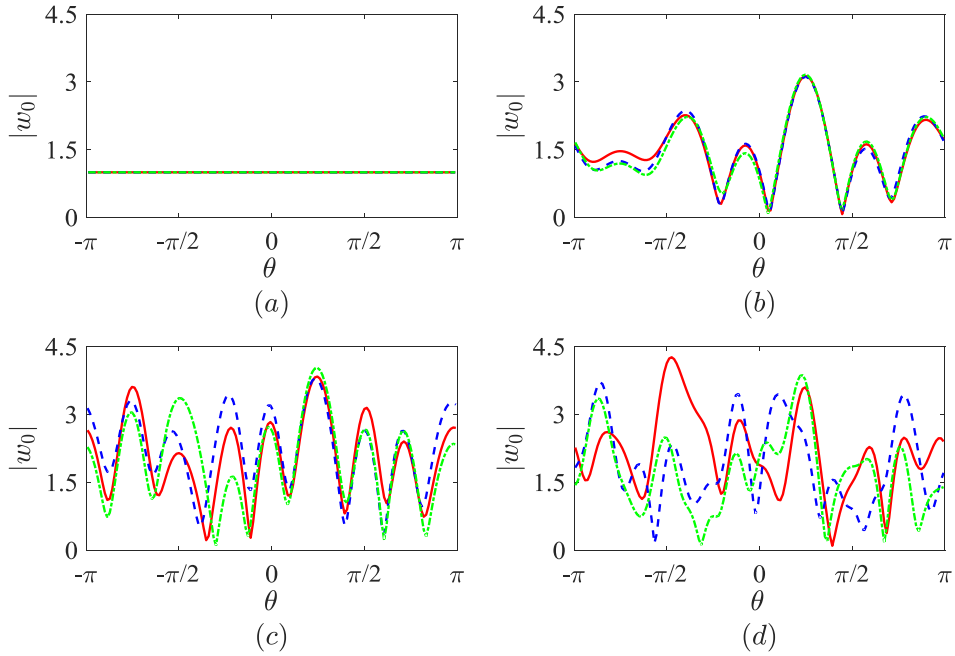
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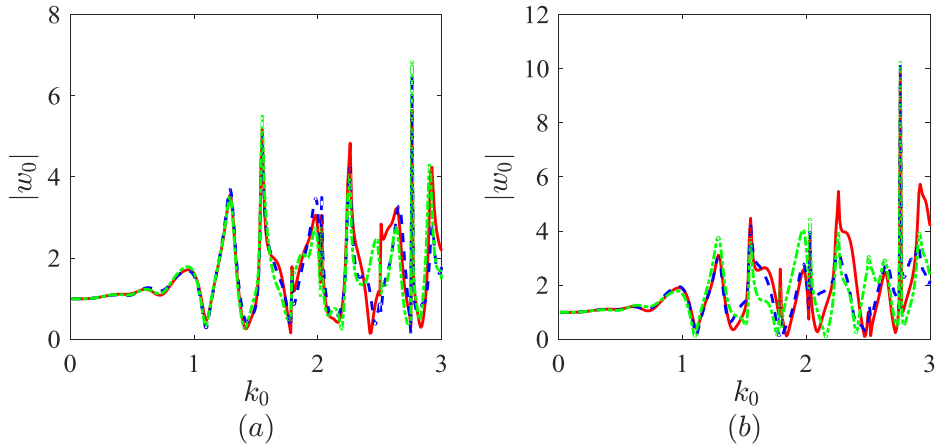
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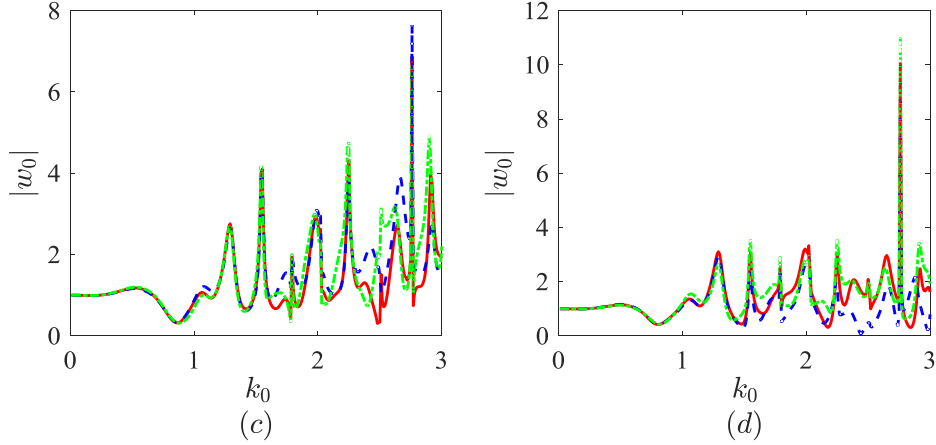
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 2 Fig. 11. Wave exciting force $f_{E,i}$ of a FPSO floating in polynya with different positions against wave number
 3 k_0 . (As in Fig. 10.)



4
 5
 6 Fig. 12. Wave elevation w_0 in polynya along the ice edge due to incident and diffracted waves at different wave
 7 number k_0 . (a) $k_0 = 0.01$; (b) $k_0 = 1.0$; (c) $k_0 = 2.0$; (d) $k_0 = 3.0$. Solid lines: centrally located with
 8 $(x_0, y_0) = (0, 0)$; dashed lines: near the right ice sheet edge with $(x_0, y_0) = (1, 0)$; dash-dotted lines: near the top
 9 ice sheet edge with $(x_0, y_0) = (0, 1)$. ($l = 1$, $c = 5$, $H = 10$, $h = 0.1$, $m = 0.09$, $L = 4.5582$, $\beta = \pi / 4$)



10



1
2 Fig. 13. Wave elevation w_0 in polynya due to incident and diffracted waves against wave number k_0 at
3 different ice edge positions. (a) $\theta = -3.11$; (b) $\theta = -1.54$; (c) $\theta = 0.03$; (d) $\theta = 1.60$. Solid lines: centrally
4 located with $(x_0, y_0) = (0, 0)$; dashed lines: near the right ice sheet edge with $(x_0, y_0) = (1, 0)$; dash-dotted lines:
5 near the top ice sheet edge with $(x_0, y_0) = (0, 1)$. ($l = 1$, $c = 5$, $H = 10$, $h = 0.1$, $m = 0.09$, $L = 4.5582$,
6 $\beta = \pi / 4$)

7 5. Conclusions

8 We have developed an accurate and efficient numerical solution approach for wave radiation
9 and diffraction problems of a three dimensional floating structure in a polynya. To solve the
10 governing equation with nonhomogeneous upper surface conditions, a vertical control surface
11 along the ice edge is introduced, which separates the fluid into two sub-domains. In exterior region
12 with an infinitely extended ice sheet, the upper surface condition contains up to fifth derivative of
13 the potential. To satisfy this and the radiation condition analytically, the velocity potential has been
14 written in the form of vertical eigenfunction series with the unknown functions in the horizontal
15 direction, which satisfy the two dimensional Helmholtz equation and are then expressed in terms
16 of the integral equation along the ice sheet edge. In interior region with a free surface, the velocity
17 potential has been written in form of the integral equation over the structure and control surfaces,
18 where the free surface Green function is taken as the kernel. On the control surface extended
19 vertically from the ice edge, both the velocity potential and the Green function are expanded into
20 the orthogonal eigen function series in the vertical direction. Thus, the integral over the control
21 surface is transformed into the integral along the ice edge, the same as that in exterior domain. The
22 unknowns in both interior and exterior sub-domains are solved together through an inner product,
23 where the continuity condition of pressure and velocity is used. Through this the ice edge
24 conditions are also satisfied.

25 Simulations have been first carried out for a bottom mounted vertical circular cylinder in a
26 circular polynya, and comparison has been made with the analytical solution derived previously.
27 Good agreements have been found, indicating that the method is accurate. The applicability of the

1 method to a realistic offshore structure with complicated geometry is demonstrated, through
2 applying the code WISPICE to the wave interactions with a FPSO floating in a polynya with
3 circular and square shapes respectively. The method can be used and extended to study the more
4 complex physical problems, for example, multiple body or multiple polynya problem by
5 distributing panels over each body surface or dividing the fluid domain into multiple sub-domains.
6 The same matching procedure can be followed. The method can be also easily extended to
7 polynya surrounded by an ice sheet of finite extent. In this case, in the third domain beyond the ice
8 sheet, the ice sheet thickness can be taken as zero and the same method used in the domain below
9 the ice sheet can be followed. Compared with the other methods for this type of linearized
10 problem, such as finite difference method or the finite element method, in the present hybrid
11 method, only the body surface and ice sheet edge need to be discretized, and the boundary
12 conditions on free surface and ice sheet as well as the radiation condition are all satisfied
13 automatically through either the Green function or the vertical modes of the potential. However,
14 the method is limited to the scope of the linear velocity potential theory, which is not valid when
15 the wave amplitude or the body motion amplitude is large compared with the wavelength or the
16 body dimension.

17 **Acknowledgement**

18 This work is supported by Lloyd's Register Foundation, to which the authors are most grateful.
19 Lloyd's Register Foundation helps to protect life and property by supporting engineering-related
20 education, public engagement, and the application of research. This work is also supported by the
21 National Natural Science Foundation of China (Grant No. 51709131 and 51879123)

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